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**RE: Regent Park Revitalization Phases 4 & 5 – Proposed Mixed-Use Development  
Zoning By-law Amendment Application  
Response to City Comments & Transportation Study Addendum**

## **1.0 INTRODUCTION**

BA Group is retained by Toronto Community Housing Corporation (TCHC) and their development partner Deltera Inc. (Tridel) to provide urban transportation consulting services in relation to a proposed mixed-use development pertaining to the lands generally bounded by Dreamers Way in the west, Gerrard Street East in the north, River Street in the east, and Oak Street to the south (excluding 40 Oak Street; the Fred Victor building) located in the City of Toronto (herein referred to as the “Site”). The municipal addresses are 325-433 Gerrard Street East, 295 & 325 Sackville Street, 259-267 & 266-272 Sumach Street, and 184-206 River Street.

The Site is part of a larger multi-phased development project known as the *Regent Park Revitalization*; the proposed redevelopment pertains to Phases 4 and 5 which are the final two phases. The *Regent Park Secondary Plan* established a framework to guide the phased redevelopment of the area to achieve a revitalized mixed income and mixed use neighbourhood.

## **1.1 STUDY BACKGROUND**

In April of 2022, BA Group submitted a transportation study as part of the Zoning By-law Amendment (ZBA) application (herein referred to as the “April 2022 TIS”). The study proposed a mixed-use development inclusive of residential uses of varied tenure, ground-floor retail units, community uses and a relocated Parliament Street library branch.

Since the April 2022 TIS, comments have been received from the City of Toronto’s Engineering & Construction Services Division in a memorandum dated August 15, 2022. Additionally, several changes regarding the development plan and the supporting road network within the Site lands are now being proposed.

## 1.2 DEVELOPMENT PLAN UPDATE

Since the April 2022 TIS, the development program has been revised. A comparison of key characteristics for each block within the previous and current development is provided in **Table 1**. Reduced architectural Site plans are included in **Appendix A** for reference.

The mixed-use development is inclusive of residential uses of varied tenure, ground-floor retail units, community uses, and a relocated library. In total, the current development program includes approximately 3,070 residential dwelling units (1,876 market units and 1,194 affordable units) and 15,490 m<sup>2</sup> non-residential GFA. This represents an increase of 96 residential units and 1,774 m<sup>2</sup> non-residential GFA in comparison to the April 2022 program.

Proposed retail/commercial non-residential space is not proposed at this time to be separated into “office” and “retail” uses. This separation – based on a 25% office and 75% retail allocation assumption – is included in this report for analytical purposes only and is subject to change at the detailed design phase.

The current development plan also includes the introduction of two additional north-south linkages which extend between Oak Street and Gerrard Street East: a private street between Dreamers Way and Sackville Street and the extension of Tubman Avenue.

## 1.3 THIS LETTER

This addendum letter provides responses to the received City comments, discusses and analyzes the changes to the development plans, and provides updates to the transportation study where applicable for the purposes of the ZBA submission.



**TABLE 1 DEVELOPMENT PROGRAM**

Block	Market Residential	Affordable Residential	Office <sup>1</sup>	Retail <sup>1</sup>	Community/Culture Uses	Library
<b>Previous April 2022 Submission</b>						
Building 1A	--	201 units	233 m <sup>2</sup> GFA	699 m <sup>2</sup> GFA	1,350 m <sup>2</sup> GFA	--
Buildings 1B & 1C	679 units	--	328 m <sup>2</sup> GFA	984 m <sup>2</sup> GFA	--	--
Buildings 2D & 2E	503 units	--	508 m <sup>2</sup> GFA	1,523 m <sup>2</sup> GFA	--	--
Buildings 2F & 2G	--	--	--	--	1,164 m <sup>2</sup> GFA	2,277 m <sup>2</sup> GFA
Buildings 2H & 2I	--	332 units	376 m <sup>2</sup> GFA	1,127 m <sup>2</sup> GFA	554 m <sup>2</sup> GFA	--
Buildings 3J & 3K	610 units	--	253 m <sup>2</sup> GFA	760 m <sup>2</sup> GFA	--	--
Buildings 4L & 4M	--	648 units	181 m <sup>2</sup> GFA	542 m <sup>2</sup> GFA	860 m <sup>2</sup> GFA	--
<b>Phases 4 &amp; 5 Previous Total</b>	<b>1,792 units</b>	<b>1,181 units</b>	<b>1,879 m<sup>2</sup> GFA</b>	<b>5,635 m<sup>2</sup> GFA</b>	<b>3,930 m<sup>2</sup> GFA</b>	<b>2,277 m<sup>2</sup> GFA</b>
	<b>Residential Unit Count Total: 2,973 units</b>		<b>Non-Residential GFA Total: 13,721 m<sup>2</sup> GFA</b>			
<b>Current Submission</b>						
Building 1A	--	202 units	237 m <sup>2</sup> GFA	711 m <sup>2</sup> GFA	1,358 m <sup>2</sup> GFA	--
Buildings 1B & 1C	713 units	--	330 m <sup>2</sup> GFA	989 m <sup>2</sup> GFA	--	--
Buildings 2D & 2E	505 units	--	511 m <sup>2</sup> GFA	1,532 m <sup>2</sup> GFA	--	--
Buildings 2F & 2G	--	--	380 m <sup>2</sup> GFA	1,139 m <sup>2</sup> GFA	1,164 m <sup>2</sup> GFA	2,277 m <sup>2</sup> GFA
Buildings 2H & 2I	--	333 units	376 m <sup>2</sup> GFA	1,127 m <sup>2</sup> GFA	572 m <sup>2</sup> GFA	--
Buildings 3J & 3K	657 units	--	270 m <sup>2</sup> GFA	809 m <sup>2</sup> GFA	--	--
Buildings 4L & 4M	--	659 units	199 m <sup>2</sup> GFA	596 m <sup>2</sup> GFA	916 m <sup>2</sup> GFA	--
<b>Phases 4 &amp; 5 Current Total</b>	<b>1,876 units</b>	<b>1,194 units</b>	<b>2,301 m<sup>2</sup> GFA</b>	<b>6,902 m<sup>2</sup> GFA</b>	<b>4,010 m<sup>2</sup> GFA</b>	<b>2,277 m<sup>2</sup> GFA</b>
	<b>Residential Unit Count Total: 3,070 units</b>		<b>Non-Residential GFA Total: 15,490 m<sup>2</sup> GFA</b>			
<b>Differences</b>						
Building 1A	--	+1 units	+4 m <sup>2</sup> GFA	+12 m <sup>2</sup> GFA	+8 m <sup>2</sup> GFA	--
Buildings 1B & 1C	+34 units	--	+2 m <sup>2</sup> GFA	+5 m <sup>2</sup> GFA	--	--
Buildings 2D & 2E	+2 units	--	+3 m <sup>2</sup> GFA	+9 m <sup>2</sup> GFA	--	--
Buildings 2F & 2G	--	--	+380 m <sup>2</sup> GFA	+1,139 m <sup>2</sup> GFA	--	--
Buildings 2H & 2I	--	+1 units	--	--	+18 m <sup>2</sup> GFA	--
Buildings 3J & 3K	+47 units	--	+17 m <sup>2</sup> GFA	+49 m <sup>2</sup> GFA	--	--
Buildings 4L & 4M	--	+11 units	+18 m <sup>2</sup> GFA	+54 m <sup>2</sup> GFA	+56 m <sup>2</sup> GFA	--
<b>Phases 4 &amp; 5 Total Difference</b>	<b>+83 units</b>	<b>+13 units</b>	<b>+424 m<sup>2</sup> GFA</b>	<b>+1,268 m<sup>2</sup> GFA</b>	<b>+82 m<sup>2</sup> GFA</b>	<b>--</b>
	<b>Residential Unit Count Total: +96 units</b>		<b>Non-Residential GFA Total: +1,774 m<sup>2</sup> GFA</b>			

Notes:

- Proposed retail/commercial non-residential space is not proposed at this time to be separated into "office" and "retail" uses. This separation – based on a 25% office and 75% retail allocation assumption – is included in this report for analytical purposes only and is subject to change at the detailed design phase.



## 2.0 RESPONSES TO CITY COMMENTS

### A. REVISIONS TO PLANS AND ADDITIONAL INFORMATION REQUIRED FOR PLANS, STUDIES AND DRAWINGS

#### Transportation Services

**Comment 1.1:** Continue to consult with City Planning and Transportation Services with regards to the proposed overall site layout and previously agreed upon public rights-of-way through the proposed site area, as further discussed in Section D of this memorandum.

**Response:** Noted.

**Comment 1.2:** Design, in consultation with Cycling and Pedestrian Projects, and City Planning/Urban Design, and construct a bi-directional bikeway along Gerrard Street East, including the following (as further discussed in Section D of this memorandum):

- a) Provide a raised two-way cycle track of 3.0 metres in width with:
  - buffer zone between the roadway and the cycle track between 0.8-1.0 metres;
  - A buffer zone between the cycle track and pedestrian clearway/sidewalk between 0.6-0.8 metres;
- b) Provide TTC transit platforms in accordance with the June 2022 design specifications (pending consultation with TTC with regards to the location of transit stops);
- c) Provide crossing conditions at the signalized intersections along Gerrard Street East in accordance with Ontario Traffic Manual Book 18 – Protected Intersections;
- d) Provide a raised bikeway and pedestrian crossing along Gerrard Street East across its intersection with Dreamers Way, in accordance with the City's Raised Crosswalk and Intersection Guidelines;
- e) Provide appropriate ramp/grades at the terminus points of the cycle track to be constructed within the site frontages on Gerrard Street East.

**Response:** The project team will work with City staff to further refine cycling facilities along this corridor within the existing Right-of-Way.

**Comment 1.3:** Provide a raised cycle track along the west side of River Street, extending from the Gerrard Street East intersection through the new signalized intersection at Oak Street, in accordance with the City's March 2022 design specification. (This specification can be provided upon request).

**Response:** The rezoning application does not preclude the implementation of cycling facilities on River Street. The design of cycling facilities along the River Street corridor should be reviewed in a fulsome manner by staff, given the implications to lands further south, which are not part of this application..

**Comment 1.4:** Provide raised contra-flow cycle tracks on Sackville Street and Sumach Street, extending from Gerrard Street East to Oak Street within the site lands, in accordance with the City's March 2022 design specification. (This specification can be provided upon request).

**Response:** BA Group has discussed this item with City Transportation Services staff. In order to maintain fire routing requirements, the design will be maintained as shown in the initial submission.



**Comment 1.5:** Label/provide minimum unobstructed/consistent 3.0 metre wide pedestrian clearway along Gerrard Street East, and identify where Pedestrian Clearway Easements will be needed to secure the required public space, as further discussed in Section D of this memorandum.

**Response:** Please refer to the updated architectural plans. The Gerrard Street corridor will be refined through further discussions with City staff.

**Comment 1.6:** Label/provide minimum unobstructed/consistent 2.1 metre wide pedestrian clearways along Oak Street, Sackville Street, Sumach Street, Dreamers Way, and River Street, and identify where Pedestrian Clearway Easements will be needed to secure the required public space, as further discussed in Section D of this memorandum.

**Response:** Please refer to the updated current architectural plans.

**Comment 1.7:** Design extensions of the public roadway for Sackville Street and Sumach Street. Revise the functional road plans as follows:

- a) Clarify the need for an R10 at the southwest corner of Gerrard Street East/Sackville Street. If possible, via a review of vehicle manoeuvring diagrams, reduce this curb radius;
- b) Reduce the curb radii at the southeast corner of Gerrard Street East/Sackville Street and the southwest corner of Gerrard Street East/Sumach Street to be 3.0 metres or less, e.g. squared off as per the one-way street configuration.

**Response:**

a) The R10 would be required to permit an HSU and a garbage truck to turn from the eastbound curb lane into the southbound travel lane on Sackville Street without encroaching into the adjacent travel lanes on Gerrard Street East and proposed bike lanes on Sackville Street.

b) Noted. This change has been made.

**Comment 1.8:** Delineate and identify unobstructed/unencumbered corner roundings on the site and landscape plans as lands to be conveyed to the City for a nominal sum at the following locations:

- a) 5.0 metre corner roundings at the southwest and southeast corners of Sackville Street/Gerrard Street East;
- b) 5.0 metre corner roundings at the southwest and southeast corners of Sumach Street/Gerrard Street East;
- c) 5.0 metre corner rounding at the southeast corner of Dreamers Way/Gerrard Street East;
- d) 5.0 metre corner rounding at the southwest corner of River Street/Gerrard Street East.

**Response:** The architectural plans and functional road plan has been updated to include the specified corner roundings.

**Comment 1.9:** Delineate and identify unobstructed corner roundings on the site and landscape plans as lands to be secured via Pedestrian Clearway Easements at the following locations:

- a) 5.0 metre corner roundings at the northwest and northeast corners of Sackville Street/Oak Street;
- b) 5.0 metre corner roundings at the northwest and northeast corners of Sumach Street/Oak Street;
- c) 5.0 metre corner rounding at the northwest corner of River Street/Oak Street.

**Response:** The architectural plans and functional road plan has been updated to include the specified corner roundings.

**Comment 1.10:** *Revise the loading areas to provide Type B spaces in place of Type C spaces where appropriate. Given the scale/sharing of non-residential GFA, this includes the loading area in Buildings 1A, 2E, 2I, 3K, and 3M, as further discussed in Section D of this memorandum.*

**Response:** All Type C spaces have been replaced with Type B spaces in the plan for the majority of loading areas. It is noted that Building 1A operates with its own loading area and will not be shared with other buildings. Application of 569-2013 to Building 1A would require 1 Type G space and no additional Type B space.

**Comment 1.11:** *Provide accessible parking spaces in accordance with Zoning By-law No. 89-2022, as noted, and label these spaces with acceptable dimensions in accordance with Zoning By-law No. 579-2017, as further discussed in Section D of this memorandum.*

**Response:** Noted.

**Comment 1.12:** *Provide a preliminary assessment of the supply and location of publically accessible car-share spaces to be provided within the overall site lands.*

**Response:** Car-share spaces would be provided within non-residential sections of parking garages. The precise location of these would be identified within future Site Plan Applications.

**Comment 1.13:** *Re-locate/consolidate the proposed loading access driveways on Gerrard Street East to be provided via Sackville Street, Sumach Street, or Oak Street.*

**Response:** Revisions to the loading and access have been included in the Revised Proposal. The plan provided with the zoning by-law amendment application is conceptual and therefore the location of loading and access driveways will be addressed the at Site Plan Application

**Comment 1.14:** *Review the feasibility of consolidating the proposed vehicular and loading access driveways on River Street to be provided via one (1) curb cut.*

**Response:** The plan seeks to provide the Tubman Avenue extension (Street 'J') as a pedestrian-friendly environment and shifts access towards River Street for both parking and loading. These facilities have been separated to reduce impacts to building ground floor spaces.



**Comment 1.15:** Re-locate the loading driveways or raised crossings on Sackville Street and Sumach Street so that they are not in conflict with each other, as further discussed in Section D of this memorandum.

**Response:** Revisions to loading and access have been included in the Revised Proposal but details will be addressed at Site Plan Application

### **Solid Waste Services**

**Comment 2.1:** Revise drawings to indicate and annotate the following with regard to the Multi-Residential Component:

- a) The Type G loading space is 13 metres in length, 4 metres in width
- b) The staging pad abutting the front of the Type G loading space will be at least
  - i. Building 1A – 20.1 square metres
  - ii. Building 1C – 67.9 square meters
  - iii. Building 2E – 50.3 square metres
  - iv. Building 2I – 33.2 square metres
  - v. Building 3K – 61 square metres
  - vi. Building 3M – 64.8 square metres
- c) The Type G loading space and the staging pad abutting the front of the Type G loading space has an unencumbered vertical clearance of 6.1 metres.
- d) A bulky storage area of minimum floor area of at least 10 square metres.
- e) A garbage storage room with a minimum floor area based on the number of units. Buildings with multiple waste rooms due to towers or other reasons will require appropriately sized waste storage rooms based on the total number of units associated with them. Detailed breakdown of units required to provide minimum requirements for waste storage room.
- f) The ability of the collection to enter and exit the site in a forward motion with no more than a three-point turn.
- g) All overhead doors have a minimum vertical clearance of 4.4 metres, and a minimum width of 4 metres.

**Comment 2.2:** Revise drawings to indicate and annotate the following with regard to the Non-Residential Component:

- a) A storage space for the waste that will be generated by the non-residential component of this development. This non-residential waste room must be independent from the residential waste room and must be accessible without entering the residential waste room.

**Comment 2.3:** Revise drawings to indicate and annotate the following with regard to the Toronto Public Library Collection:

- a) Collection of waste materials for this development will take place curbside on Gerrard St E.
- b) A waste storage area that located within the building and is at least 12 square meters, but may need to be larger to allow storage of all waste material between collection days.



**Comment 2.4:** *Revise drawings to indicate the collection vehicle, which is a length of 12 metres and a width of 2.4 metres, has a minimum turning radii of 9.5m inside and 14m outside entering, exiting and travelling throughout the site and the type G loading space in an unobstructed fashion.*

**Comment 2.5:** *Show on the drawings that all access driveways to be used by the collection vehicle will have:*

- a) *A maximum gradient of 8%;*
- b) *A minimum vertical clearance of 4.4 metres throughout;*
- c) *A minimum width of 4.5 metres throughout; and,*
- d) *6 metres wide at point of ingress and egress.*

**Responses:** These comments will be address as part of Site Plan Applications for each building / development phase.

## **B. (PRELIMINARY) ZONING BY-LAW AMENDMENT CONDITIONS**

**Comment 1.1:** *Provide and maintain parking spaces in accordance with the requirements of Zoning By-law No. 89-2022 (amended as No. 125-2022) for Parking Zone A.*

**Response:** The revised parking requirement calculations based on By-law 89-2022 and the currently proposed parking strategy is provided in **Section 4.0**.

**Comment 1.2:** *Provide and maintain accessible parking spaces in accordance with the requirements of Zoning By-law No. 89-2022 (amended as No. 125-2022) for Parking Zone A, with minimum dimensions to be provided in accordance with Zoning By-law No. 579-2017.*

**Response:** Noted.

**Comment 1.3:** *The overall development site is required to provide a minimum number of publically accessible car-share spaces within each block. This total is to be determined, as noted in Section A.*

**Response:** Noted.

**Comment 1.4:** *Include the following definitions in the Site-Specific By-law for this project:*

- a) *Car-share means the practice where a number of people share the use of one or more cars that are owned by a profit or non-profit car-sharing organization and where such organization may require that use of cars to be reserved in advance, charge fees based on time and/or kilometers driven, and set membership requirements of the car-sharing organization, including the payment of a membership fee that may or may not be refundable;*
- b) *Car-share parking space means a parking space that is reserved and actively used for car-sharing.*

**Response:** Noted.





**Comment 1.5:** Provide and maintain loading spaces in accordance with the following minimums and with dimensions as per Zoning By-law No. 569-2013:

*For Block/Phase 1:*

- a) Two (2) Type G spaces;
- b) One (1) Type B space;
- c) One (1) Type C space;

*For Block/Phase 2:*

- a) Two (2) Type G spaces;
- b) Two (2) Type B spaces;

*For Block/Phase 3:*

- a) Two (2) Type G spaces; and
- b) Two (2) Type B spaces.

**Response:** The proposed revised loading requirements are summarized in **Section 6.0**.



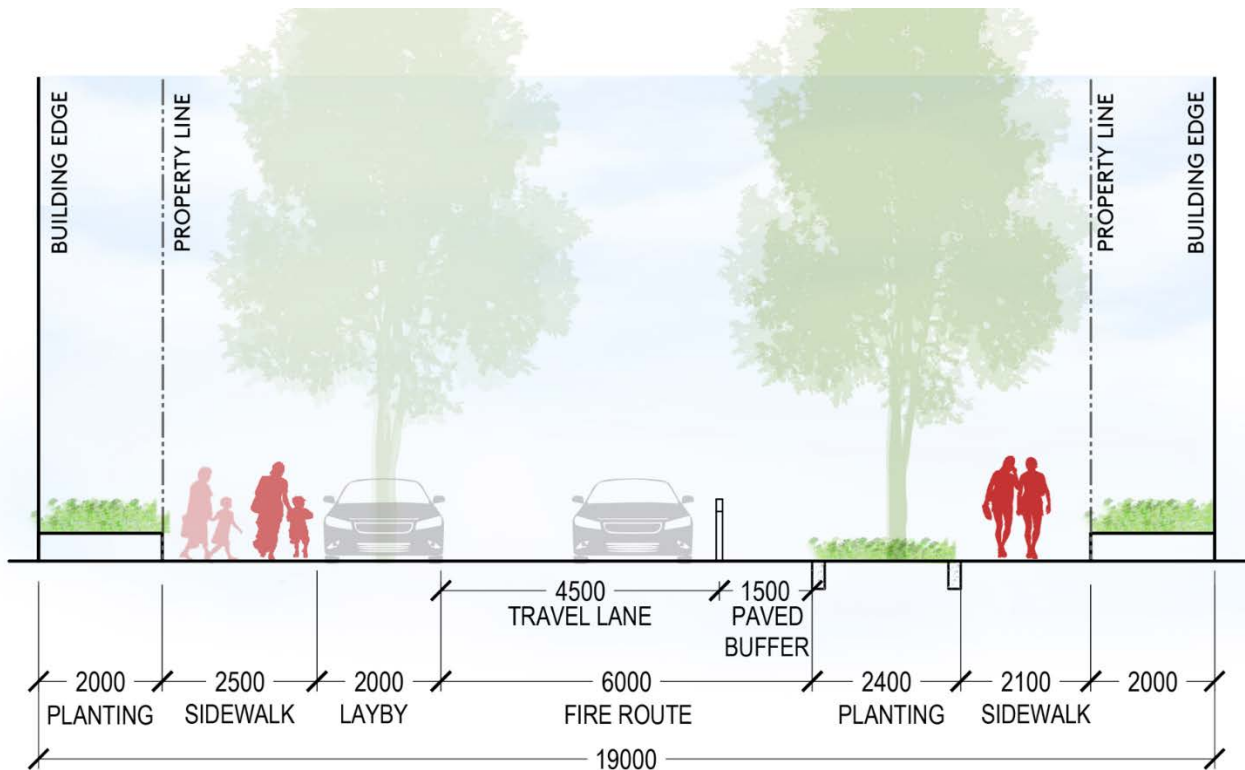
### 3.0 STREET NETWORK & BLOCK DESIGN UPDATES

The Regent Park Revitalization Plan for Phases 4 and 5 includes a reconfiguration of streets located within or adjacent to the Site in order to foster a more complete, balanced, and connected network. Streets that are currently privately-owned (i.e. Sackville Street and Sumach Street) will become public streets. The functional road plan is provided in **Appendix B**.

While key street design objectives remain the same as the previous submission, several road network changes listed below are now proposed. These further enhance connectivity within the plan.

### 3.1 TUBMAN AVENUE EXTENSION (STREET 'J')

A public street extension of Tubman Avenue (Street 'J') is proposed between Gerrard Street East in the north to Oak Street in the south. The street will have a 15.0m right-of-way, including a 4.5m one-way southbound vehicle travel lane, a 1.5m paved buffer and a 2.0m parking lay-by. The paved buffer has been provided on the east side of the street to reduce the travel width the vehicular lanes and provide additional space for active travel modes. Sidewalks are provided on both sides of the street.

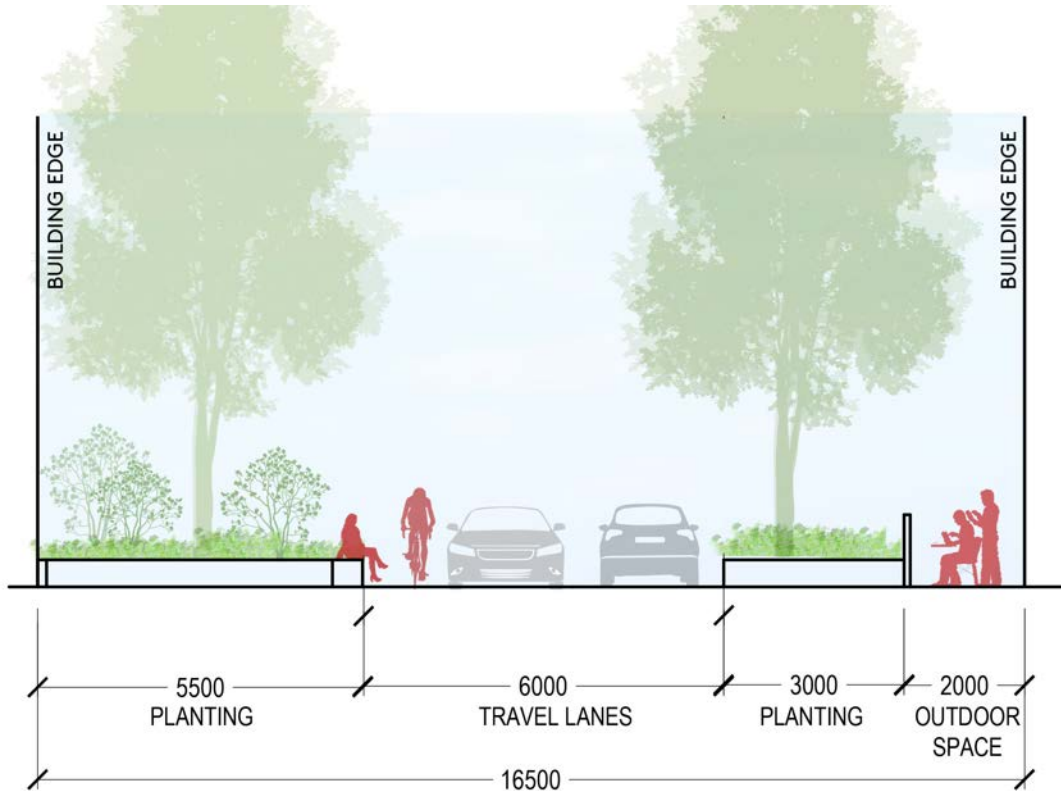


The fire route comprises of the 4.5m travel lane and the 1.5m paved buffer. These areas are separated by bollards. These dimensions accommodate the travel path of a fire truck and permit the fire truck to activate the stabilizing legs between the bollards as shown in the vehicle manoeuvring diagrams VMD-08 to VMD-10 provided in **Appendix E**.



### 3.2 NEW NORTH-SOUTH PRIVATE STREET (STREET 'G')

A new north-south private street between Building 1A and 1B (referred to as Street 'G') is proposed, extending between Gerrard Street East and Oak Street is proposed. This would be 6.0m in width accommodating two-way traffic flow. The private street has been designed to provide connectivity to loading areas within Buildings 1A, 1B and the Fred Victor Building.



### 3.3 FRED VICTOR SERVICING LANE

A new east-west driveway extending from Dreamers Way to the new private north-south street is also proposed. It has a width of 4.5m and is intended to operate one-way eastbound, to facilitate loading operations associated with the Fred Victor Building. This connection is shown in the Functional Road plan in **Appendix B**.

## 4.0 VEHICULAR PARKING CONSIDERATIONS

### 4.1 VEHICULAR PARKING SPACE REQUIREMENTS

#### 4.1.1 Current Zoning Considerations

As noted in the April 2022 TIS, the Site (and much of its surrounding area) is not under the jurisdiction of current comprehensive City of Toronto Zoning By-law 569-2013; it is under the jurisdiction of former City of Toronto Zoning By-law 438-86. Area-specific By-law 141-2005 was carried forward and maintained as “Exceptions Applying to Specific Use Districts or Specific Lands” in Zoning By-law 438-86, while Area-specific By-law 275-2014 was passed in implementation of the City of Toronto Secondary Plan for the Regent Park Area.

The relevant Site-specific minimum parking requirements for the Site are the following:

- Residential: requirements outlined in By-law 275-2014
- Residential Visitor: requirements outlined in By-law 141-2005
- Social Housing: requirements outlined in By-law 141-2005
- Social Housing Visitor: By-law 141-2005 states that no visitor parking spaces shall be required in respect of dwelling units which are social housing building
- Office: requirements outlined in Zoning By-law 438-86 (Downtown Parking and Loading Zone)
- Retail: requirements outlined in Zoning By-law 438-86 (Downtown Parking and Loading Zone)
- Community/Culture Uses: requirements outlined in Zoning By-law 438-86 (Downtown Parking and Loading Zone)
- Library Uses: requirements outlined in Zoning By-law 438-86 (Downtown Parking and Loading Zone)

#### 4.1.2 Minimum Vehicular Parking Requirements

The minimum parking space supply requirement for the entirety of Phases 4 and 5, in aggregate, is outlined in **Table 2** based upon the currently applicable minimum parking requirements. Minimum parking space requirements, per block, are included in **Appendix C**.

Application of the minimum parking requirements associated with Zoning By-law 438-86 and the area-specific parking requirements results in a total Site parking requirement of 1,616 parking spaces after the application of time-of-day sharing factors.



**TABLE 2 PHASES 4 & 5 ZONING BY-LAW MINIMUM PARKING REQUIREMENTS (IN AGGREGATE)**

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
<b>Residential</b>						
1-Bedroom (Market)	1,304 units	0.41 spaces per unit	534 spaces	100%	100%	100%
2-Bedroom (Market)	453 units	0.58 spaces per unit	263 spaces			
3-Bedroom (Market)	121 units	1.04 spaces per unit	126 spaces			
Affordable Housing	1,194 units	0.40 spaces per unit	478 spaces			
<b>Resident Sub-Total</b>	<b>3,070 units</b>	<b>0.46 spaces per unit (blended)</b>	<b>1,401 spaces</b>	<b>1,401</b>	<b>1,401</b>	<b>1,401</b>
<b>Non-Residential</b>						
Market Residential Visitors	1,876 units	0.06 spaces per unit	112 spaces	0 0%	39 35%	112 100%
Affordable Housing Residential Visitors	1,194 units	0.00 spaces per unit	0 spaces	0 0%	0 35%	0 100%
Community	3,605 sm GFA	1 spaces / 175 sm GFA	20 spaces	0 0%	7 35%	20 100%
Library	2,277 sm GFA	1 spaces / 175 sm GFA	13 spaces	13 100%	13 100%	13 100%
Culture	405 sm GFA	1 spaces / 175 sm GFA	2 spaces	2 100%	2 100%	2 100%
Office	2,301 sm GFA	1 spaces / 300 sm GFA	8 spaces	8 100%	5 60%	0 0%
Retail	6,902 sm GFA	1 spaces / 100 sm GFA	68 spaces	14 20%	68 100%	68 100%
<b>Non-Resident Sub-Total</b>			<b>223 spaces</b>	<b>37</b>	<b>134</b>	<b>215</b>
<b>TOTAL</b>			<b>1,624 spaces</b>	<b>1,438</b>	<b>1,535</b>	<b>1,616</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Calculation of required parking supply is not directly based on the minimum parking requirement rates in **Table 2**; it is the aggregate of the calculation of minimum parking requirements for each block, as outlined in **Appendix C**.



### 4.1.3 Zoning By-law 89-2022 Parking Supply Requirements

Based on comment provided by City staff, the parking requirements resulting from Zoning By-law 569-2013, as amended by 89-2022, are also evaluated. By-laws 89-2022 and 125-2022 were passed (and now in effect as of October 12, 2022) by City Council in February 2022 to amend the vehicle parking requirements outlined in the current City of Toronto Zoning By-law 569-2013. These standards generally aim to improve the efficiency and effectiveness of local parking as the City continues to evolve. The formation of this By-law (e.g. turning minimum requirements into maximum requirements for all land uses) reflects the City's direction and intent to replace the minimum requirements of in-force Zoning By-law 569-2013, making other existing Former Zoning By-laws generally outdated.

The Site falls under Parking Zone 'A' boundaries, as governed by Zoning By-law 89-2022 (as amended by Zoning By-law 125-2022). Application of minimum and maximum parking requirements to the Site is summarized in **Table 3**. It is notable that no minimum parking spaces are required for residents, with exception to accessible parking spaces discussed in **Section 4.1.4.2**. Visitor spaces are required, but only for market buildings.

Under By-law 89-2022, the Site development requires a minimum of 53 accessible parking spaces, inclusive of 24 accessible residential visitor parking spaces and remainder distributed equitably between the proposed uses.



**TABLE 3 BY-LAW 89-2022 MINIMUM AND MAXIMUM PARKING REQUIREMENTS (INCLUDING ACCESSIBLE PARKING)**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum Parking Required <sup>3</sup> (Spaces)	Maximum Parking Provided (Ratio)	Maximum Parking Provided <sup>3</sup> (Spaces)	Effective Parking Required (Ratio)	Effective Parking Required <sup>3,4</sup> (Spaces)
<b>Residential</b>							
Bachelor	0	None	0	0.30 spaces per unit	0	0.30 spaces per unit	0
1-Bedroom	1,304 units	None	0	0.50 spaces per unit	652	0.50 spaces per unit	652
2-Bedroom	453 units	None	0	0.80 spaces per unit	361	0.80 spaces per unit	361
3-Bedroom	121 units	None	0	1.00 spaces per unit	121	1.00 spaces per unit	121
Affordable Housing <sup>1</sup>	1,194 units	None	0	0.50 spaces per unit	596	0.20 spaces per unit	237
<b>Resident Sub-Total</b>	<b>3,070 units</b>	<b>None</b>	<b>0</b>	<b>0.56 spaces per unit (blended)</b>	<b>1,730</b>	<b>0.45 spaces per unit (blended)</b>	<b>1,371</b>
<b>Non-Residential</b>							
Market Residential Visitors	1,876 units	2 plus 0.01 spaces per unit	24	5 plus 0.10 spaces per unit	201	0.10 spaces per unit	186
Affordable Housing Residential Visitors <sup>2</sup>	1,194 units	None	0	None	0	None	0
Community	3,605 sm GFA	None	0	0.80 spaces per 100 sm GFA	27	0.40 spaces per 100 sm GFA	13
Library	2,277 sm GFA	None	0	3.50 spaces per 100 sm GFA	79	1.00 spaces per 100 sm GFA	22
Culture	405 sm GFA	None	0	0.80 spaces per 100 sm GFA	3	0.40 spaces per 100 sm GFA	1
Office	2,301 sm GFA	None	0	0.80 spaces per 100 sm GFA	16	0.40 spaces per 100 sm GFA	6
Retail	6,902 sm GFA	None	0	3.50 spaces per 100 sm GFA	237	1.00 spaces per 100 sm GFA	66
<b>Non-Resident Sub-Total</b>		--	<b>24</b>	--	<b>563</b>	--	<b>294</b>
<b>TOTAL</b>		--	<b>24</b>	--	<b>2,293</b>	--	<b>1,665</b>
<b>Adjusted Minimum Parking Requirement<sup>5</sup></b>							
53 accessible parking spaces.							<b>53</b>

Notes:

- By-law 89-2022 does not specify parking requirements for affordable housing uses. Instead, parking rates that were provided for alternative housing uses within the By-law were assumed to best represent the proposed affordable housing uses.
- Visitor parking spaces are not required under By-law 89-2022 for alternative housing uses on the Site.
- Refer to Table 200.5.10.1 of By-law 89-2022. If the number of required parking spaces results in a number with a fraction, the number is rounded down to the nearest whole number but there may not be less than one parking space.
- Refer to Table 200.15.10.5 of By-law 89-2022. Application of "Effective" Parking Ratio and Requirement is a procedural requirement, stipulated by By-law 89-2022, intended to calculate the required quantity of parking spaces to only be used for determining the minimum number of accessible parking spaces required.
- Based on Section 200.15.10.10 of By-law 89-2022.
- Calculation of required parking supply is not directly based on the minimum parking requirement rates in **Table 3**; it is the aggregate of the calculation of minimum parking requirements for each block, as outlined in **Appendix C**.



#### 4.1.4 Proposed Vehicular Parking Supply

Within the architectural plans (see **Appendix A**), vehicular parking has been provided in accordance with the following rates:

##### Market Housing Blocks:

- Residents: 0.40 spaces per unit
- Residential visitors: 0.06 spaces per unit (to be provided on a shared, non-exclusive basis with parking for non-residential uses)
- Non-residential uses: no dedicated parking

##### Affordable Housing / Municipal Blocks:

- Residents: 0.40 spaces per unit
- Residential visitors: no parking
- Non-residential uses: no parking

Notably, the proposed parking supply is compliant with Zoning By-law 569-2013, as amended by Zoning By-laws 89-2022 and 125-2022. The proposed parking supply is greater than the minimum parking requirement and lesser than the maximum parking requirement.

The provided parking supply for the entirety of Phases 4 and 5, in aggregate, is outlined in **Table 4** based upon the application of the above parking supply rates. The proposed parking supply, per block, is included in **Appendix C**.

Application of the parking supply rates results in a total Site parking supply of 1,340 parking spaces. The site is seeking to provide 1,341 parking spaces.

##### 4.1.4.1 Non-Standard Parking Spaces

It is expected at this time that several parking spaces (number to be determined at the Site Plan Approval stage of the development process) within the proposed parking garages will not meet the parking space dimensional requirements of City of Toronto Zoning By-laws 438-86 and 569-2013.

The relevant and basic parking space dimensional requirements are 2.6 metres in width, 5.6 metres in length, 2.0 metres in height, and be accessed by a 6.0 metre drive aisle.

Parking spaces that do not meet all of the above requirements will be small car / obstructed parking spaces. As building plans will adapt to respond to City comments and the acceleration of the design process, the number of small car / obstructed spaces will change. It is expected that the number of obstructed parking spaces will not exceed 10% of the total parking space supply within the parking garages. The small car / obstructed parking spaces will be reviewed to determine functionality; this analysis will be conducted in conjunction with a subsequent submission associated with the project.





**TABLE 4 PHASES 4 & 5 PARKING SUPPLY (IN AGGREGATE)**

Use / Type	# of Units / GFA	Proposed Parking Supply (Ratio)	Proposed Parking Supply (Spaces)	Shared Parking		
				AM	PM	Evening
<b>Residential</b>						
All Market Units	1,876 units	0.40 spaces per unit	750 spaces	100%	100%	100%
All Affordable Units	1,194 units	0.40 spaces per unit	478 spaces			
<b>Resident Sub-total</b>	<b>3,070 units</b>	<b>0.40 spaces per unit</b>	<b>1,228 spaces</b>	<b>1,228</b>	<b>1,228</b>	<b>1,228</b>
<b>Non-Residential</b>						
Market Residential Visitors	1,876 units	0.06 spaces per unit	112 spaces	0 0%	39 35%	112 100%
Affordable Housing Residential Visitors	1,194 units	No parking	0 spaces	0 0%	0 35%	0 100%
Community	3,605 sm GFA	No parking	0 spaces	0 0%	0 35%	0 100%
Library	2,277 sm GFA	No parking	0 spaces	0 100%	0 100%	0 100%
Culture	405 sm GFA	No parking	0 spaces	0 100%	0 100%	0 100%
Office	2,301 sm GFA	No parking	0 spaces	0 100%	0 60%	0 0%
Retail	6,902 sm GFA	No parking	0 spaces	0 20%	0 100%	0 100%
<b>Non-Resident Sub-Total</b>			<b>112 spaces</b>	<b>0</b>	<b>39</b>	<b>112</b>
<b>TOTAL</b>			<b>1,340 spaces</b>	<b>1,228</b>	<b>1,267</b>	<b>1,340</b>

Notes:

- Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
- Calculation of proposed parking supply is not directly based on the minimum parking requirement rates in **Table 4**; it is the aggregate of the calculation of minimum parking requirements for each block, as outlined in **Appendix C**.

#### 4.1.4.2 Accessible Parking Supply

Greater detail pertaining to accessible parking supply and functionality will be provided in subsequent redevelopment applications. It is intended for the Site to meet the accessible parking supply requirements of City of Toronto By-law 89-2022 for each Block, provided that parking supply is provided for a use on the block.

#### 4.1.4.3 Electric Vehicle Parking

As part of the Site's Sustainability Strategy, the Proposed Development will meet Toronto Green Standard (TGS) Version 4 (V4). For electric vehicle parking, City of Toronto By-law 89-2022 further updated requirements. As a result, all resident parking spaces (1,228 parking spaces) and 25% of non-resident parking spaces (28 parking spaces) will include an energized outlet capable of providing Level 2 charging or higher to the parking space. For the 75% of non-resident parking spaces omitted from the above, rough-in conduits will be provided to permit future Electric Vehicle Supply Equipment (EVSE) installation.



## 5.0 BICYCLE PARKING CONSIDERATIONS

### 5.1 BICYCLE PARKING REQUIREMENTS

As previously noted, the Site is not under the jurisdiction of the Zoning By-law 569-2013, but rather the former Zoning By-law 438-86. The minimum bicycle parking requirements of Zoning By-law 438-86 are applicable to the Site. However, the Toronto Green Standards (TGS) Version 4.0 (V4) for Zone 1 (Tier 1) bicycle parking standards (which are based upon Zoning By-law 569-2013) represent a more progressive approach, and as such have been applied to the development.

The Site is designated as Bicycle Zone 1 under Zoning By-law 569-2013, which sets the minimum bicycle parking requirements for developments located within the zone. These bicycle parking requirements are consistent with the minimum bicycle parking requirements of the Toronto Green Standards (TGS) Version 4, Tier 1 that apply to all developments within the City of Toronto.

#### 5.1.1 Minimum Bicycle Parking Requirements

The minimum bicycle parking space supply requirement for the entirety of Phases 4 and 5, in aggregate, is outlined in **Table 5** based upon the applicable minimum bicycle parking requirements. Minimum bicycle parking space requirements, per block, are included in **Appendix D**.

Application of the minimum bicycle parking requirements associated with TGS V4 Tier 1 (which are based upon Zoning By-law 569-2013) results in a total Site requirement of 3,175 bicycle parking spaces.

**TABLE 5 PHASES 4 & 5 ZONING BY-LAW MINIMUM BICYCLE PARKING REQUIREMENTS (IN AGGREGATE)**

Use / Type	Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Residential	3,070 units	Long-Term	0.90 spaces per unit	2,765 spaces
		Short-Term	0.10 spaces per unit	310 spaces
Community	3,605 sm GFA	Total	--	0 spaces
Library	2,277 sm GFA	Total	--	0 spaces
Culture	405 sm GFA	Total	--	0 spaces
Office	2,301 sm GFA	Long-Term	0.2 spaces per 100 sm GFA	8 spaces
		Short-Term	3 plus 0.2 spaces per 100 sm GFA	29 spaces
Retail	6,902 sm GFA	Long-Term	0.2 spaces per 100 sm GFA	18 spaces
		Short-Term	3 plus 0.3 spaces per 100 sm GFA	45 spaces
<b>Long-Term Subtotal</b>				<b>2,791 spaces</b>
<b>Short-Term Subtotal</b>				<b>384 spaces</b>
<b>TOTAL</b>				<b>3,175 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. In addition to the standards listed, the Toronto Green Standards require all short-term bicycle parking to be located in a highly visible location or at-grade or on the first parking level below-grade.
3. Calculation of required bicycle parking supply is not directly based on the minimum parking requirement rates in **Table 5**; it is the aggregate of the calculation of minimum parking requirements for each block, as outlined in **Appendix D**.



### 5.1.2 Proposed Bicycle Parking Supply

In comparison to the aforementioned minimum bicycle parking requirements, it is proposed to provide a bicycle parking supply that exceeds the requirements, as outlined in **Table 6** and as outlined in the architectural plans (**Appendix A**).

Bicycle parking has been provided in accordance with the following rates:

- Residential (Long-Term): 1.06 spaces per unit
- Residential (Short-Term): 0.17 spaces per unit
- Non-Residential Uses (Long-Term & Short-Term): 0.79 spaces per 100 m<sup>2</sup> GFA

The application of the rates, in aggregate across the Site, results in the provision of 3,890 bicycle parking spaces inclusive of 3,245 long-term residential bicycle parking spaces, 522 short-term residential bicycle parking spaces, and 123 non-residential bicycle parking spaces.

**TABLE 6 PROPOSED BICYCLE PARKING SUPPLY (IN AGGREGATE)**

Use / Type	Units / GFA	Type	Proposed Parking Supply (Ratio)	Proposed Parking Supply (Spaces)
Residential	3,070 units	Long-Term	1.06 spaces per unit	3,245 spaces
		Short-Term	0.17 spaces per unit	522 spaces
Non-Residential	15,490 sm GFA	Long-Term & Short-Term	0.79 spaces per 100 sm GFA	123 spaces
<b>TOTAL</b>				<b>3,890 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.

#### 5.1.2.1 Bicycle Parking Location

As part of the Site's Sustainability Strategy, the Proposed Development will meet Toronto Green Standard (TGS) Version 4 (V4). To meet the requirements, all long-term bicycle parking, within all blocks, will be provided in a secure controlled-access bicycle parking facility or purpose-built bicycle locker on the first or second storey of the building or on levels below ground commencing with the first level below ground. All short-term bicycle parking, within all blocks, will be provided in a highly visible and publicly accessible location at-grade or on the first parking level of the building below grade.

#### 5.1.2.2 Electric Bicycle Parking

As part of the Site's Sustainability Strategy, the Proposed Development will meet Toronto Green Standard (TGS) Version 4 (V4). To meet the requirements, at least 15% of required long-term bicycle parking (2,791 long-term spaces x 15% = 419 bicycle parking spaces) will include an energized outlet (120 V) adjacent to the bicycle parking space.



## 6.0 LOADING CONSIDERATIONS

### 6.1 LOADING REQUIREMENTS

#### 6.1.1 Zoning Considerations

As previously noted, the Site (and much of its surrounding area) is not under the jurisdiction of current comprehensive City of Toronto Zoning By-law 569-2013; it is under the jurisdiction of former City of Toronto Zoning By-law 438-86. The minimum loading requirements of contemporary City of Toronto Zoning By-law 569-2013 are also considered within this section.

The library and community facilities in Block 2F (and 2G) have been conceptually shown in the plans. The programming and design of the facilities will be refined as part of a separate application process. Therefore, consideration of a future loading facility is not included in this report and as part of this application.

#### 6.1.2 Minimum Loading Requirements – ZBL 438-86

The minimum loading requirements for each development block in Phases 4 and 5, based upon Zoning By-law 438-86, is outlined in **Table 7**.

Application of the minimum loading requirements associated with Zoning By-law 438-86 results in a total requirement of 8 loading spaces for the entire Site after the application of sharing factors.



**TABLE 7 BUILDING 1A ZONING BY-LAW 438-86 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	202 units	0 spaces	0 spaces	0 spaces	1 spaces	1 spaces
Community	1,358 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
Office	237 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	711 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Section 4.8(d).
2. Shared Loading Calculations based on Section 4.8(e).

**TABLE 8 BUILDINGS 1B AND 1C ZONING BY-LAW 438-86 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	713 units	0 spaces	0 spaces	0 spaces	1 spaces	1 spaces
Community	0 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Office	330 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	989 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Section 4.8(d).
2. Shared Loading Calculations based on Section 4.8(e).

**TABLE 9 BUILDINGS 2D AND 2E ZONING BY-LAW 438-86 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	505 units	0 spaces	0 spaces	0 spaces	1 spaces	1 spaces
Community	0 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Office	511 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
Retail	1,532 sm GFA	0 spaces	2 spaces	0 spaces	0 spaces	2 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>2 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>3 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Section 4.8(d).
2. Shared Loading Calculations based on Section 4.8(e).

**TABLE 10 BUILDINGS 2H AND 2I ZONING BY-LAW 438-86 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	333 units	0 spaces	0 spaces	0 spaces	1 spaces	1 spaces
Community	572 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
Office	376 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	1,127 sm GFA	0 spaces	2 spaces	0 spaces	0 spaces	2 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>2 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>3 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Section 4.8(d).
2. Shared Loading Calculations based on Section 4.8(e).

**TABLE 11 BUILDINGS 3J AND 3K ZONING BY-LAW 438-86 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	657 units	0 spaces	0 spaces	0 spaces	1 spaces	1 spaces
Community	0 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Office	270 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	809 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Section 4.8(d).
2. Shared Loading Calculations based on Section 4.8(e).

**TABLE 12 BUILDINGS 4L AND 4M ZONING BY-LAW 438-86 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	659 units	0 spaces	0 spaces	0 spaces	1 spaces	1 spaces
Community	916 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
Office	199 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	596 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Section 4.8(d).
2. Shared Loading Calculations based on Section 4.8(e).



### 6.1.3 Minimum Loading Requirements – ZBL 569-2013

The minimum loading requirements for each development block in Phases 4 and 5, based upon Zoning By-law 569-2013, is outlined in **Table 13**.

Application of the minimum loading requirements associated with Zoning By-law 569-2013 results in a total requirement of 10 loading spaces after the application of sharing factors.

**TABLE 13 BUILDING 1A ZONING BY-LAW 569-2013 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	202 units	0 spaces	0 spaces	0 spaces	1 spaces	1 spaces
Community	1,358 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
Office	237 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	711 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Chapter 220.5.10.1(9).
3. Shared Loading Calculations based on Section 40.10.90.1.

**TABLE 14 BUILDINGS 1B AND 1C ZONING BY-LAW 569-2013 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	713 units	0 spaces	0 spaces	1 spaces	1 spaces	2 spaces
Community	0 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Office	330 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	989 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>3 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Chapter 220.5.10.1(9).
3. Shared Loading Calculations based on Section 40.10.90.1.

**TABLE 15 BUILDINGS 2D AND 2E ZONING BY-LAW 569-2013 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	505 units	0 spaces	0 spaces	1 spaces	1 spaces	2 spaces
Community	0 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Office	511 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
Retail	1,532 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>3 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Chapter 220.5.10.1(9).
3. Shared Loading Calculations based on Section 40.10.90.1.

**TABLE 16 BUILDINGS 2H AND 2I ZONING BY-LAW 569-2013 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	333 units	0 spaces	0 spaces	0 spaces	1 spaces	1 spaces
Community	572 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	0 spaces
Office	376 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	1 spaces
Retail	1,127 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Chapter 220.5.10.1(9).
3. Shared Loading Calculations based on Section 40.10.90.1.

**TABLE 17 BUILDINGS 3J AND 3K ZONING BY-LAW 569-2013 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	657 units	0 spaces	0 spaces	1 spaces	1 spaces	2 spaces
Community	0 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Office	270 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	809 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>3 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Chapter 220.5.10.1(9).
3. Shared Loading Calculations based on Section 40.10.90.1.





**TABLE 18 BUILDINGS 4L AND 4M ZONING BY-LAW 569-2013 MINIMUM LOADING REQUIREMENTS**

Use / Type	Units / GFA <sup>1</sup>	Type A Spaces	Type B Spaces	Type C Spaces	Type G Spaces	Total
Residential	659 units	0 spaces	0 spaces	1 spaces	1 spaces	2 spaces
Community	916 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
Office	199 sm GFA	0 spaces	0 spaces	0 spaces	0 spaces	0 spaces
Retail	596 sm GFA	0 spaces	1 spaces	0 spaces	0 spaces	1 spaces
<b>TOTAL (with sharing)<sup>2</sup></b>		<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>3 spaces</b>
<b>TOTAL (with sharing)<sup>3</sup></b>		<b>0 spaces</b>	<b>0 spaces</b>	<b>1 spaces</b>	<b>1 spaces</b>	<b>2 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022.
2. Shared Loading Calculations based on Chapter 220.5.10.1(9).
3. Shared Loading Calculations based on Section 40.10.90.1.

### 6.1.4 Proposed Loading Supply

Consolidated primary loading facilities are proposed for each development block included in the proposal with dedicated access off adjacent north-south streets (with the exception of Block 1A).

It is therefore proposed to modestly exceed the minimum loading requirements of Zoning By-law 569-2013 as outlined in **Table 19**.

**TABLE 19 PROPOSED LOADING SUPPLY BY BLOCK**

Block	Loading Supply			
	Type B	Type C	Type G	Total
Building 1A	1 spaces	-	1 space	2 spaces
Buildings 1B & 1C	1 spaces	-	1 space	2 spaces
Buildings 2D & 2E	1 spaces	-	1 space	2 spaces
Buildings 2H & 2I	1 spaces	-	1 space	2 spaces
Buildings 3J & 3K	1 spaces	-	1 space	2 spaces
Buildings 4L & 4M	1 spaces	-	1 space	2 spaces
<b>TOTAL</b>	<b>6 spaces</b>	<b>-</b>	<b>6 spaces</b>	<b>12 spaces</b>

Notes:

1. Development statistics based on architectural Site plans provided by Karakusevic Carson Architects, dated November 24, 2022. Blocks 2F & 2G are omitted.



For the entirety of Phases 4 and 5 in aggregate, a total supply of 12 loading spaces is proposed, inclusive of 6 Type B and 6 Type G loading spaces.

Vehicle manoeuvring is intended to be accommodated on-site such that vehicles can enter and exit each block in a forward motion. Vehicle manoeuvring diagrams for each block are included in **Appendix E**.

The staging area for each block has been sized to accommodate the minimum area required for the tower with the greatest number of residential units, based on City's "*Requirements For Garbage, Recycling And Organics Collection Services For New Developments and Redevelopments*" (March 2022).



## 7.0 VEHICULAR TRIP GENERATION UPDATE

Based on the updated development program, the resulting trip generation is summarized below in **Table 20**. The forecasting methodologies are analogous to those presented in the April 2022 TIS.

The Site is expected to generate in the order of 405 and 380 two-way vehicle trips during the weekday morning and afternoon peak hours, respectively, representing an increase of 10 to 15 trips in the peak hours compared to the previous submission. The impacts of these additional trips distributed across the entire Site and study area road network are expected to be minimal, and are not expected to alter the conclusions of the vehicular traffic analysis.

**TABLE 20 VEHICULAR SITE TRIP GENERATION**

Use	Parameter	AM Peak Hour			PM Peak Hour		
		In	Out	2-Way	In	Out	2-Way
<b>Current Submission</b>							
Residential	<i>Selected Person Trip Rate (trips / unit)</i>	0.05	0.45	0.50	0.30	0.15	0.45
	Person Site Trips (3,070 residential units)	155	1380	1535	920	460	1380
	Auto Site Trips (22% of total persons)	35	305	340	205	100	305
Non-Residential	<i>Selected Vehicle Trip Rate (trips / spaces)</i>	0.40	0.20	0.60	0.25	0.40	0.65
	Auto Site Trips (113 non-residential spaces)	45	20	65	35	40	75
<b>Total</b>	<b>Auto Site Trips</b>	<b>80</b>	<b>325</b>	<b>405</b>	<b>240</b>	<b>140</b>	<b>380</b>
<b>Previous Submission</b>							
Residential	Auto Site Trips	35	295	330	195	100	295
Non-Residential	Auto Site Trips	45	20	65	30	40	70
<b>Total</b>	<b>Auto Site Trips</b>	<b>80</b>	<b>315</b>	<b>395</b>	<b>225</b>	<b>140</b>	<b>365</b>
<b>Differences</b>							
Residential	Auto Site Trips	0	+10	+10	+10	0	+10
Non-Residential	Auto Site Trips	0	0	0	+5	0	+5
<b>Total</b>	<b>Auto Site Trips</b>	<b>0</b>	<b>+10</b>	<b>+10</b>	<b>+15</b>	<b>0</b>	<b>+15</b>

Notes:

1. All trips are rounded to the nearest five (5).

## 8.0 TRAFFIC OPERATIONS ANALYSIS UPDATE

Given the minor increase in trip generation discussed in **Section 7.0**, the traffic volume forecasts and analysis methodology are consistent with those published in the April 2022 TIS. The updated analysis in the sections below are based on the changes to the proposed road network previously discussed in **Section 3.0**.

### 8.1 ANALYSIS SCENARIOS

The following analysis scenarios were reviewed for the weekday morning and afternoon peak hours:

1. Baseline existing traffic conditions (volumes as illustrated in **Figure 3 of Appendix F**);
2. 2032 Future background traffic conditions on the existing road network (volumes as illustrated in **Figure 6 of Appendix F**);
3. 2032 Future background traffic conditions on the proposed road network (volumes as illustrated in **Figure 7 of Appendix F**);
4. 2032 Future total traffic conditions on the proposed road network (volumes as illustrated in **Figure 12 of Appendix F**).

Future analysis scenarios are updated to incorporate the current plan that features two additional north-south linkages extending between Oak Street and Gerrard Street East: a private street between Dreamers Way and Sackville Street and the extension of Tubman Avenue. These new linkages create or modify the following intersections:

- **Oak Street / New Private Street** – a new 3-way intersection would be created. It has been assumed for this analysis that the New Private Street Driveway operates under STOP control. Two-way flow has been considered for the New Private Street.
- **Gerrard Street East / New Private Street** - a new 3-way intersection would be created. It has been assumed that the New Private Street would operate under STOP control. Two-way flow has been considered for the New Private Street.
- **Tubman Avenue / Sword Street / Gerrard Street East** – the existing intersection would be updated to become a 4-way intersection. The south leg would be the Tubman Avenue extension and operate one-way southbound. It is assumed that left turns from Gerrard Street East to Tubman Avenue, and no southbound through moves from Sword Street to Tubman Avenue would be restricted during peak periods.
- **Tubman Avenue / Oak Street** – the existing 3-way intersection would be updated to become a 4-way intersection. The north leg would be the proposed Tubman Avenue extension. It has been assumed that the southbound Tubman Avenue approach to the intersection would operate under STOP control.

All updated lane configurations and traffic volume figures are provided in **Appendix F**. All Synchro worksheets for the four scenarios above are provided in **Appendix G**.

## 8.2 SIGNALIZED INTERSECTION ANALYSIS

### 8.2.1 Parliament Street / Gerrard Street East

The intersection of Parliament Street / Gerrard Street East operates under signal control with variable cycle lengths in both peak hours due to the implementation of transit signal priority and phase extensions.

**Table 21** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.62 and 0.59 during the weekday morning and afternoon peak hours, respectively.

Under all future traffic conditions with the proposed road network, the intersection will continue to operate under capacity at overall v/c ratios of 0.72 and 0.67 or better during the weekday morning and afternoon peak hours, respectively.

Based on the foregoing, no mitigation measures or improvements are recommended at this intersection.

**TABLE 21 PARLIAMENT STREET / GERRARD STREET EAST TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBLTR	0.28 (0.53)	B (C)	0.29 (0.56)	B (C)	0.29 (0.56)	B (C)	0.29 (0.58)	B (C)
WBLTR	0.68 (0.37)	C (C)	0.70 (0.40)	C (C)	0.69 (0.42)	C (C)	0.74 (0.44)	C (C)
NBLTR	0.40 (0.44)	B (B)	0.46 (0.50)	B (B)	0.46 (0.50)	B (B)	0.52 (0.54)	C (B)
SBLTR	0.39 (0.54)	B (B)	0.41 (0.56)	B (B)	0.41 (0.56)	B (B)	0.41 (0.58)	B (B)
<b>Overall</b>	<b>0.62 (0.59)</b>	<b>C (B)</b>	<b>0.67 (0.63)</b>	<b>C (C)</b>	<b>0.66 (0.64)</b>	<b>C (C)</b>	<b>0.72 (0.67)</b>	<b>C (C)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).

## 8.2.2 Gerrard Street East / Sackville Street

The intersection of Gerrard Street East / Sackville Street operates under signal control with variable cycle lengths in both peak hours due to the implementation of transit signal priority and phase extensions.

**Table 22** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.43 and 0.50 during the weekday morning and afternoon peak hours, respectively.

Under all future traffic conditions with the proposed road network, the intersection will continue to operate under capacity at overall v/c ratios of 0.53 and 0.49 or better during the weekday morning and afternoon peak hours, respectively.

Based on the foregoing, no other mitigation measures or improvements are recommended at this intersection, aside from the one-way configuration along Sackville Street.

**TABLE 22 GERRARD STREET EAST / SACKVILLE STREET TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBTR	0.19 (0.48)	A (A)	0.19 (0.48)	A (A)	0.18 (0.48)	A (A)	0.18 (0.51)	A (A)
WBLT	0.49 (0.32)	A (A)	0.50 (0.34)	A (A)	0.55 (0.37)	A (A)	0.61 (0.51)	A (A)
NBLTR	0.02 (0.02)	B (B)	0.02 (0.03)	B (B)	--(2)			
SBLTR	0.26 (0.48)	C (B)	0.27 (0.48)	C (B)	0.25 (0.39)	C (B)	0.26 (0.40)	C (C)
<b>Overall</b>	<b>0.43 (0.50)</b>	<b>A (A)</b>	<b>0.44 (0.50)</b>	<b>A (A)</b>	<b>0.48 (0.47)</b>	<b>A (A)</b>	<b>0.53 (0.49)</b>	<b>A (A)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).
2. Sackville Street is proposed to be a one-way, southbound-only travel corridor.

### 8.2.3 Gerrard Street East / Sumach Street

The intersection of Gerrard Street East / Sumach Street operates under signal control with variable cycle lengths in both peak hours due to the implementation of transit signal priority and phase extensions.

**Table 23** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.52 and 0.46 during the weekday morning and afternoon peak hours, respectively.

Under all future traffic conditions with the proposed road network, the intersection will continue to operate under capacity at overall v/c ratios of 0.64 and 0.57 or better during the weekday morning and afternoon peak hours, respectively.

Based on the foregoing, no other mitigation measures or improvements are recommended at this intersection, aside from the one-way configuration along Sumach Street.

**TABLE 23 GERRARD STREET EAST / SUMACH STREET TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBLTR	0.20 (0.45)	A (A)	0.20 (0.48)	A (A)	--(2)			
EBLT	Lane group does not currently exist.				0.19 (0.47)	A (A)	0.21 (0.49)	A (A)
WBLTR	0.53 (0.30)	A (A)	0.53 (0.32)	A (A)	--(2)			
WBTR	Lane group does not currently exist.				0.47 (0.29)	A (A)	0.53 (0.31)	A (A)
NBLTR	0.16 (0.25)	D (D)	0.32 (0.28)	D (D)	0.39 (0.37)	D (C)	0.74 (0.62)	D (D)
<b>Overall</b>	<b>0.52 (0.46)</b>	<b>A (A)</b>	<b>0.55 (0.49)</b>	<b>A (A)</b>	<b>0.50 (0.49)</b>	<b>A (A)</b>	<b>0.64 (0.57)</b>	<b>B (A)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).
2. Sumach Street is proposed to be a one-way, northbound-only travel corridor.



#### 8.2.4 Gerrard Street East / River Street

The intersection of Gerrard Street East / River Street operates under signal control with variable cycle lengths in both peak hours due to the implementation of transit signal priority and phase extensions.

**Table 24** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.79 and 0.96 during the weekday morning and afternoon peak hours, respectively. The eastbound movement operates at near capacity conditions in the weekday afternoon peak hour.

Under future background traffic conditions with the proposed road network, most of the intersection's movements operate under capacity at v/c ratios between 0.14 to 0.89. The eastbound movements will begin to operate over capacity at a v/c ratio of 1.04 during the weekday afternoon peak hours.

With the addition of Site traffic, most of the intersection's movements continue to operate under capacity. The eastbound movements will continue to operate over capacity at overall v/c ratios of 1.07 during the weekday afternoon peak hours. Site-related traffic impact is minimal on the eastbound lane group and in the order of 3%.

Signal timing optimization is recommended to accommodate the eastbound vehicle movements at the intersection. As post-pandemic travel conditions continue to evolve and adjust to more consistent traffic patterns, the operations at this intersection should continue to be monitored.



**TABLE 24 GERRARD STREET EAST / RIVER STREET TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBLTR	0.49 (0.98)	B (D)	0.47 (1.04)	B (E)	0.46 (1.04)	B (E)	0.85 (1.07)	B (E)
WBLTR	0.86 (0.64)	C (C)	0.82 (0.73)	B (C)	0.82 (0.74)	B (C)	0.85 (0.75)	C (C)
NBL	0.12 (0.27)	B (B)	0.17 (0.32)	B (C)	0.17 (0.32)	B (C)	0.17 (0.41)	B (C)
NBTR	0.42 (0.64)	B (C)	0.61 (0.69)	C (C)	0.61 (0.69)	C (C)	0.62 (0.69)	C (C)
SBL	0.37 (0.81)	B (D)	0.55 (0.89)	C (E)	0.56 (0.89)	C (E)	0.56 (0.89)	C (E)
SBT	0.41 (0.50)	B (C)	0.49 (0.59)	B (C)	0.49 (0.59)	B (C)	0.49 (0.61)	B (C)
SBR	0.17 (0.14)	B (B)	0.18 (0.14)	B (B)	0.18 (0.14)	B (B)	0.19 (0.15)	B (B)
<b>Overall</b>	<b>0.79 (0.96)</b>	<b>B (C)</b>	<b>0.85 (1.02)</b>	<b>B (D)</b>	<b>0.86 (1.02)</b>	<b>B (D)</b>	<b>0.88 (1.04)</b>	<b>B (D)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).



## 8.2.5 Parliament Street / Dundas Street East

The intersection of Parliament Street / Dundas Street East operates under signal control with cycle lengths of 76 seconds in both peak hours.

**Table 25** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.35 and 0.44 during the weekday morning and afternoon peak hours, respectively.

Under all future traffic conditions with the proposed road network, the intersection will continue to operate under capacity at overall v/c ratios of 0.44 and 0.51 or better during the weekday morning and afternoon peak hours, respectively.

Based on the foregoing, no mitigation measures or improvements are recommended at this intersection.

**TABLE 25 PARLIAMENT STREET / DUNDAS STREET EAST TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBLTR	0.22 (0.39)	A (B)	0.25 (0.45)	A (B)	0.24 (0.45)	A (B)	0.25 (0.45)	A (B)
WBLTR	0.24 (0.22)	B (B)	0.30 (0.26)	B (B)	0.30 (0.25)	B (B)	0.34 (0.26)	B (B)
NBLTR	0.43 (0.53)	C (C)	0.45 (0.58)	C (C)	0.45 (0.58)	C (C)	0.44 (0.61)	C (C)
SBLTR	0.54 (0.42)	C (C)	0.57 (0.48)	C (C)	0.58 (0.50)	C (C)	0.60 (0.51)	C (C)
<b>Overall</b>	<b>0.35 (0.44)</b>	<b>B (B)</b>	<b>0.40 (0.50)</b>	<b>B (B)</b>	<b>0.40 (0.50)</b>	<b>B (B)</b>	<b>0.44 (0.51)</b>	<b>B (B)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).

## 8.2.6 Dundas Street East / Sackville Street

The intersection of Dundas Street East / Sackville Street operates under signal control with cycle lengths of 76 seconds in both peak hours.

**Table 26** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.32 and 0.23 during the weekday morning and afternoon peak hours, respectively.

Under all future traffic conditions with the proposed road network, the intersection will continue to operate under capacity at overall v/c ratios of 0.47 and 0.27 or better during the weekday morning and afternoon peak hours, respectively.

Based on the foregoing, no mitigation measures or improvements are recommended at this intersection.

**TABLE 26 DUNDAS STREET EAST / SACKVILLE STREET TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBTR	0.20 (0.23)	A (A)	0.22 (0.27)	A (A)	0.23 (0.27)	A (A)	0.23 (0.27)	A (A)
WBLT	0.33 (0.24)	A (A)	0.40 (0.27)	A (A)	0.40 (0.26)	A (A)	0.41 (0.26)	A (A)
NBLTR	0.13 (0.14)	C (C)	0.13 (0.14)	C (C)	0.13 (0.14)	C (C)	0.13 (0.14)	C (C)
SBLTR	0.29 (0.23)	C (C)	0.29 (0.23)	C (C)	0.49 (0.26)	C (C)	0.59 (0.26)	C (C)
<b>Overall</b>	<b>0.32 (0.23)</b>	<b>A (A)</b>	<b>0.37 (0.26)</b>	<b>A (A)</b>	<b>0.42 (0.27)</b>	<b>A (A)</b>	<b>0.47 (0.27)</b>	<b>B (A)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).

## 8.2.7 Dundas Street East / Sumach Street

The intersection of Dundas Street East / Sumach Street operates under signal control with variable cycle lengths in both peak hours due to the implementation of transit signal priority and phase extensions.

**Table 27** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.35 and 0.43 during the weekday morning and afternoon peak hours, respectively.

Under all future traffic conditions with the proposed road network, the intersection will continue to operate under capacity at overall v/c ratios of 0.46 and 0.51 or better during the weekday morning and afternoon peak hours, respectively.

Based on the foregoing, no mitigation measures or improvements are recommended at this intersection.

**TABLE 27 DUNDAS STREET EAST / SUMACH STREET TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBLTR	0.34 (0.41)	A (A)	0.42 (0.50)	A (A)	0.42 (0.51)	A (A)	0.44 (0.51)	A (A)
WBLTR	0.33 (0.26)	A (A)	0.45 (0.33)	A (A)	0.41 (0.34)	A (A)	0.47 (0.45)	A (A)
NBLTR	0.33 (0.43)	C (C)	0.23 (0.44)	C (C)	0.35 (0.45)	D (C)	0.36 (0.45)	D (C)
SBLTR	0.25 (0.12)	C (C)	0.35 (0.17)	C (C)	0.15 (0.09)	C (C)	0.27 (0.12)	D (C)
<b>Overall</b>	<b>0.35 (0.43)</b>	<b>A (B)</b>	<b>0.43 (0.50)</b>	<b>B (B)</b>	<b>0.42 (0.51)</b>	<b>A (B)</b>	<b>0.46 (0.51)</b>	<b>A (B)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).

## 8.2.8 Dundas Street East / River Street

The intersection of Dundas Street East / River Street operates under signal control with variable cycle lengths in both peak hours due to the implementation of transit signal priority and phase extensions. **Table 28** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.72 and 0.75 during the weekday morning and afternoon peak hours, respectively.

Under future background traffic conditions with the proposed road network, the intersection continues to operate under capacity at overall v/c ratios of 0.95 and 0.79 during the weekday morning and afternoon peak hours, respectively. However, the northbound shared through-right movement will operate near capacity at a v/c ratio of 0.99 during the weekday morning peak hours.

With the addition of Site traffic, most of the intersection's movements continue to operate under capacity. The northbound shared through-right movement will operate at capacity (i.e. v/c ratio of 1.00) during the weekday morning peak hour. Site-related traffic impact is minimal on this movement and in the order of 1%.

As post-pandemic travel conditions continue to evolve and adjust to more consistent traffic patterns, the operations at this intersection should continue to be monitored prior to any improvements such as signal timing optimization.

**TABLE 28 DUNDAS STREET EAST / RIVER STREET TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBTLR	0.56 (0.59)	B (B)	0.80 (0.71)	C (C)	0.82 (0.71)	C (C)	0.89 (0.73)	D (C)
WBTLR	0.58 (0.56)	B (B)	0.86 (0.73)	C (C)	0.86 (0.73)	C (C)	0.90 (0.81)	D (C)
NBL	0.26 (0.06)	C (B)	0.44 (0.22)	C (B)	0.46 (0.22)	C (B)	0.72 (0.41)	D (B)
NBTR	0.71 (0.86)	C (D)	0.99 (0.78)	E (C)	0.99 (0.78)	E (C)	1.00 (0.79)	E (C)
SBL	0.59 (0.40)	C (B)	0.61 (0.61)	C (C)	0.56 (0.61)	C (C)	0.79 (0.71)	D (C)
SBTR	0.86 (0.56)	D (B)	0.78 (0.72)	C (C)	0.80 (0.72)	C (C)	0.93 (0.77)	D (C)
<b>Overall</b>	<b>0.72 (0.75)</b>	<b>C (C)</b>	<b>0.95 (0.79)</b>	<b>D (C)</b>	<b>0.95 (0.79)</b>	<b>D (C)</b>	<b>1.00 (0.84)</b>	<b>D (C)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).

## 8.2.9 Dundas Street East / Regent Park Boulevard

The intersection of Dundas Street East / Regent Park Boulevard operates under signal control with cycle lengths of 76 seconds in both peak hours.

**Table 29** summarizes the results of the traffic operations analysis at the intersection.

Under existing traffic conditions, the intersection operates under capacity at overall v/c ratios of 0.28 and 0.34 during the weekday morning and afternoon peak hours, respectively.

Under all future traffic conditions with the proposed road network, the intersection will continue to operate under capacity at overall v/c ratios of 0.37 and 0.41 or better during the weekday morning and afternoon peak hours, respectively.

Based on the foregoing, no mitigation measures or improvements are recommended at this intersection.

**TABLE 29 DUNDAS STREET EAST / REGENT PARK BOULEVARD TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	V/C	LOS	V/C	LOS	V/C	LOS	V/C	LOS
EBTR	0.32 (0.38)	A (A)	0.36 (0.46)	A (A)	0.39 (0.46)	A (A)	0.41 (0.46)	A (A)
WBLT	0.28 (0.21)	A (A)	0.34 (0.24)	A (A)	0.33 (0.23)	A (A)	0.33 (0.23)	A (A)
<b>Overall</b>	<b>0.28 (0.34)</b>	<b>A (A)</b>	<b>0.31 (0.40)</b>	<b>A (A)</b>	<b>0.35 (0.41)</b>	<b>A (A)</b>	<b>0.37 (0.41)</b>	<b>A (A)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).

### 8.3 UNSIGNALIZED INTERSECTION ANALYSIS

**Table 30** summarizes the analysis results for unsignalized intersections in the study area.

Under future traffic conditions, most movements operate acceptably in both peak hours at levels-of-service (LOS) E or better.

In the analysis of baseline existing traffic volumes, the shared eastbound movement at River Street / Oak Street indicates delays of up to 86 seconds in the weekday afternoon peak hour. Under future background conditions, these delays are expected to increase to 97 seconds. With the redevelopment of the Site, drivers are expected to experience similar delays as compared to the future background condition.

Signal warrant analysis at the intersections of River Street / Oak Street in addition to the intersection of Parliament Street / Oak Street is further discussed in **Section 8.4**.



**TABLE 30 UNSIGNALIZED INTERSECTION TRAFFIC OPERATIONS**

Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)
Parliament Street / Gerrard Street East (North Intersection)								
NBLT	A (A)	0.3 (0.7)	A (A)	0.3 (0.7)	A (A)	0.3 (0.7)	A (A)	0.3 (0.7)
Gerrard Street East / Gifford Street								
EBLT	A (A)	0.4 (0.4)	A (A)	0.4 (0.4)	A (A)	0.4 (0.4)	A (A)	0.5 (0.5)
SBLR	C (C)	18.1 (22.7)	C (C)	18.6 (21.9)	C (C)	19.1 (21.1)	C (C)	19.2 (22.3)
Gerrard Street East / Nasmith Avenue								
EBLT	A (A)	0.4 (0.2)	A (A)	0.4 (0.2)	A (A)	0.5 (0.2)	A (A)	0.5 (0.2)
SBLR	C (C)	19.4 (15.4)	C (C)	20.2 (15.2)	C (B)	20.6 (15.0)	C (C)	20.9 (15.8)
Gerrard Street East / Sword Street								
SBLR	C (C)	18.7 (18.5)	C (C)	19.3 (17.1)	C (C)	22.5 (20.0)	C (C)	21.9 (21.1)
Parliament Street / Oak Street <sup>(5)</sup>								
WBLR	C (C)	23.8 (17.2)	D (C)	30.7 (23.9)	D (C)	30.8 (24.1)	E (D)	49.3 (26.6)
SBLT	A (A)	1.1 (2.2)	A (A)	1.1 (2.2)	A (A)	1.1 (2.2)	A (A)	0.4 (1.4)
Oak Street / Dreamer's Way								
SBLR	A (A)	9.2 (9.2)	A (A)	9.6 (9.4)	A (A)	9.6 (9.4)	B (A)	10.2 (9.5)
Oak Street / Regent Street								
NBLR	A (A)	9.4 (9.5)	A (A)	9.4 (9.6)	A (A)	9.4 (9.6)	A (A)	9.7 (9.6)
Oak Street / Sackville Street								
EBLTR	A (A)	7.5 (7.7)	A (A)	7.5 (7.8)	-- <sup>(2)</sup>			
EBTR	Lane group does not currently exist.				A (A)	7.7 (7.8)	A (A)	8.1 (8.0)
WBLT <sup>(4)</sup>	A (A)	7.2 (7.6)	A (A)	7.3 (7.6)	A (A)	7.5 (7.4)	A (A)	8.0 (7.5)
SBLTR	A (A)	7.8 (7.7)	A (A)	7.8 (7.8)	A (A)	8.4 (8.1)	A (A)	10.0 (8.6)
Oak Street / Sumach Street								
EBLTR	A (A)	7.5 (8.0)	A (A)	7.6 (8.0)	A (A)	7.8 (8.2)	A (A)	8.2 (8.7)
NBLTR	A (A)	7.5 (7.8)	A (A)	7.9 (7.9)	A (A)	7.8 (8.0)	A (A)	8.5 (9.2)
SBLTR	A (A)	7.7 (7.7)	A (A)	7.8 (7.8)	-- <sup>(3)</sup>			
Oak Street / Tubman Avenue								
SBLT	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	9.7 (9.7)	A (A)	9.8 (9.8)
River Street / Oak Street <sup>(5)</sup>								
EBLTR	C (F)	23.7 (86.0)	D (F)	27.3 (93.5)	D (F)	28.5 (97.1)	E (F)	35.6 (96.0)
WBLTR	C (C)	16.1 (21.6)	C (C)	19.0 (24.3)	C (C)	19.0 (24.3)	C (C)	23.9 (24.5)
SBLT	A (A)	1.0 (0.9)	A (A)	1.2 (1.5)	A (A)	1.2 (1.4)	A (A)	1.1 (1.5)
Parliament Street / Cole Street								





Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)
SBLT	A (A)	1.5 (2.0)	A (A)	1.4 (1.9)	A (A)	1.4 (1.8)	A (A)	1.4 (1.8)
Dundas Street East / Regent Street								
EBLT	A (A)	0.6 (1.8)	A (A)	0.6 (1.6)	A (A)	0.6 (1.6)	A (A)	0.8 (1.6)
WBLT	A (A)	0.5 (0.4)	A (A)	0.5 (0.4)	A (A)	0.5 (0.4)	A (A)	0.5 (0.4)
NBLTR	C (D)	21.8 (26.1)	D (D)	25.2 (28.9)	D (D)	25.7 (28.9)	D (D)	26.7 (29.0)
SBLTR	C (C)	19.1 (16.1)	C (C)	22.0 (17.0)	C (C)	22.3 (16.9)	C (C)	23.3 (17.0)
Dundas Street East / Tubman Avenue								
EBLTR	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)
WBLTR	A (A)	0.2 (0.5)	A (A)	0.2 (0.4)	A (A)	0.2 (0.4)	A (A)	0.2 (0.4)
SBLTR	A (B)	0.0 (14.4)	A (C)	0.0 (16.9)	C (C)	22.3 (19.6)	D (C)	25.9 (24.6)
Gerrard Street East / Dreamer's Way								
WBTL	A (A)	0.0 (0.0)	A (A)	0.2 (0.8)	A (A)	0.2 (0.8)	A (A)	0.2 (1.1)
Sackville Street / Site Driveway								
EBLTR	B (B)	10.8 (10.6)	B (B)	10.9 (10.7)	-- (2)			
EBTR	Lane group does not currently exist.				B (A)	10.7 (9.5)	B (B)	11.1 (10.1)
WBLTR	B (A)	10.4 (9.4)	B (A)	10.4 (9.5)	-- (2)			
WBLT	Lane group does not currently exist.				B (B)	11.7 (10.3)	C (B)	15.7 (12.8)
NBLTR	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	-- (2)			
SBLTR	A (A)	0.8 (0.4)	A (A)	0.8 (0.4)	A (A)	0.8 (0.4)	A (A)	0.9 (1.7)
Sumach Street / Site Driveway								
EBLT	Lane group does not exist.						B (B)	10.5 (10.7)
WBLR	A (A)	0.0 (9.0)	A (A)	0.0 (9.1)	-- (3)			
WBR	Lane group does not exist.				A (A)	0.0 (9.0)	-- (3)	
WBTR	Lane group does not exist.						A (A)	9.3 (9.4)
NBTR	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	A (A)	0.0 (0.0)	Lane group does not exist.	
NBLTR	Lane group does not exist.						A (A)	0.0 (0.7)
SBLT	A (A)	0.6 (0.0)	A (A)	0.6 (0.0)	-- (3)			
Dreamer's Way / Site Driveway								
WBL	Intersection does not exist.						A (A)	8.7 (9.0)
SBLT							A (A)	0.0 (1.4)
River Street / Site Driveway <sup>(4)</sup>								
EBR	Intersection does not exist.						B (B)	14.1 (12.9)
Gerrard Street East / New Private Street								
NBR	Intersection does not exist.				A (B)	9.4 (10.1)	A (B)	9.5 (10.2)



Movement	Existing Road Network				Proposed Road Network			
	Existing		2032 Future Background		2032 Future Background		2032 Future Total	
	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)	LOS	Delay (s)
Oak Street / New Private Street								
EBLT	Intersection does not exist.				A (A)	0.7 (0.3)	A (A)	1.4 (0.8)
SBLR					A (A)	9.1 (0.0)	A (A)	9.5 (0.0)

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).
2. Sackville Street is proposed to be a one-way, southbound-only travel corridor.
3. Sumach Street is proposed to be a one-way, northbound-only travel corridor.
4. Intersection is proposed to be a right-in / right-out configuration.
5. A pedestrian crossover (PXO) is currently provided south and north of the intersections at River Street / Oak Street and Parliament Street / Oak Street, respectively. However, the Synchro modelling software does not have an option to model a two-way stop control with a PXO. For the purposes of analysis, the intersections are assumed to be fully two-way stop controlled.

## 8.4 SIGNAL WARRANT REVIEW

The April 2022 TIS provided a signal warrant review at the Parliament Street / Oak Street and River Street / Oak Street intersections. The signals would be located approximately 150m from the Gerrard Street East and Dundas Street East signals to the north and south of these intersections, respectively.

Based on the updated traffic volumes under future conditions, the signal warrants have been revised. They continue to be based on the methodology prescribed in Book 12 of the Ontario Traffic Manual (OTM).

### 8.4.1 Signal Warrant Results

The warrant results under future conditions are summarized in **Table 31**. Detailed warrant calculations are provided in **Appendix H**.

**TABLE 31 SIGNAL WARRANT RESULTS**

Intersection	2032 Future Background	2032 Future Total
Parliament Street / Oak Street	<ul style="list-style-type: none"> <li>Justification 2 is close to satisfied (98% of threshold)</li> </ul>	<ul style="list-style-type: none"> <li>Justification 2 is close to satisfied (99% of threshold)</li> <li>Justification 3 is satisfied</li> </ul>
River Street / Oak Street	<ul style="list-style-type: none"> <li>Justification 1 is close to satisfied (96% of threshold)</li> <li>Justification 2, 3 and 4 are satisfied</li> </ul>	<ul style="list-style-type: none"> <li>Justification 1 is close to satisfied (99% of threshold)</li> <li>Justification 2, 3 and 4 are satisfied</li> </ul>

The signal warrant results are analogous to those previously summarized in the April 2022 TIS.

Under future background traffic conditions, signals are close to warranted at the Parliament Street / Oak Street intersection, but is fully warranted at the River Street / Oak Street intersection.

Under future total traffic conditions, signals are warranted at both intersections.



The implementation of the traffic signals at these two Oak Street intersections will serve to support traffic operations in the area and urban design objectives by providing additional mid-block crossing opportunities and reducing drivers' reliance on courtesy gaps.

#### 8.4.2 Analysis of the Recommended Signals

Capacity analysis at the two proposed signals is summarized in **Table 32**.

Under future total traffic conditions, both intersections are expected to operate under capacity at overall v/c ratios between 0.27 to 0.59 in the peak hours, with delays onto the major corridors of up to 28 seconds (summarized as LOS C in the table below).

**TABLE 32 PROPOSED SIGNALS' TRAFFIC OPERATIONS (YEAR 2032 FUTURE TOTAL)**

Movement	Proposed Road Network	
	V/C	LOS
Parliament Street / Oak Street		
WBLR	0.39 (0.26)	C (C)
NBTR	0.20 (0.28)	B (A)
SBTL	0.23 (0.22)	A (A)
<b>Overall</b>	<b>0.27 (0.28)</b>	<b>B (A)</b>
River Street / Oak Street		
EBTLR	0.19 (0.44)	C (C)
WBTLR	0.21 (0.11)	C (C)
NBTR	0.36 (0.44)	A (A)
SBTL	0.64 (0.64)	A (B)
<b>Overall</b>	<b>0.55 (0.59)</b>	<b>B (B)</b>

Notes:

1. XX (XX) – Weekday Morning Peak Hour (Weekday Afternoon Peak Hour).



## 9.0 SUMMARY

BA Group is retained by Toronto Community Housing Corporation (TCHC) and their development partner Deltera Inc. (Tridel) to provide urban transportation consulting services in relation to a proposed mixed-use development pertaining to the lands generally bounded by Dreamers Way in the west, Gerrard Street East in the north, River Street in the east, and Oak Street to the south (excluding 40 Oak Street) located in the City of Toronto.

The Site is part of a larger multi-phased development project known as *Regent Park Revitalization*; the proposed redevelopment pertains to Phases 4 and 5 which are the final two phases.

A transportation study was originally submitted in April of 2022. This addendum letter provides responses to the received City comments, discusses and analyzes the changes to the development plans, and provides updates to the transportation study where applicable for the purposes of the ZBA submission.

### Updated Development Plan

1. Since the April 2022 TIS, the development program has been revised. In total, the development program includes approximately 3,070 residential dwelling units (1,876 market units and 1,194 affordable units) and 15,490 m<sup>2</sup> non-residential GFA.
2. While key street design objectives remain the same as the previous submission, several road network changes listed below are now being proposed:
  - Tubman Avenue Extension: A public street extension of Tubman Avenue is proposed between Gerrard Street East in the north to Oak Street in the south. The street will have a 15.0 right-of-way, including a 4.5m one-way southbound vehicle travel lane, a 2.0m width for lay-by parking, and walkways on both sides.
  - A new north-south private street between Building 1A and 1B extending between Gerrard Street East and Oak Street is proposed. This would be 6.0m in width accommodating two-way traffic flow. The private street has been designed to provide connectivity to loading areas within Buildings 1A, 1B and the Fred Victor Building.
  - A new east-west driveway extending from Dreamers Way to the new private north-south street is also proposed. It has a width of 4.5m and is intended to operate one-way eastbound, to facilitate loading operations associated with the Fred Victor Building.

### Vehicular Parking Considerations

3. The Site is currently subject to Zoning By-law 438-86 and related site-specific zoning by-laws. In discussion with City staff, application of the minimum vehicular parking requirements associated with Zoning By-law 89-2022 would be appropriate for the Site. By-law 89-2022 results in a total Site parking requirement of 53 accessible parking spaces.

4. Vehicular parking is proposed in accordance with the following rates:

Market Housing Blocks:

- Residents: 0.40 spaces per unit
- Residential visitors: 0.06 spaces per unit (to be provided on a shared, non-exclusive basis with parking for non-residential uses)
- Non-residential uses: no dedicated parking

Affordable Housing / Municipal Blocks:

- Residents: 0.40 spaces per unit
- Residential visitors: no parking
- Non-residential uses: no parking

5. Application of the parking supply rates results in a total proposed Site parking supply of 1,341 parking spaces to be split between each block. This supply meets the requirements set forth in By-law 89-2022.

### **Bicycle Parking Considerations**

6. Application of the minimum bicycle parking requirements associated with TGS V4 Tier 1 (which are based upon Zoning By-law 569-2013) results in a total Site requirement of 3,466 bicycle parking spaces.
7. It is proposed to provide a bicycle parking supply that exceeds the requirements; in aggregate across the Site, the plans include 3,890 bicycle parking spaces inclusive of 3,245 long-term residential bicycle parking spaces, 522 short-term residential bicycle parking spaces, and 123 non-residential bicycle parking spaces.
8. As part of the Site's Sustainability Strategy, 15% of the required long-term bicycle parking supply include an energized outlet (120 V) adjacent to the bicycle parking space, this meeting TGS V4.

### **Loading Considerations**

9. Application of the minimum loading requirements associated with Zoning By-law 438-86 results in a total requirement of 8 loading spaces for the entire Site after the application of sharing factors.
10. Application of the minimum loading requirements associated with Zoning By-law 569-2013 results in a total requirement of 10 loading spaces after the application of sharing factors.
11. For the entirety of Phases 4 and 5 in aggregate, a total supply of 12 loading spaces is proposed, inclusive of 6 Type B and 6 Type G loading spaces, modestly exceeding the requirements of both by-laws.



## Trip Generation Update

12. The Site is expected to generate in the order of 405 and 380 two-way vehicle trips during the weekday morning and afternoon peak hours, respectively, representing an increase of 10 to 15 trips in the peak hours compared to the previous submission. The impacts of these additional trips distributed across the entire Site and study area road network are expected to be minimal, and are not expected to alter the conclusions of the vehicular traffic analysis.

## Traffic Operations Analysis

13. All conclusions from the traffic operations analysis are similar to those presented in the April 2022 TIS (and are re-summarized below).
14. Under all future conditions, most of the signalized intersections operate under capacity (i.e. demonstrate volume-to-capacity ratios of less than 1.00).
15. Under future background traffic conditions with the proposed road network, most of the Gerrard Street East / River Street intersection's movements operate under capacity at v/c ratios between 0.14 to 0.89. However, the eastbound movements will begin to operate over capacity at overall v/c ratios of 1.04 during the weekday afternoon peak hours. With the addition of Site traffic, most of the intersection's movements continue to operate under capacity. The eastbound movements will continue to operate over capacity at overall v/c ratios of 1.07 during the weekday afternoon peak hours. Site-related traffic impact is minimal on the eastbound lane group and in the order of 3%. Signal timing optimization is recommended to accommodate the eastbound vehicle movements at the intersection. As post-pandemic travel conditions continue to evolve and adjust to more consistent traffic patterns, the operations at this intersection should continue to be monitored.
16. Under all future conditions, most movements operate acceptably in both peak hours at levels-of-service (LOS) E or better. In the analysis of baseline existing traffic volumes, the shared eastbound movement at River Street / Oak Street indicates delays of up to 86 seconds in the weekday afternoon peak hour. Under future background conditions, these delays are expected to increase to 97 seconds. With the redevelopment of the Site, drivers are expected to experience similar delays as compared to the future background condition.

## Signal Warrant Review

17. All conclusions from the signal warrant review remain the same as those presented in the April 2022 TIS (and are re-summarized below).

18. Under future background traffic conditions, signals are close to warranted at the Parliament Street / Oak Street intersection, but is fully warranted at the River Street / Oak Street intersection.
19. Under future total traffic conditions, signals are warranted at both intersections.
20. The signals would also serve to provide additional mid-block crossing opportunities and reduce driver reliance on courtesy gaps from Oak Street onto the external area road network.
21. If implemented, both intersections are expected to operate under capacity at overall v/c ratios between 0.27 to 0.59 in the peak hours, with delays onto the major corridors of up to 28 seconds.

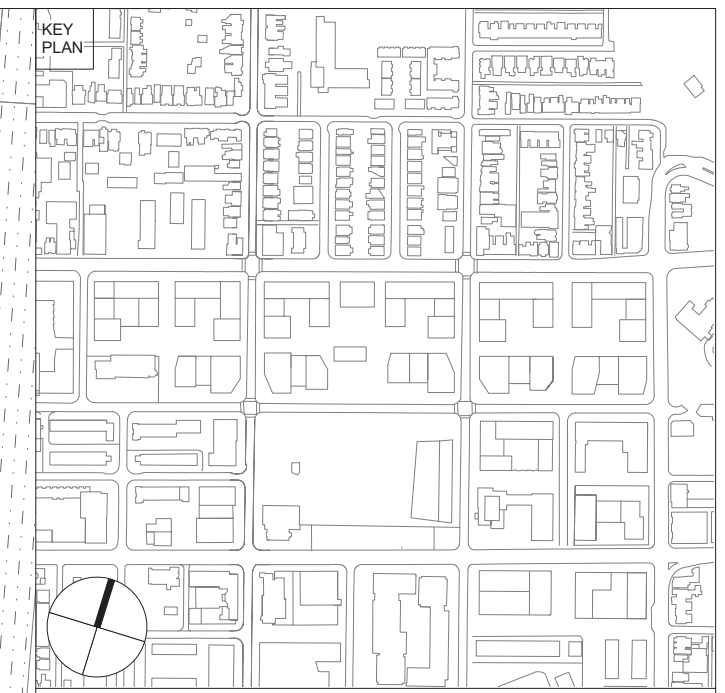
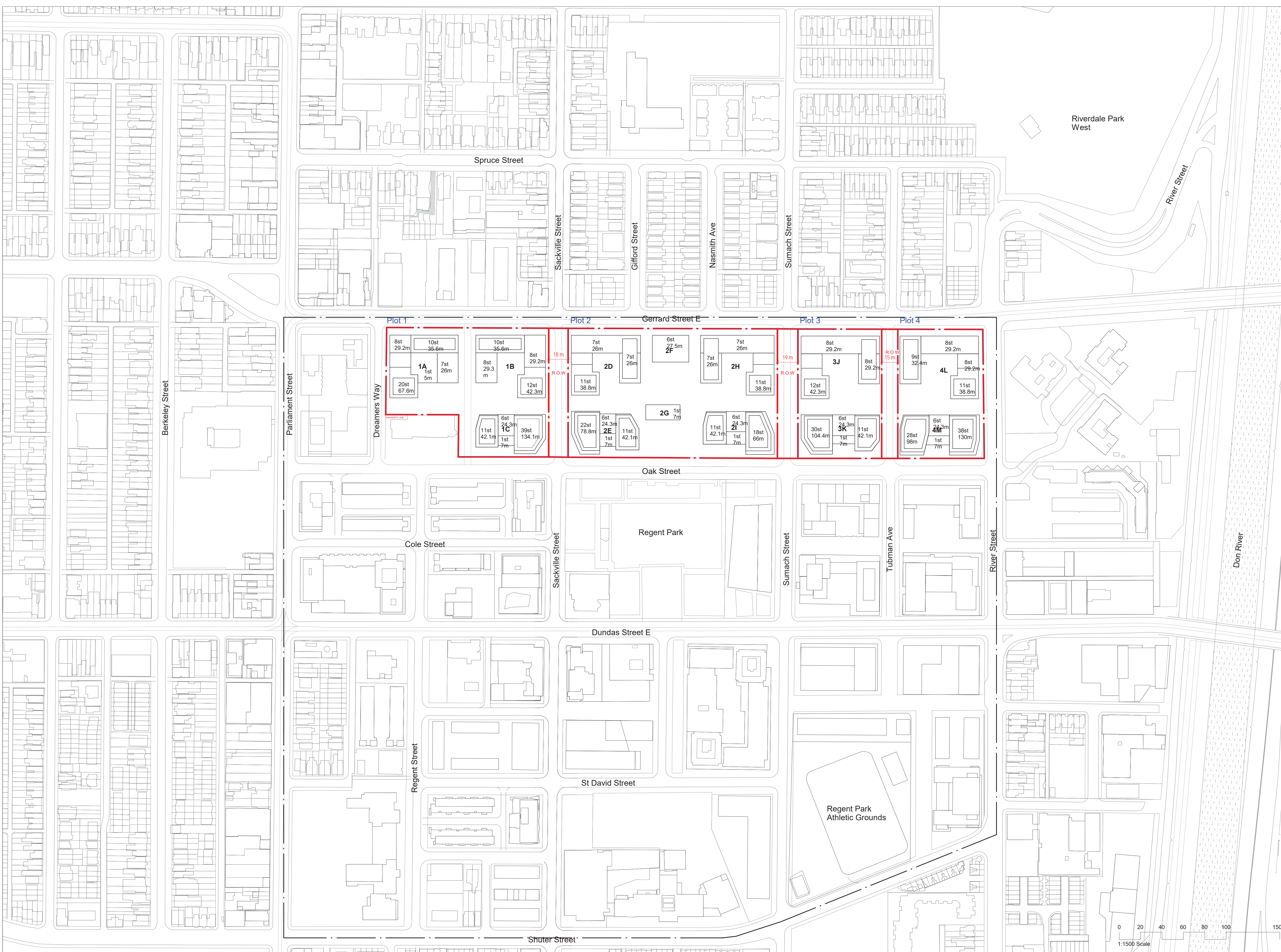
**Based on our review, the proposal is appropriate from a transportation perspective.**



## Appendix A: Reduced-Scale Architectural Site Plans







CLIENT  
**Toronto Community Housing Corporation (TCHC) and 2747199 Ontario Limited (Deltra Inc.)**

THIS DRAWING IS BASED ON DIMENSIONAL SURVEY INFORMATION PROVIDED BY OTHERS AND REPRESENTS THE ARCHITECT'S BEST CURRENT UNDERSTANDING OF EXISTING CONDITIONS. THE ARCHITECT CANNOT ACCEPT RESPONSIBILITY FOR THE ACCURACY OF THIS SURVEY INFORMATION.  
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 THESE DRAWINGS ARE NOT FOR CONSTRUCTION

- NOTES
- Regent Park Revitalization
  - Phases 4&5 Property Line
  - Plot Boundary
  - City of Toronto Owned Lands Right of Way
  - Potential Mechanical Plant

All heights measured from established grade to top of roof slab, excluding mechanical plant and parapets.

P03   S4   P2 - Issued for ZBA	11/29/22
P03   S3   P2 - Draft for Client Review	11/15/22
P01   S4   P1 - Issued for ZBA	04/14/22
P00   S3   P1 - Draft for Client Review	04/01/22

Rev. Status Reason for Issue Date

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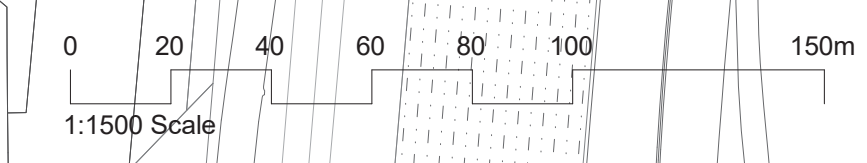


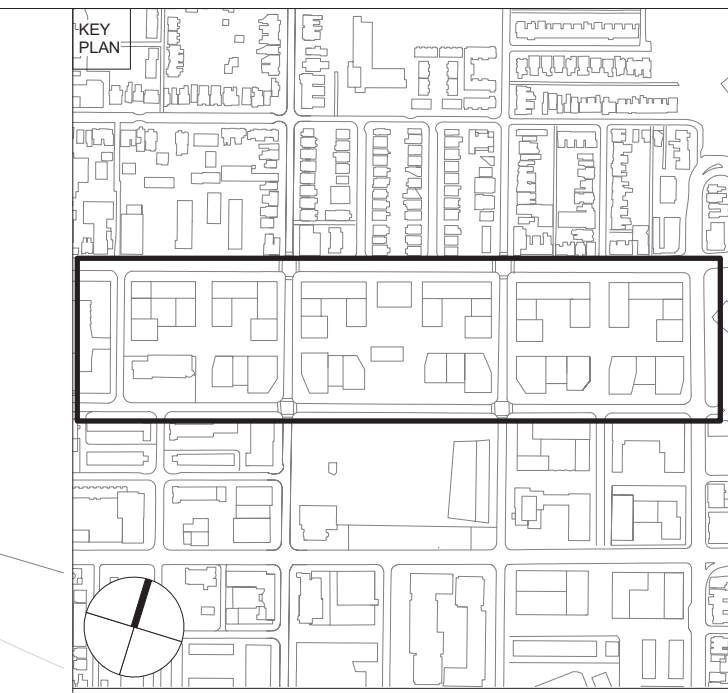
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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Context Plan**

DRAWING NUMBER <b>577 - KCA-XX-XX-DR-A - 0001</b>	REVISION <b>P03</b>
STATUS <b>S4 - Suitable for stage approval</b>	STAGE <b>P2</b>
REVISION DATE <b>11/29/22</b>	DRAWN BY <b>H/AJAN</b>
FIRST ISSUED <b>04/01/22</b>	CHECKED BY <b>PK/RM</b>
SCALE <b>1:1500 @ ARCH D</b>	KCA PROJECT NUMBER <b>577</b>





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 THESE DRAWINGS ARE NOT FOR CONSTRUCTION

- NOTES
- Plot Boundary
  - Phases 4&5 Property Line
  - City of Toronto Owned Lands
  - Potential Retention
  - Proposed Underground Parking Layout
  - Potential Mechanical Penthouse

All heights measured from established grade to penthouse, excluding mechanical plant and parapets.  
 Established grade measured above Canadian Geodetic Datum

P03	S4	P2	- Issued for ZBA	11/29/22
P02	S3	P2	- Draft for Client Review	11/15/22
P01	S4	P1	- Issued for ZBA	04/14/22
P00	S3	P1	- Draft for Client Review	04/01/22

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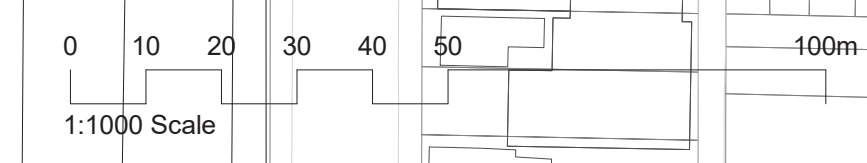


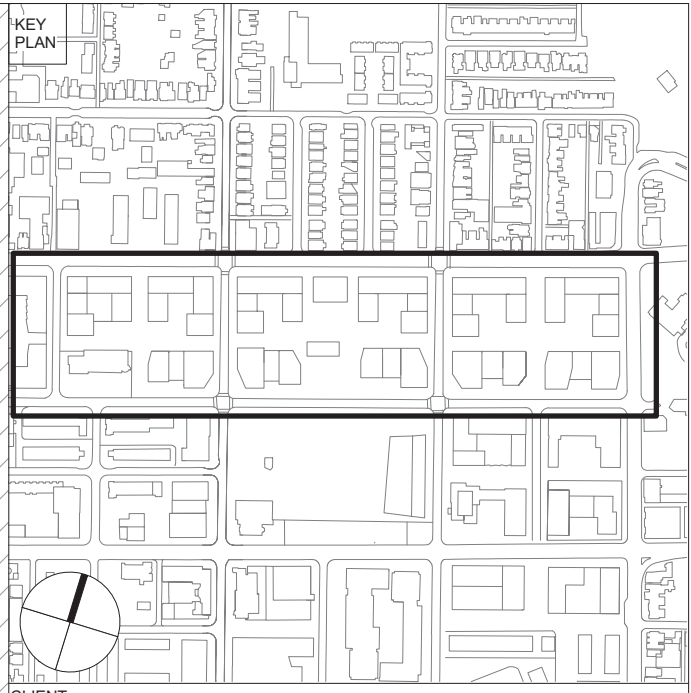
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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Site Plan**

DRAWING NUMBER	577 - KCA - XX - DR - A - 0003	REVISION	P03
STATUS	S4 - Suitable for stage approval	STAGE	P2
REVISION DATE	11/29/22	DRAWN BY	H/AJAN
FIRST ISSUED	04/01/22	CHECKED BY	PK/RM
		KCA PROJECT NUMBER	577



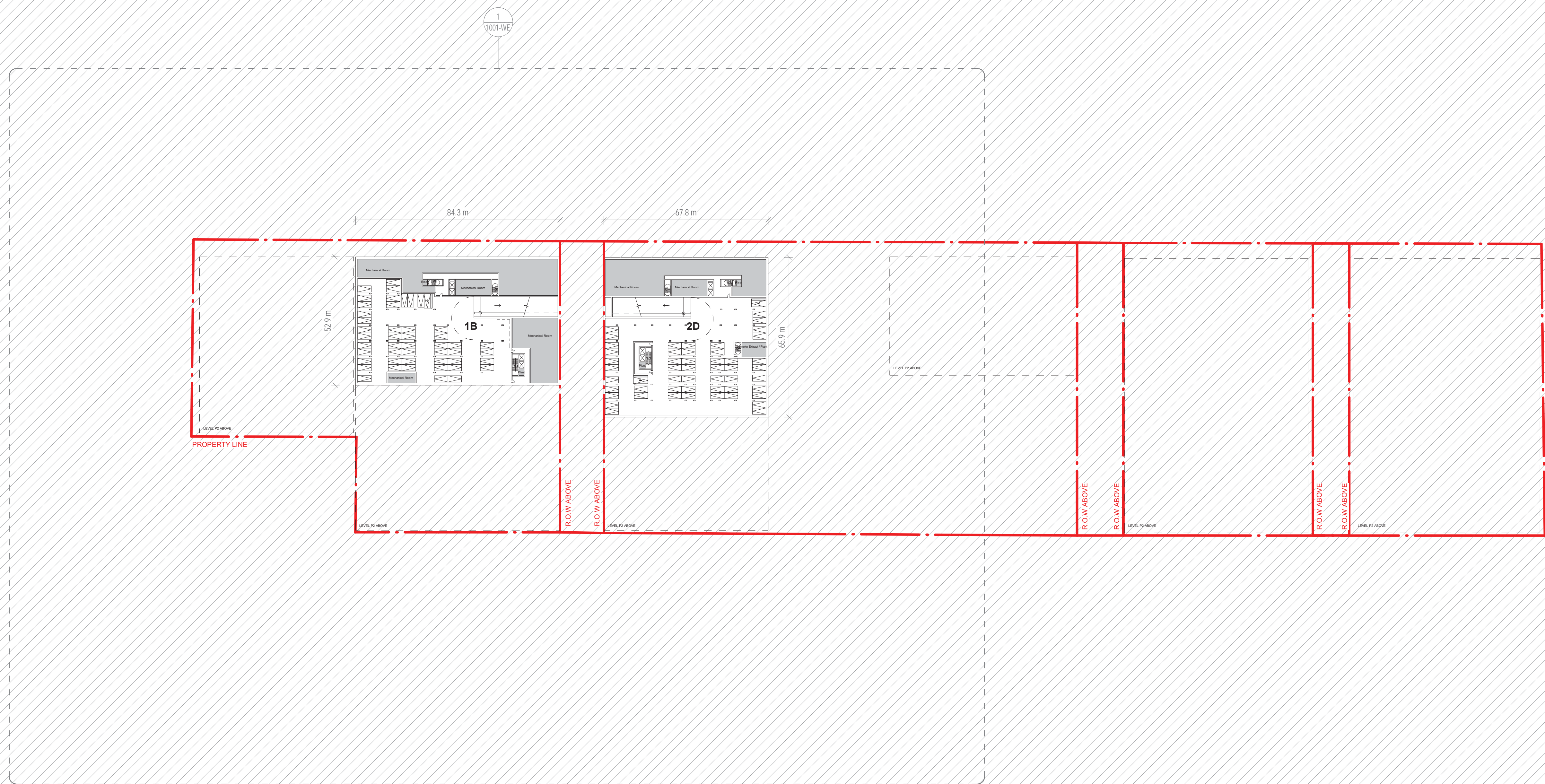


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- Ancillary



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P02	S3	P2	- Draft for Client Review	11/15/22
P01	S4	P1	- Issued for ZBA	04/14/22
P00	S3	P1	- Draft for Client Review	04/01/22

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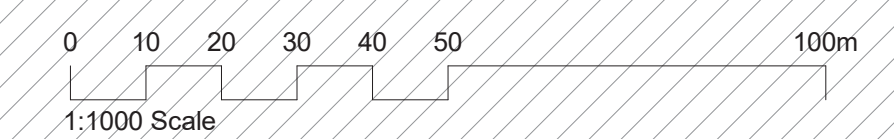


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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Level P3 Plan**

DRAWING NUMBER	REVISION	
<b>577 - KCA-XX-P3-DR-A - 1001</b>	<b>P03</b>	
STATUS	STAGE	
<b>S4 - Suitable for stage approval</b>	<b>P2</b>	
REVISION DATE	DRAWN BY	SCALE
11/29/22	H/A/J	1 : 1000 @ ARCH D
FIRST ISSUED	CHECKED BY	KCA PROJECT NUMBER
04/01/22	PK/RM	577



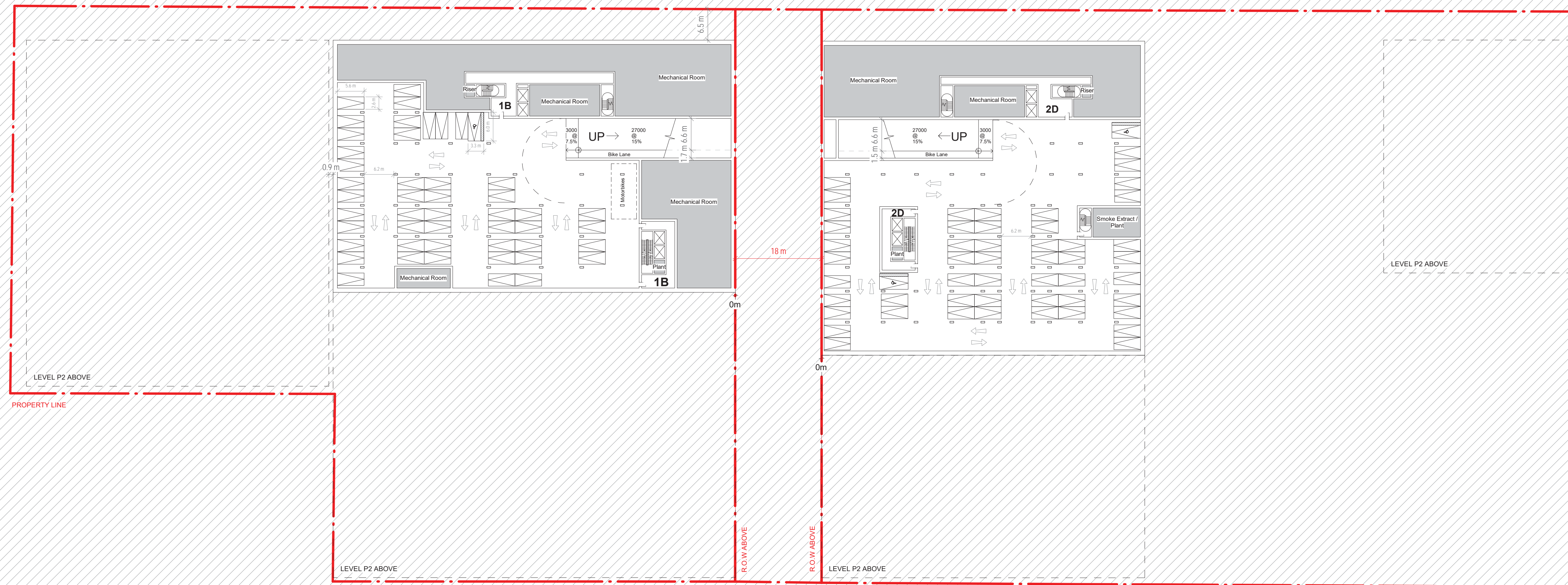


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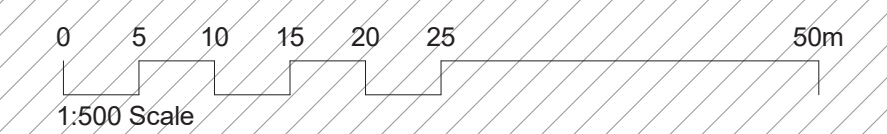


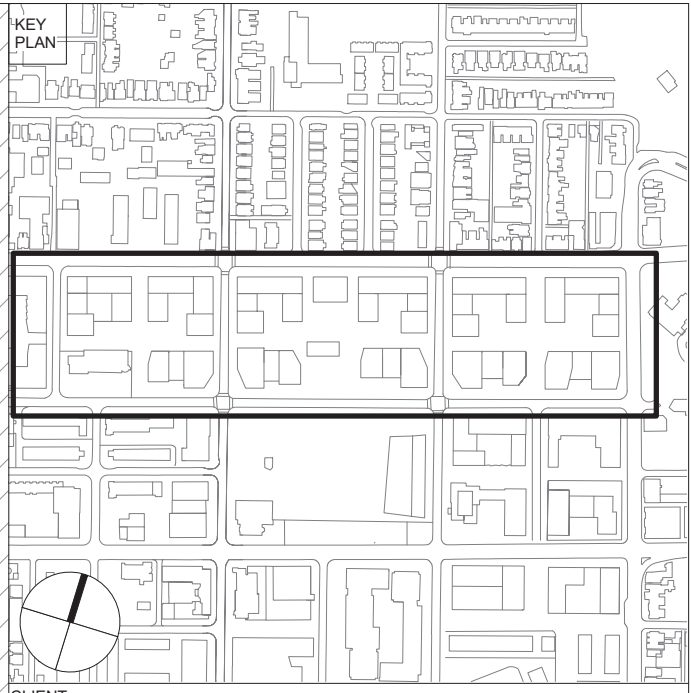
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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Level P3 Plan - West**

DRAWING NUMBER	REVISION	
577 - KCA-WE-P3-DR-A - 1001	P03	
STATUS	STAGE	
S4 - Suitable for stage approval	P2	
REVISION DATE	DRAWN BY	SCALE
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FIRST ISSUED	CHECKED BY	KCA PROJECT NUMBER
04/01/22	PK/RM	577



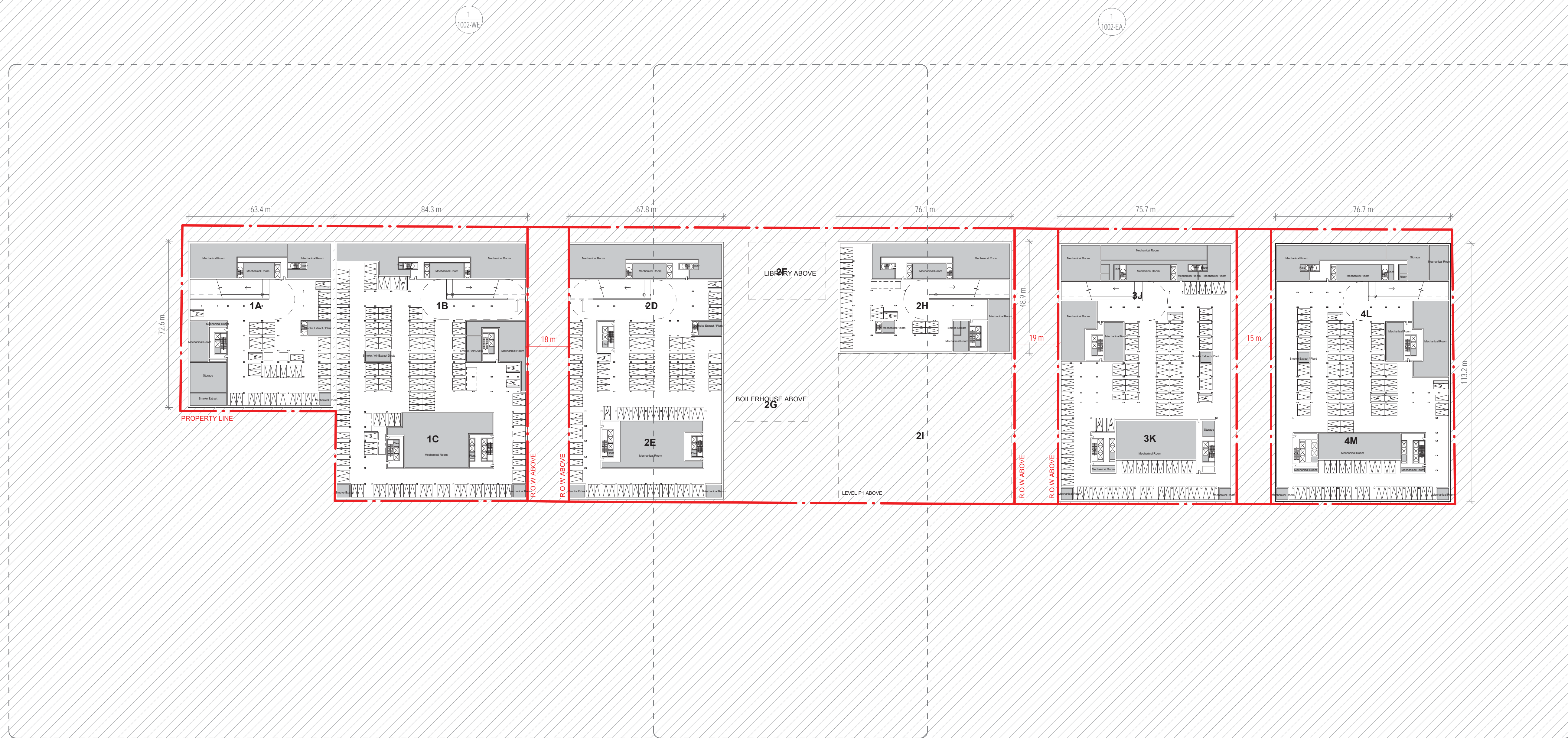


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PROJECT

**Regent Park Phases 4 & 5**

TITLE

**Level P2 Plan**

DRAWING NUMBER

**577 - KCA-XX-P2-DR-A - 1002 P03**

STATUS

**S4 - Suitable for stage approval P2**

REVISION DATE

**11/29/22**

FIRST ISSUED

**04/01/22**

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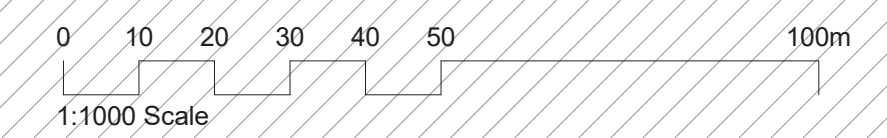
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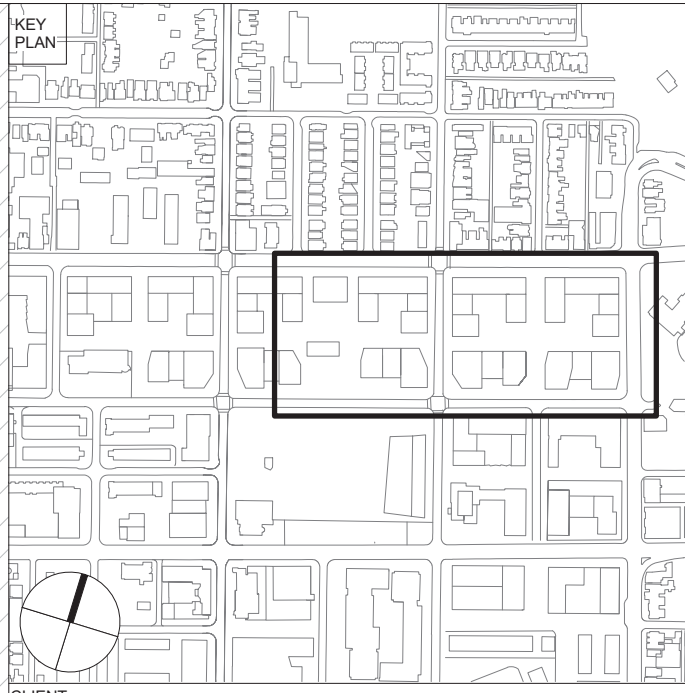
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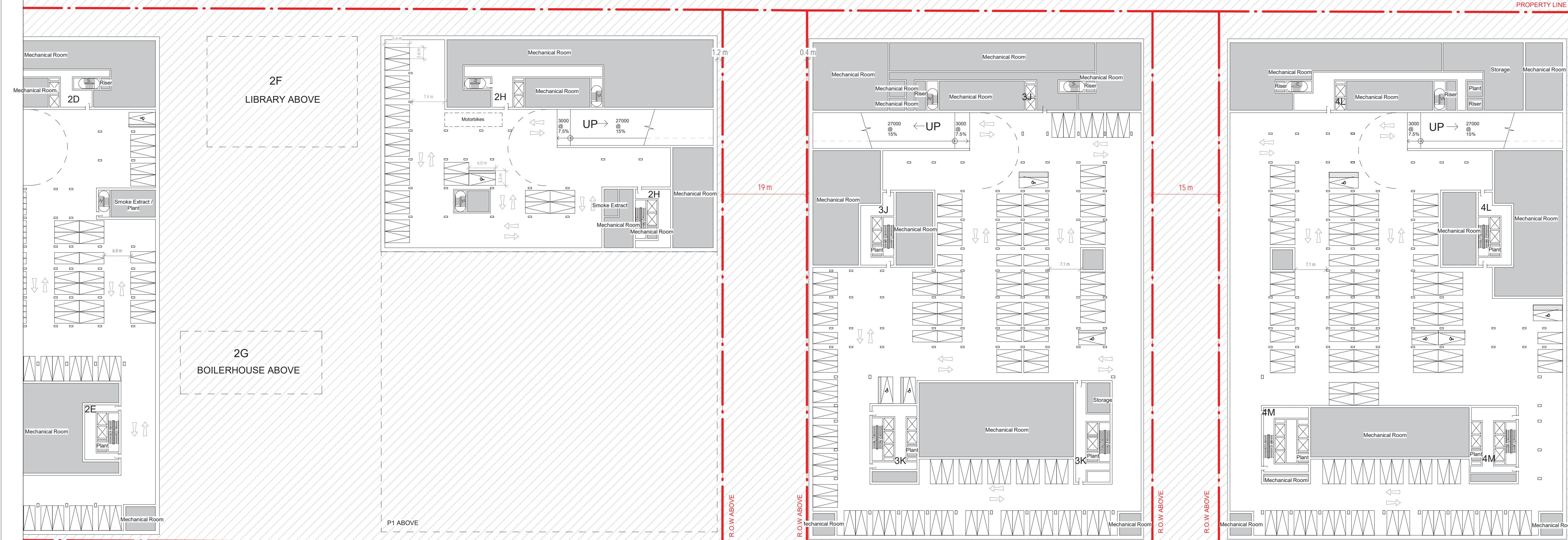
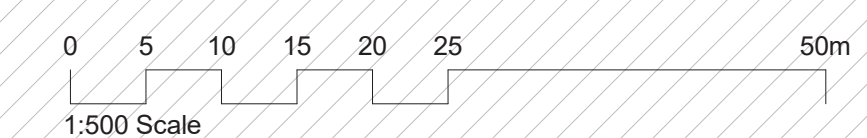


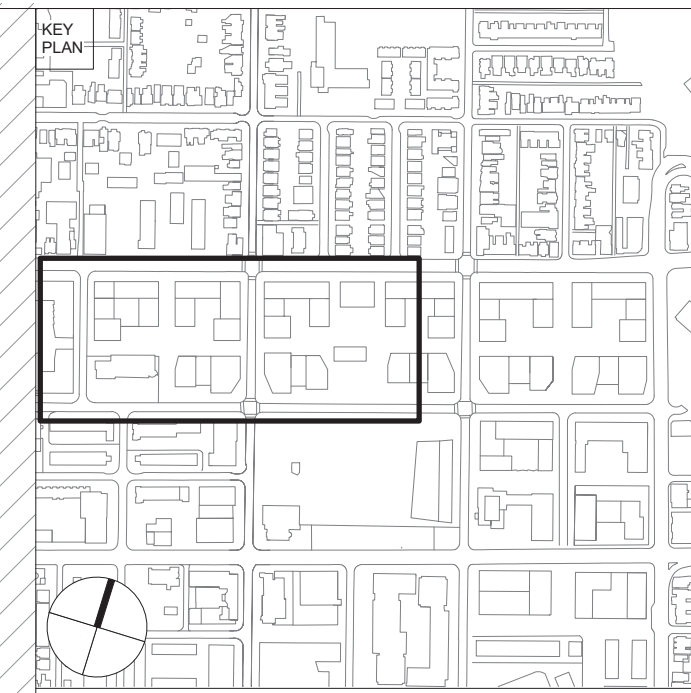
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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Level P2 Plan - East**

DRAWING NUMBER	REVISION	
577 - KCA-EA-P2-DR-A - 1002	P03	
STATUS	STAGE	
S4 - Suitable for stage approval	P2	
REVISION DATE	DRAWN BY	SCALE
11/29/22	H/AJAN	1 : 500 @ ARCH D
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04/01/22	PK/RM	577



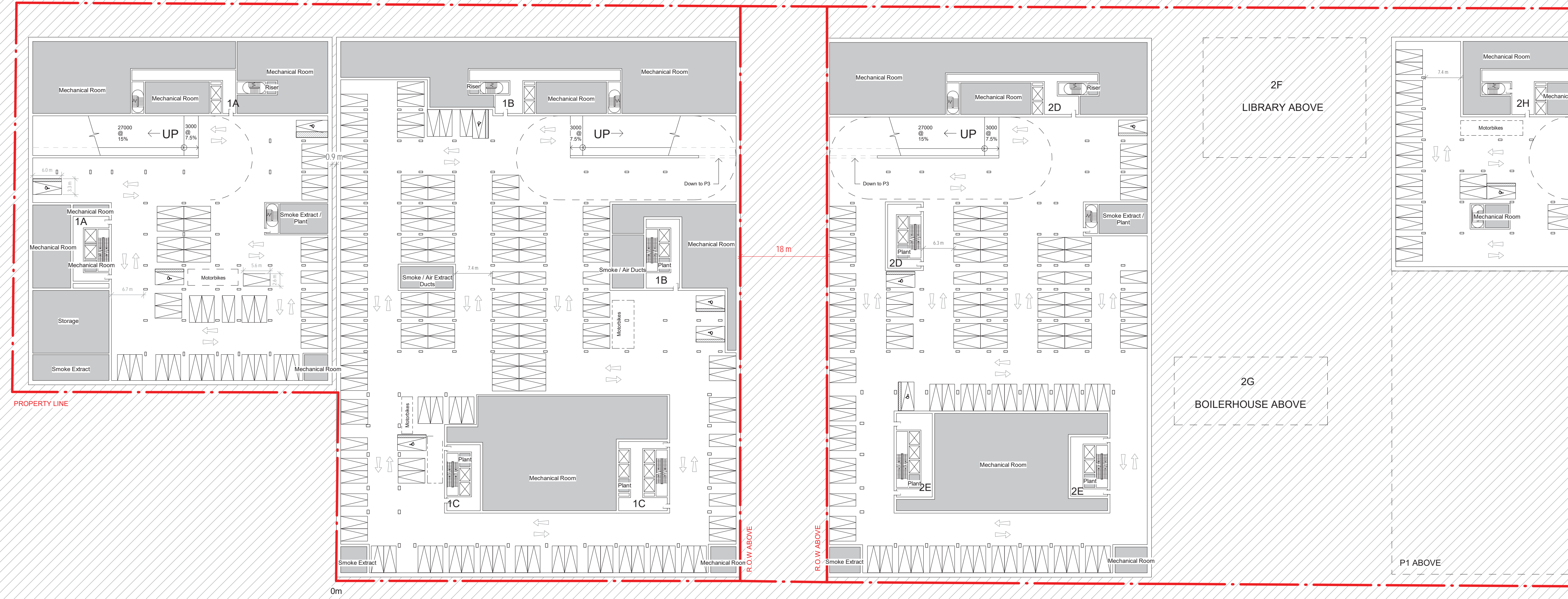


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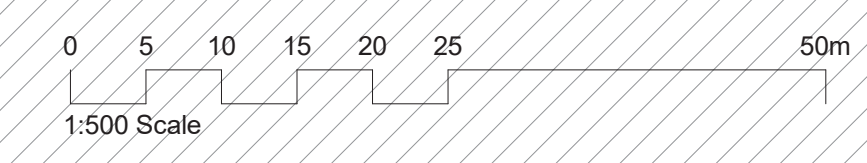


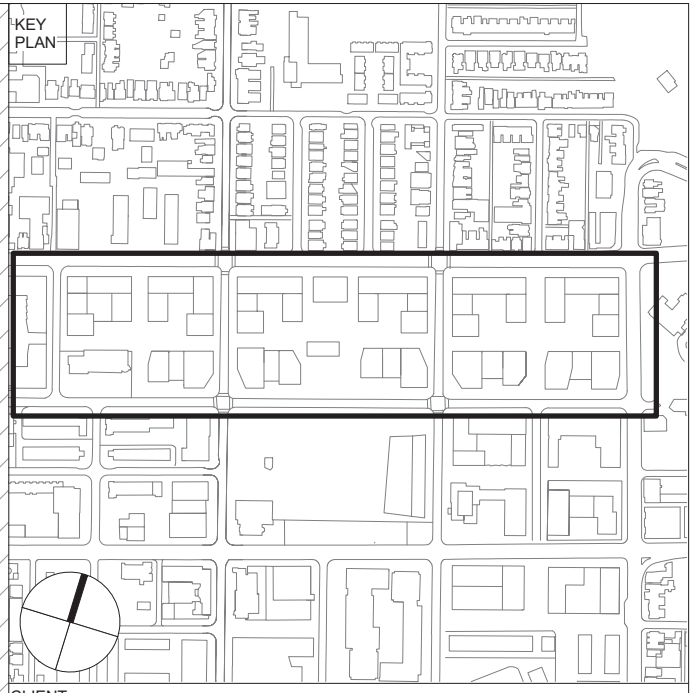
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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Level P2 Plan - West**

DRAWING NUMBER	REVISION	
577 - KCA-WE-P2-DR-A - 1002	P03	
STATUS	STAGE	
S4 - Suitable for stage approval	P2	
REVISION DATE	DRAWN BY	SCALE
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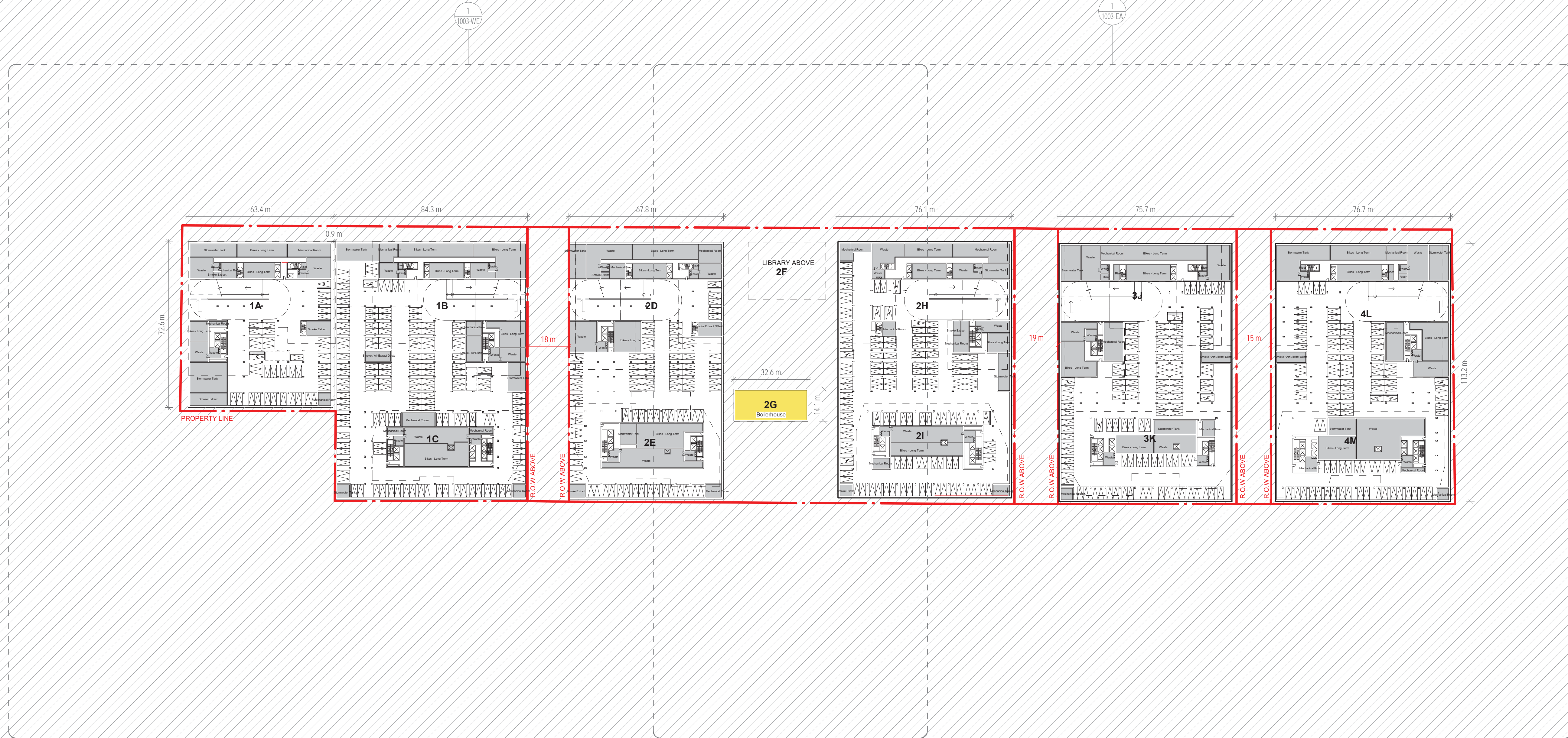


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PROJECT

**Regent Park Phases 4 & 5**

TITLE

**Level P1 Plan**

DRAWING NUMBER

**577 - KCA-XX-P1-DR-A - 1003 P03**

STATUS

**S4 - Suitable for stage approval P2**

REVISION DATE

**11/29/22**

FIRST ISSUED

**04/01/22**

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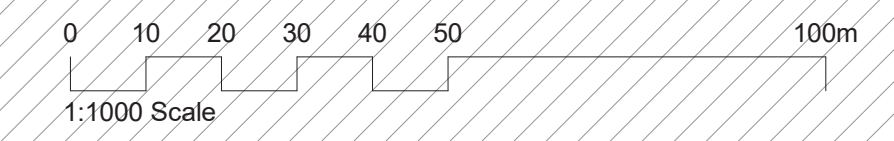
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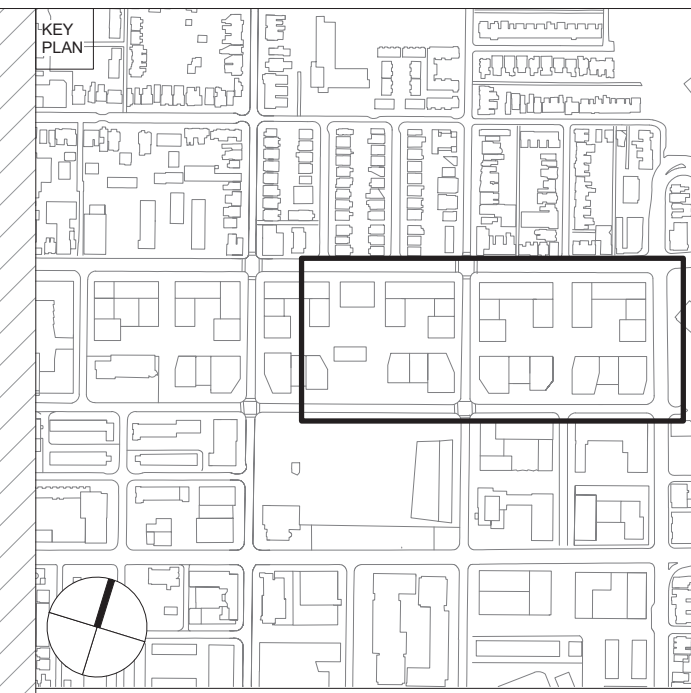
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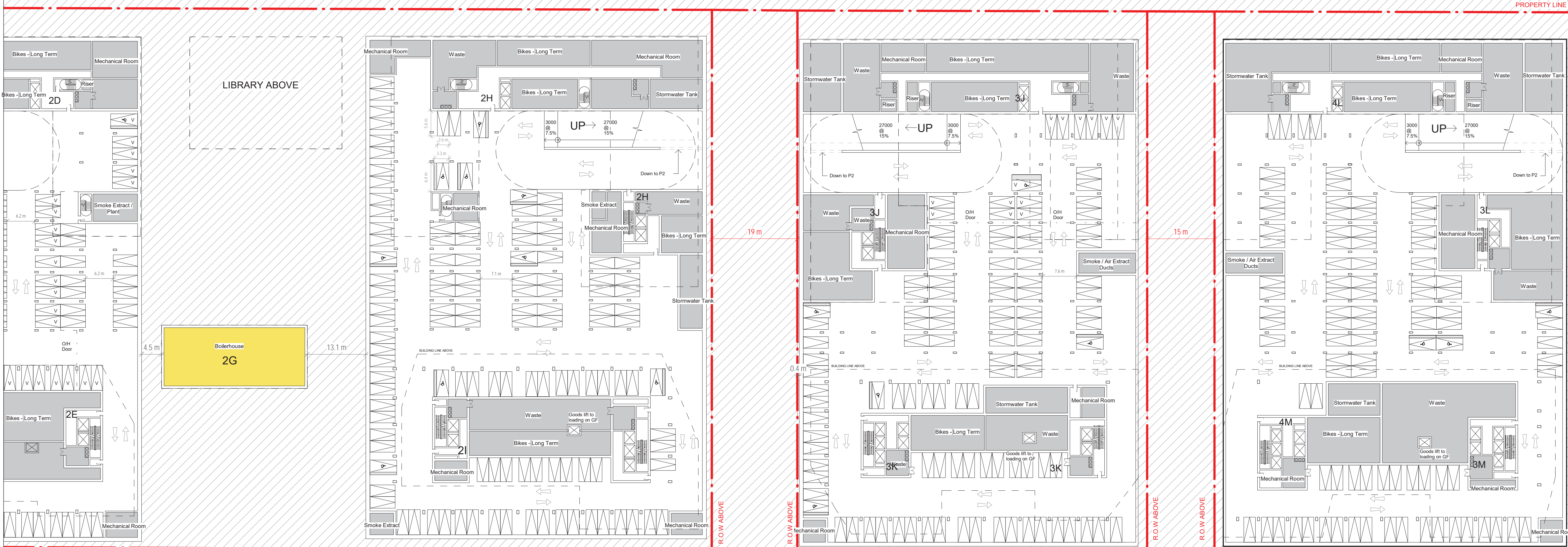


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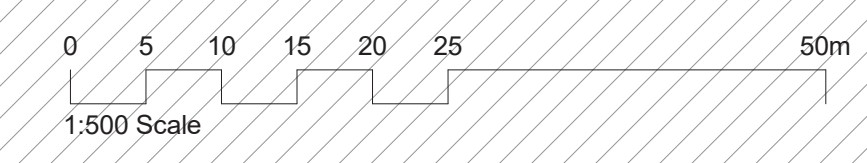


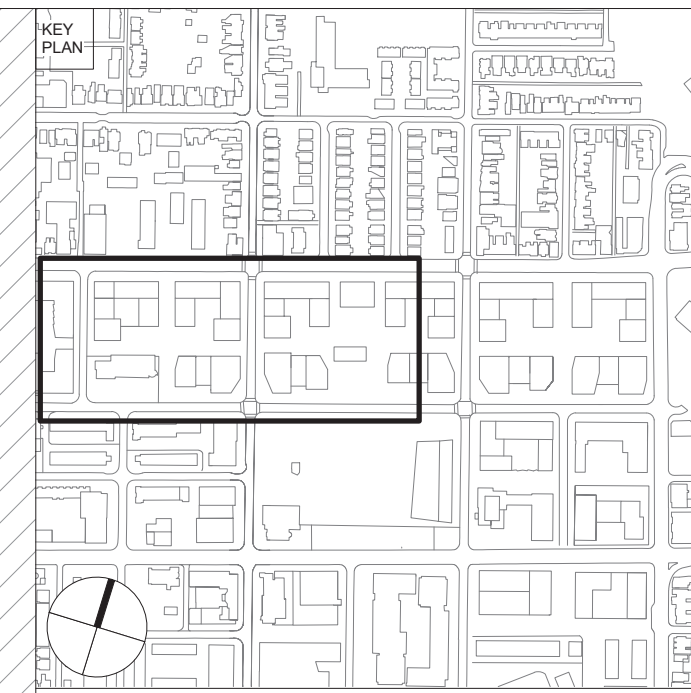
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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Level P1 Plan - East**

DRAWING NUMBER	577 - KCA-EA-P1-DR-A - 1003	REVISION	P03
STATUS	S4 - Suitable for stage approval	STAGE	P2
REVISION DATE	11/29/22	DRAWN BY	H/AJAN
FIRST ISSUED	04/01/22	CHECKED BY	PK/RM
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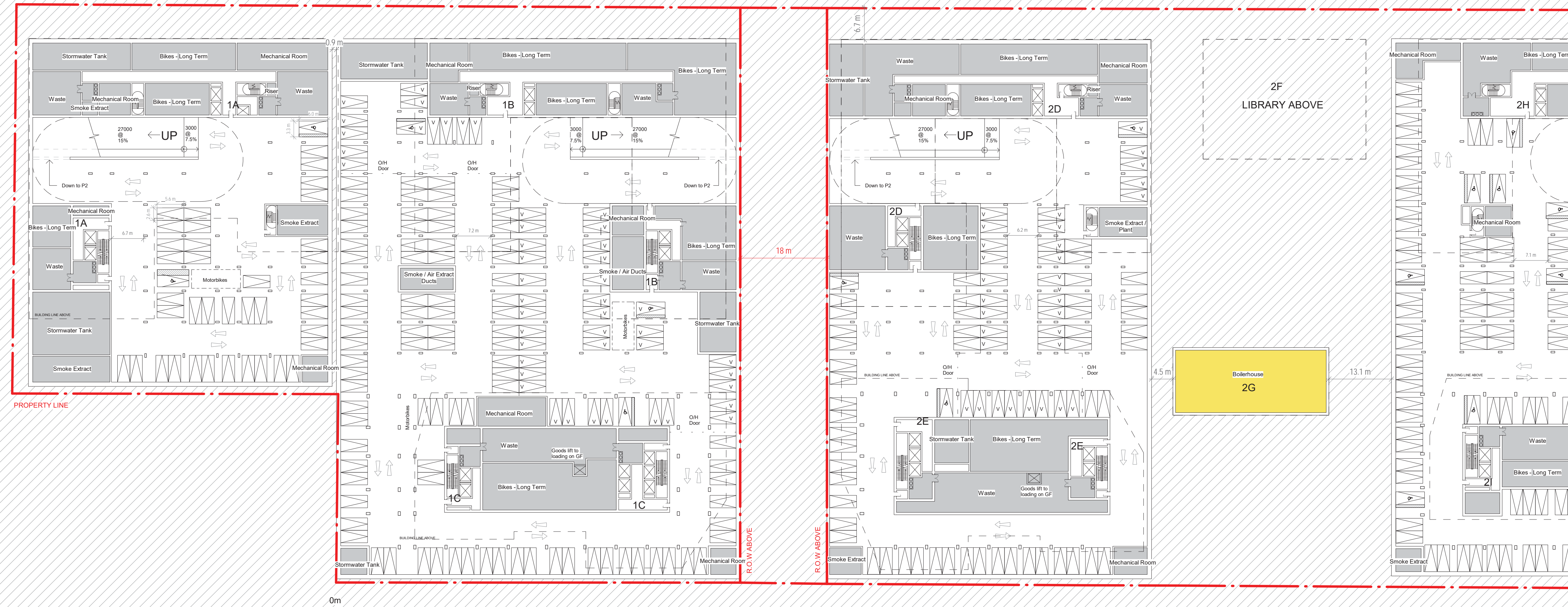


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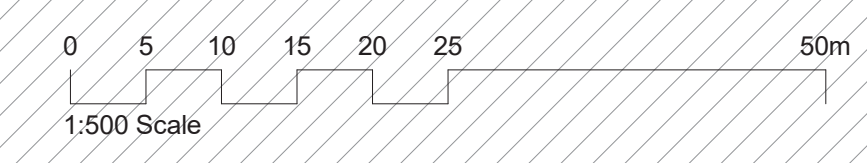


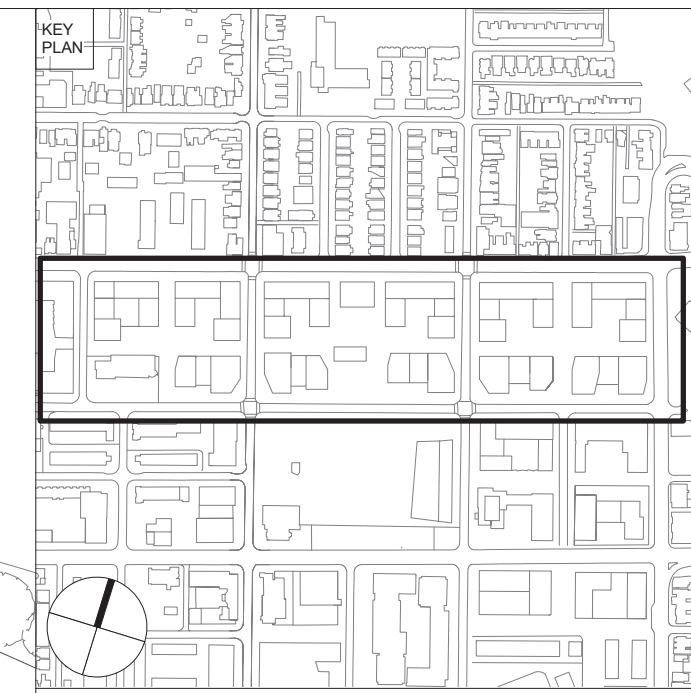
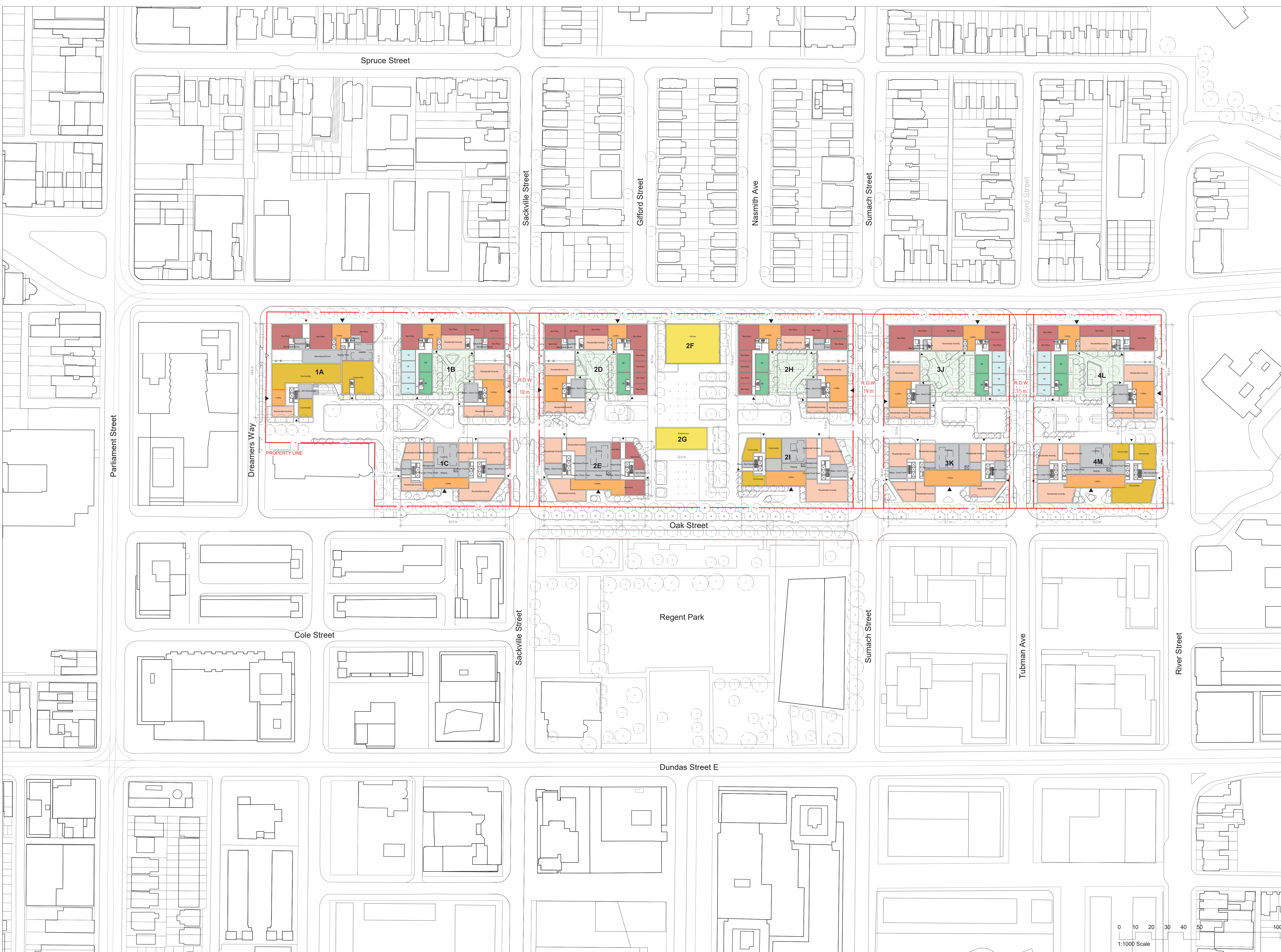
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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Level P1 Plan - West**

DRAWING NUMBER	577 - KCA-WE-P1-DR-A - 1003	REVISION	P03
STATUS	S4 - Suitable for stage approval	STAGE	P2
REVISION DATE	11/29/22	DRAWN BY	H/AJAN
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**Phases 4&5 Property Line**  
 --- City of Toronto Owned Lands Right of Way

- 1B
- 1B+
- 2B
- 2B+
- 3B
- 3B+
- 4B
- 5B
- Amenity
- Ancillary
- Civic
- Community
- Lobby
- Retail
- Residential Outdoor Amenity

Plot	Building	Level	Name	Estimated No. Bikes
1	A	Level 0	Bikes - Non Residential	95
1	B	Level 0	Bikes - Short Term	71
2	D	Level 0	Bikes - Short Term	70
2	E	Level 0	Bikes - Short Term	58
2	F	Level 0	Bikes - Non Residential	20
2	H	Level 0	Bikes - Short Term	70
2	I	Level 0	Bikes - Non Residential	19
2	J	Level 0	Bikes - Short Term	58
3	J	Level 0	Bikes - Short Term	71
3	K	Level 0	Bikes - Long Term	53
3	K	Level 0	Bikes - Short Term	98
3	L	Level 0	Bikes - Short Term	71
3	M	Level 0	Bikes - Non Residential	24
3	M	Level 0	Bikes - Long Term	45
Grand Total				799

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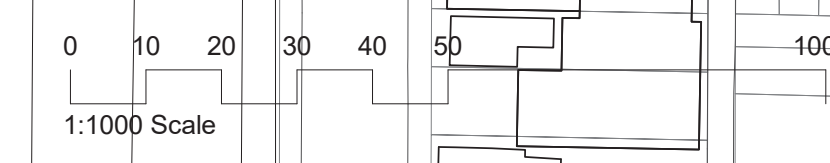


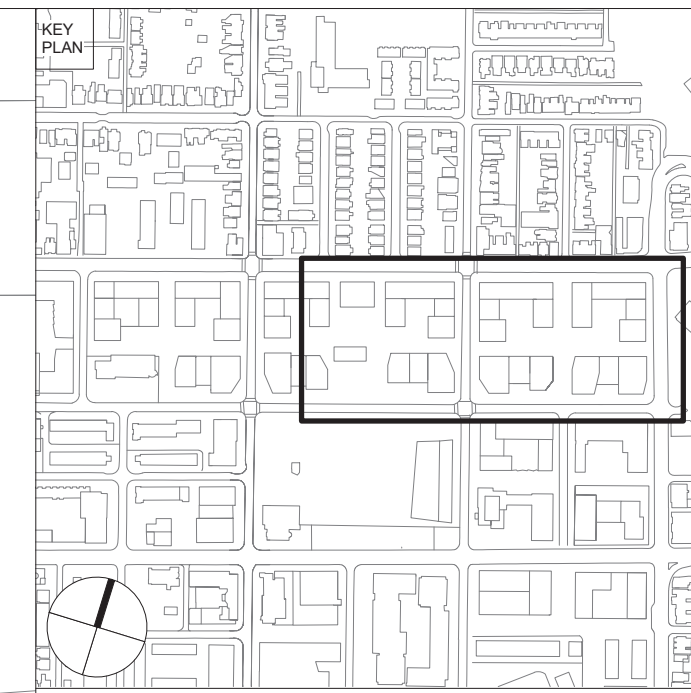
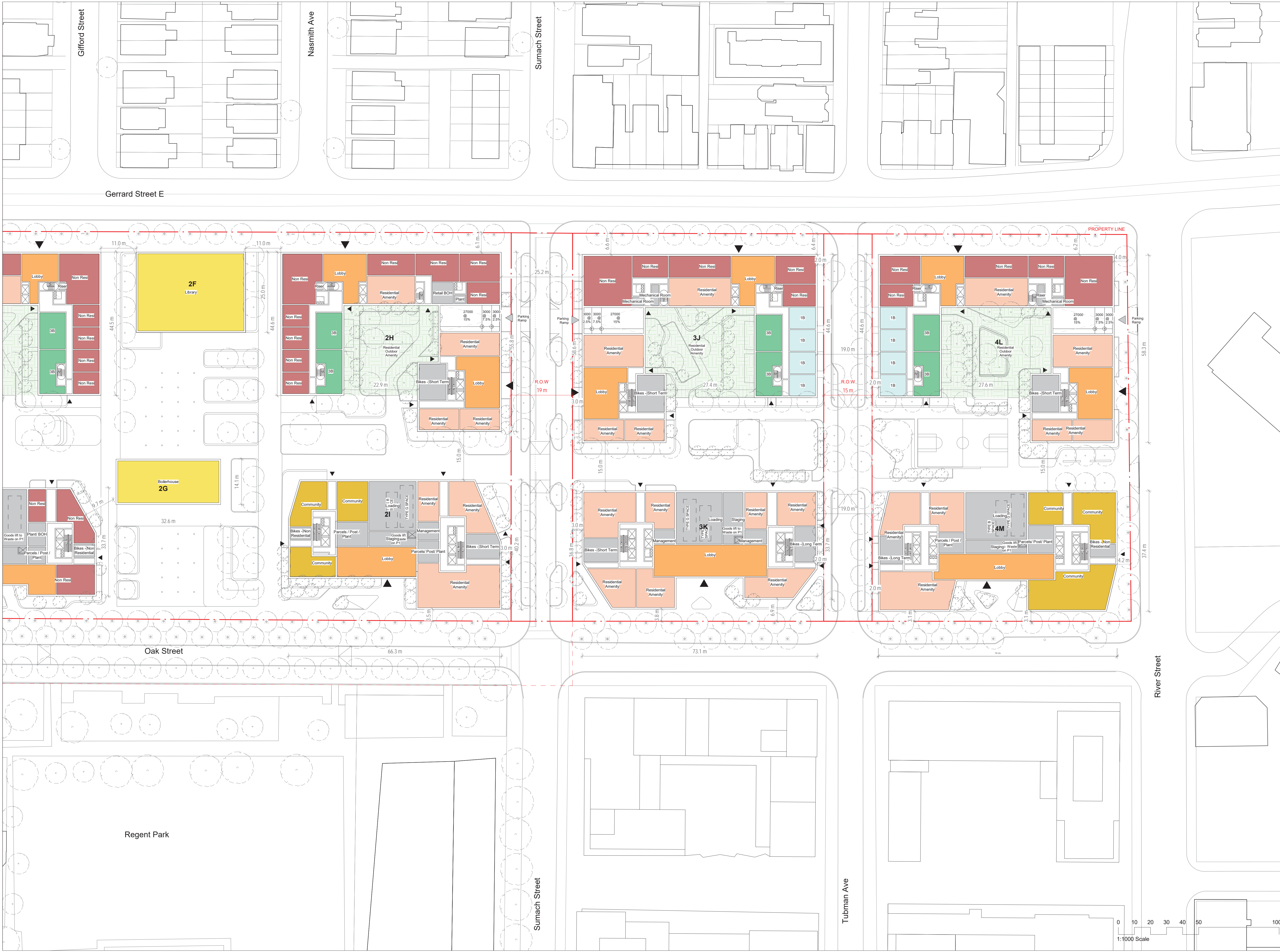
DESIGN  
**Karakusevic Carson Architects**  
 Studio 501  
 37 Cremer St  
 Hackney  
 London E2 8HD  
 mail@karakusevic-carson.com | +44(0)207 566 6300

PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Ground Floor Plan**

DRAWING NUMBER	577 - KCA-XX-01-DR-A - 1004	REVISION	P03
STATUS	S4 - Suitable for stage approval	STAGE	P2
REVISION DATE	11/29/22	DRAWN BY	H/AJAN
FIRST ISSUED	04/01/22	CHECKED BY	PK/RM
		SCALE	1:1000 @ ARCH D
		KCA PROJECT NUMBER	577





CLIENT  
**Toronto Community Housing Corporation (TCHC) and 2747199 Ontario Limited (Deltra Inc.)**

THIS DRAWING IS BASED ON DIMENSIONAL SURVEY INFORMATION PROVIDED BY OTHERS AND REPRESENTS THE ARCHITECT'S BEST CURRENT UNDERSTANDING OF EXISTING CONDITIONS. THE ARCHITECT CANNOT ACCEPT RESPONSIBILITY FOR THE ACCURACY OF THIS SURVEY INFORMATION.  
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**THESE DRAWINGS ARE NOT FOR CONSTRUCTION**

NOTES  
 FOR LANDSCAPE DETAILS REFER TO DRAWINGS BY PFS  
 FOR PARKING AND LOADING OPERATION REFER TO DRAWINGS BY BA GROUP.

Phases 4&5 Property Line  
 City of Toronto Owned Lands Right of Way

- 1B
- 1B+
- 2B
- 2B+
- 3B
- 3B+
- 4B
- 5B
- Amenity
- Ancillary
- Civic
- Community
- Lobby
- Retail
- Residential Outdoor Amenity

P03	S4	P2 - Issued for ZBA	11/29/22
P02	S3	P2 - Draft for Client Review	11/15/22
P01	S4	P1 - Issued for ZBA	04/14/22
P00	S3	P1 - Draft for Client Review	04/01/22

ARCHITECT OF RECORD  
**E.R.A. Architects Inc.**  
 625 Church St  
 Suite 600  
 Toronto  
 Ontario, M4Y 2G1  
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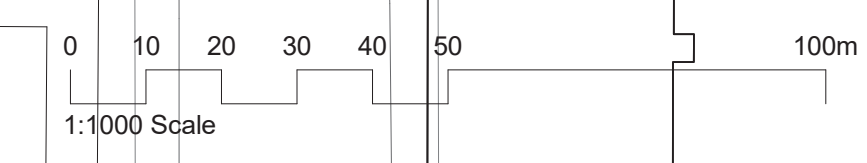


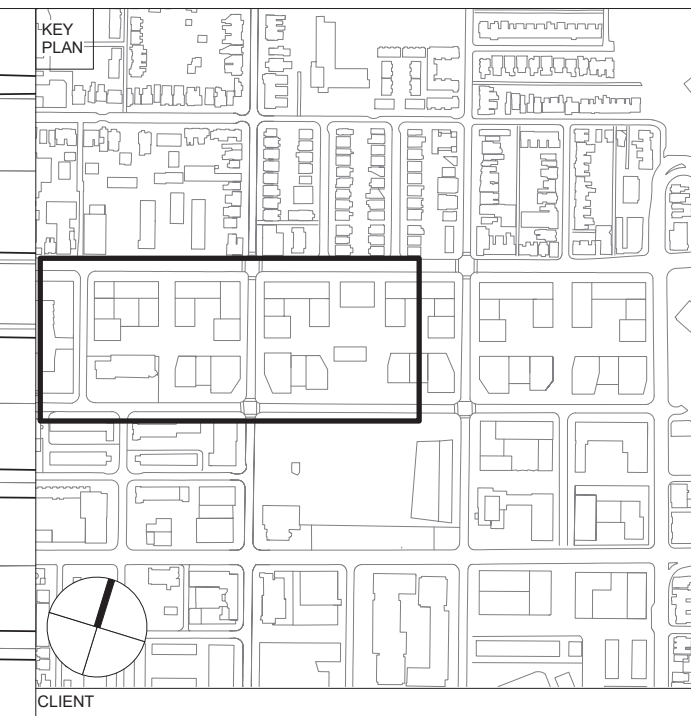
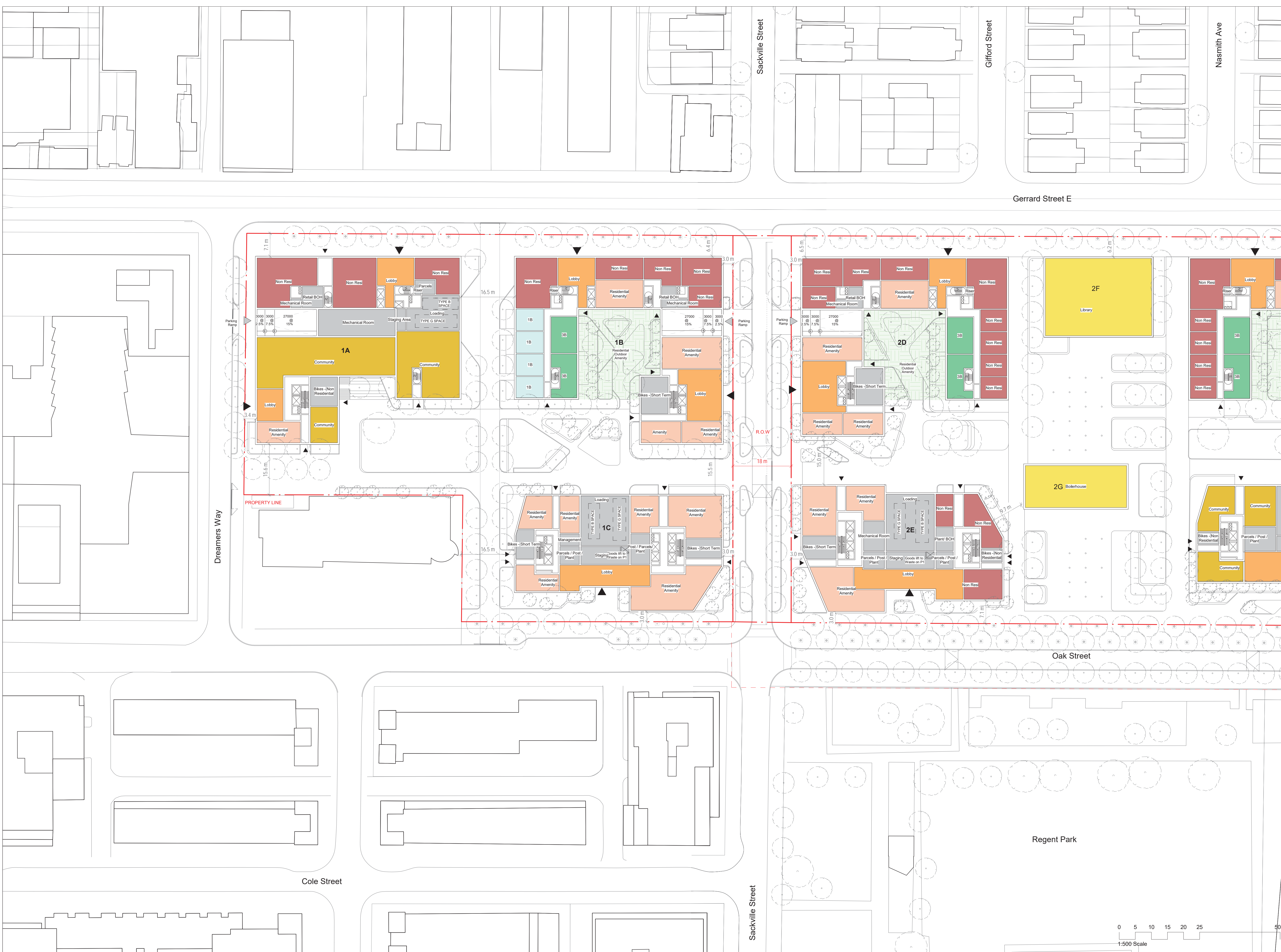
DESIGN  
**Karakusevic Carson Architects**  
 Studio 501  
 37 Cremer St  
 Hackney  
 London E2 8HD  
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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Ground Floor Plan - East**

DRAWING NUMBER	577 - KCA - EA - 01 - DR - A - 1004	REVISION	P03
STATUS	S4 - Suitable for stage approval	STAGE	P2
REVISION DATE	11/29/22	DRAWN BY	H/AJAN
FIRST ISSUED	04/01/22	CHECKED BY	PK/RM
		KCA PROJECT NUMBER	577





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NOTES  
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 FOR PARKING AND LOADING OPERATION REFER TO DRAWINGS BY BA GROUP.

**Phases 4&5 Property Line** (Red solid line)  
**City of Toronto Owned Lands Right of Way** (Red dashed line)

- 1B
- 1B+
- 2B
- 2B+
- 3B
- 3B+
- 4B
- 5B
- Amenity
- Ancillary
- Civic
- Community
- Lobby
- Retail
- Residential Outdoor Amenity

P03	S4	P2 - Issued for ZBA	11/29/22
P02	S3	P2 - Draft for Client Review	11/15/22
P01	S4	P1 - Issued for ZBA	04/14/22
P00	S3	P1 - Draft for Client Review	04/01/22

ARCHITECT OF RECORD  
**E.R.A. Architects Inc.**  
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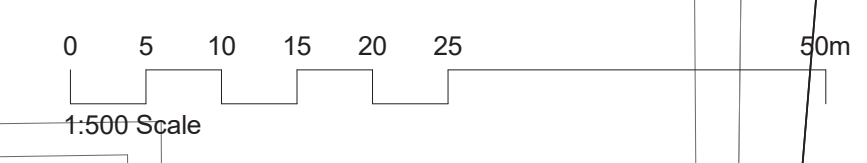


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PROJECT  
**Regent Park Phases 4 & 5**

TITLE  
**Ground Floor Plan - West**

DRAWING NUMBER	577 - KCA-WE-01-DR-A - 1004	REVISION	P03
STATUS	S4 - Suitable for stage approval	STAGE	P2
REVISION DATE	11/29/22	DRAWN BY	H/A/JAN
FIRST ISSUED	04/01/22	CHECKED BY	PK/RM
		KCA PROJECT NUMBER	577



Site Areas

Gross Site Area (inc. public streets) sqm	64405			
Net Site Area (ex. public streets) sqm	58122			

	North (Gerrard St)	East (River St)	South (Oak St)	West (Dreamers Way)
Frontage	555.6m	120.3m	490.3m	81.3m
Existing Lot Line Setbacks (minimum)	6.1m	4.8m	0.0m	7.8m
Proposed Lot Line Setbacks (minimum)	6.1m	4m	3m	3m

	Sackville St	Sumach St	Street J	Total
Area of Public Streets (sqm)	2169	2307	1807	6283

	Plot 1	Plot 2	Plot 3	Plot 4
Plot Area (sqm)	17764	25796	12224	9621
Grade	91.8	90.2	90.4	90.4
Maximum Building Heights (m)	123.6	78.8	104.4	130
Below Grade GFA (sqm)	0	0	0	0

	Total
Existing Ground Floor Area (sqm)	17191
Proposed Ground Floor Area (sqm)	28810

Lot Coverage Ratio	0.50
Floor Space Index	5.22

Open Space	Area of POPs (sqm)
Area of the central plaza	5520
Area of linear strip	9772
<b>Total</b>	<b>15292</b>

Total GFA Per Tenure

Tenure	Use	Gross Construction Area (sqm)	Gross Floor Area (sqm)*
TCHC	Community Space	3,835	3,605
	Non Residential	5,499	5,168
	<b>Total</b>	<b>143,919</b>	<b>135,283</b>
MARKET	Non Residential	4,723	4,440
	<b>Total</b>	<b>162,169</b>	<b>152,439</b>
TPL	Library	2,423	2,277
	<b>Total</b>	<b>2,423</b>	<b>2,277</b>
<b>Total</b>		<b>322,568</b>	<b>303,212</b>

\*GCA to GFA based on 94% efficiency

Total GFA Per Plot & Building

Plot	Building	Total GFA (sqm)	GFA Breakdown per Use (sqm)			
			Residential	Non Residential*	Community Space	Library
1	A	25,873	23,566	948	1,358	
	B	21,863	20,546	1,318		
	C	37,502	37,502			
	<b>Total</b>	<b>17,799</b>	<b>16,355</b>	<b>1,444</b>		
2	D	17,799	16,355	1,444		
	E	25,315	24,716	599		
	F	4,554	4,554		759	2277
	G	405	405			
	H	18,199	16,695	1,503		
	<b>Total</b>	<b>22,538</b>	<b>21,966</b>		<b>572</b>	
3	J	21,442	20,363	1,079		
	<b>Total</b>	<b>32,957</b>	<b>32,957</b>			
4	L	21,760	20,965	794		
	<b>Total</b>	<b>53,007</b>	<b>52,091</b>		<b>916</b>	
<b>Total</b>		<b>303,212</b>	<b>287,722</b>	<b>9,608</b>	<b>3,605</b>	<b>2,277</b>

\*Non residential includes retail and office

Total Unit Count per Tenure

Tenure	Type	Number
TCHC	RGI	633
	Affordable	561
	<b>Total:</b>	<b>1194</b>
Market		1876
	<b>Total:</b>	<b>3,070</b>

Residential Unit Mix

Plot	Building	Tenure	Type	1 Bed	1 Bed Plus*	2 Bed	2 Bed Plus*	3 Bed	3 Bed Plus*	4 Bed	4 Bed Plus*	5 Bed	5 Bed Plus*	Total Units	
				Average Size (sqm)	48	58	71	82	89	103	111	134.5	134		152
1	A	TCHC	RGI	14	5	57	19	66	22	11	3	4	1	202	
	B	Market		137	38	47	14	13	4					252	
	C	Market		251	70	86	25	23	7					461	
2	D	Market		110	30	38	11	10	3					201	
	E	Market		166	46	57	16	15	4					304	
	F	Non Residential												0	
	G	Non Residential												0	
	H	TCHC	RGI	13	4	41	14	44	15	8	2	3	1	144	
	I	TCHC	RGI	10	3	56	19	64	22	8	3	3	1	189	
3	J	Market		136	38	47	14	12	4					251	
	K	Market		221	61	76	22	20	6					406	
4	L	TCHC	RGI	5	2	17	6	35	11	12	4	5	1	98	
		Affordable		8	2	32	11	23	8	0	0	0	0	83	
	M	TCHC	Affordable	77	26	123	41	126	42	23	8	9	3	478	
<b>Total</b>				<b>1148</b>	<b>325</b>	<b>676</b>	<b>210</b>	<b>451</b>	<b>148</b>	<b>62</b>	<b>20</b>	<b>24</b>	<b>7</b>	<b>3070</b>	
				Percentage	37.4%	10.6%	22.0%	6.8%	14.7%	4.8%	2.0%	0.6%	0.8%	0.2%	100.0%
<b>Total</b>				<b>1473</b>		<b>886</b>		<b>598</b>		<b>82</b>		<b>31</b>			
				Percentage	48.0%		28.9%		19.5%		2.7%		1.0%		

\*25% of units are assumed plus sized. These units are a mixture of Barrier Free/Accessible Units and Growing Up Guidelines.

Residential Amenity Space

Indoor		Outdoor		Total Amenity Space (sqm)
Required Area (2 sqm per unit)	Provided Area (sqm)	Required Area	Provided Area (sqm)	
6140	6140	0	0	6140

Bicycle Parking

Plot	Building	Units	Non Residential/Community (sqm)	Required			Provided*				
				Residential Long Term (0.9 per unit)	Non Residential Long & Short Term (0.5 per 100 sqm)	Short Term (0.1 per unit)	Residential Long Term (0.9 per unit)	Short Term (0.1 per unit)	Non Residential Long & Short Term (0.5 per 100 sqm)	Total per Underground	
1	A	202	2306	182	20	12	214	215		56	271
	B	252	1318	227	25	7					
	C	461	0	415	46	0	720	725	71		796
	D	201	1444	181	20	7					
2	E	304	599	274	30	3	516	525	123	20	668
	F	0	4554	0	0	23					0
	G	0	405	0	0	2	25				0
	H	144	1503	130	14	8					
	I	189	572	170	19	3	343	429	128	19	576
	J	251	1079	226	25	5					
3	K	406	0	365	41	0	662	591	129		720
	L	181	794	163	18	4					
4	L	478	916	430	48	5	668	760	71	28	859
	M										
<b>Total</b>				<b>2763</b>	<b>307</b>	<b>77</b>	<b>3245</b>	<b>522</b>	<b>123</b>		
<b>Total Bicycle Parking</b>							<b>3148</b>			<b>3890</b>	

\*bicycle parking estimated from illustrative scheme and subject to change at Site Plan

Loading

Plot	Building	Provided Loading Space			
		Type A	Type B	Type C	Type G
1	A	0	1	0	1
	B	0	1	0	1
	C	0	1	0	1
	D	0	1	0	1
2	E	0	1	0	1
	F	0	0	0	0
	G	0	0	0	0
	H	0	1	0	1
3	J	0	1	0	1
	K	0	1	0	1
4	L	0	1	0	1
	M	0	1	0	1
<b>Total</b>		<b>0</b>	<b>6</b>	<b>0</b>	<b>6</b>

Car Parking

Residential (0.4 per unit)	Visitor (0.06 per unit - Market only)	Required		Provided	
		Residential (0.4 per unit)	Visitor (0.06 per unit - Market only)	Residential (0.4 per unit)	Visitor (0.06 per unit - Market only)
1228	113	1228	113	1341	113

\*parking estimated from illustrative scheme and subject to change at Site Plan



1 Key Plan - Site Statistics  
1: 2000

Toronto Green Standards (TGS)

Regent Park - Phases 4 & 5	Statistics Template - Toronto Green Standard Version 4.0
Mid to High Rise Residential and all New Non-Residential Development	Mid to High Rise Residential and all New Non-Residential Development
General Project Description	General Project Description
Total Gross Floor Area	303,212 m <sup>2</sup>
Breakdown of project components (m <sup>2</sup> ):	
Residential	287,722 m <sup>2</sup>
Retail	9,608 m <sup>2</sup> (including TCH space)
Commercial	916 m <sup>2</sup> (community space)
Industrial	
Institutional/Other	2,277 m <sup>2</sup> (library)
Total number of residential units	3,070 units

Section 1: For Stand Alone Zoning Bylaw Amendment Applications and Site Plan Control Applications	Required	Proposed	Proposed %
Low Emissions Transportation			
Number of Parking Spaces	24	1341	559%
Number of EV Parking Spaces (Residential)	1341	1341	100%
Number of EV Parking Spaces (non-residential)	included above		

Cycling Infrastructure	Required	Proposed	Proposed %
Number of long-term bicycle parking spaces (all uses)	2763	3245	117%
Number of long-term bicycle parking located on:			
a) first storey of building	Y		
b) second storey of building	Y		
c) first level below-ground	Y		
d) second level below-ground	N		
e) other levels below-ground	N		

Statistics Template - Toronto Green Standard Version 4.0	Required	Proposed	Proposed %
Cycling Infrastructure			
Number of short-term bicycle parking spaces	384	645	168%
Number of shower and change facilities (non-residential)	0	0	N/A

Tree Canopy	Required	Proposed	Proposed %
Total Soil Volume (40% of the site area = 66 m <sup>2</sup> x 30 m <sup>3</sup> )	11,710 m <sup>3</sup>		
Soil volume provided within the site area (m <sup>3</sup> )		20,160 m <sup>3</sup>	172%
Soil volume provided within the public boulevard (m <sup>3</sup> )		0	

Section 2: For Site Plan Control Applications

Cycling Infrastructure	Required	Proposed	Proposed %
Number of short-term bicycle parking spaces (all uses) at-grade or on first level below grade			
Number of publicly accessible bicycle parking spaces			
Number of energized outlets for electric bicycles			

Tree Canopy	Required	Proposed	Proposed %
Total site area (m <sup>2</sup> )			
Total Soil Volume (40% of the site area = 66 m <sup>2</sup> x 30 m <sup>3</sup> )			
Total number of trees planted			
Number of surface parking spaces (if applicable)			
Number of shade trees located in surface parking area			

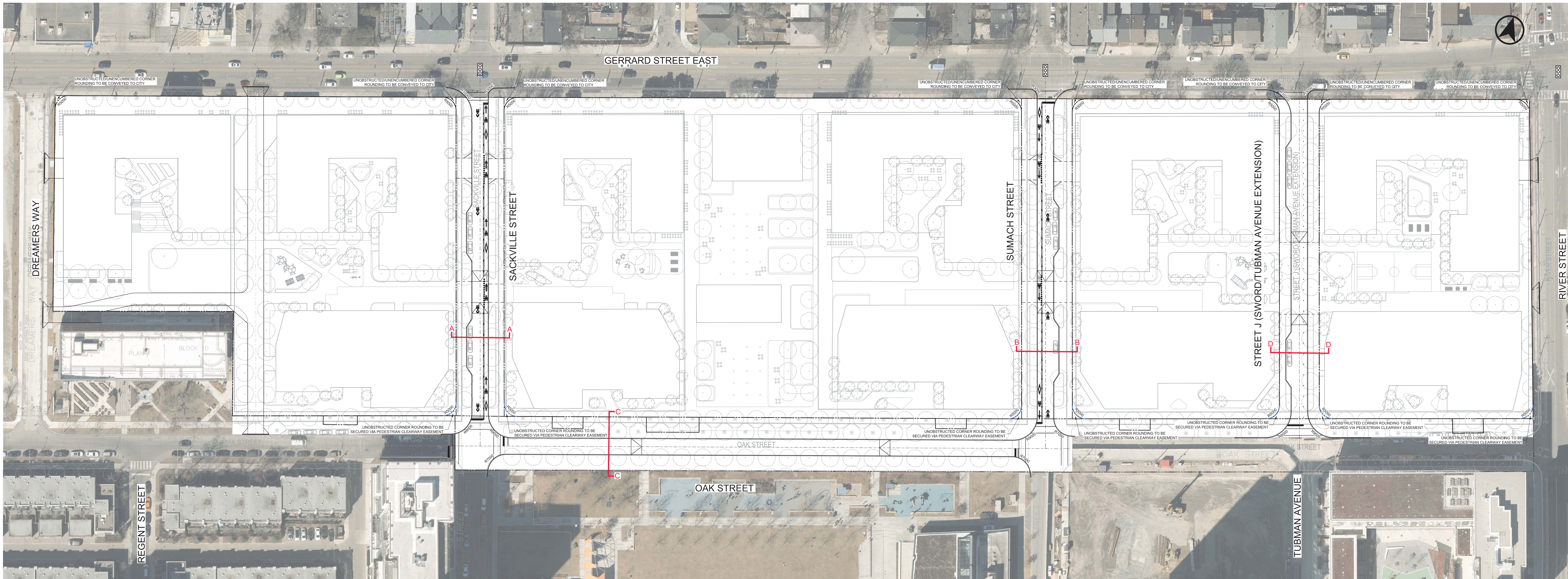
Landscaping & Biodiversity	Required	Proposed	Proposed %
Total non-roof hardscape area (m <sup>2</sup> )			
Total non-roof hardscape area treated for Urban Heat Island (premium residential 75% or non-residential 50%)			
Area of non-roof hardscape treated with: (indicate m <sup>2</sup> )			
a) High-albedo surface material			
b) open-grid pavement			
c) shade from tree canopy			

Statistics Template - Toronto Green Standard Version 4.0	Required	Proposed	Proposed %
Landscaping & Biodiversity			
a) shade from high-albedo structures			
e) shade from energy generation structures			
Percentage of lot Area as Soft Landscaping (non-residential only)			
Total number of plants			
Total number of native plants and % of total plants			
Available Roof Space (m <sup>2</sup> )			
Available Roof Space provided as Green Roof (m <sup>2</sup> )			
Available Roof Space provided as Cool Roof (m <sup>2</sup> )			
Available Roof Space provided as Solar Panels (m <sup>2</sup> )			

Bed Collision Mitigation	Required	Proposed	Proposed %
Total area of glazing of all elevations within 1.6m above grade			
Total area of treated glazing (minimum 85% of total area of glazing within 1.6m above grade) (m <sup>2</sup> )			
Percentage of glazing within 1.6m above grade treated with:			
a) Visual markers			
b) non-reflective glass			
c) Building integrated structures			

## Appendix B: Functional Road Plan



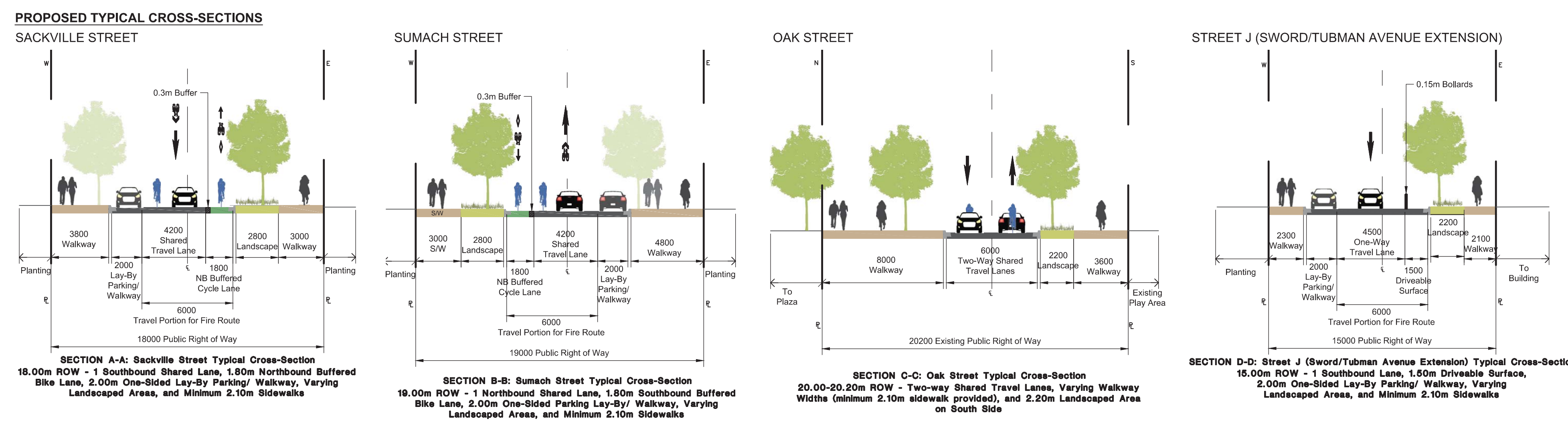


- GENERAL NOTES**
1. BASE SURVEY PLAN 21-15-128-00 DATED AUGUST 18, 2021, PREPARED BY J.D. BARNES LTD.
  2. ARCHITECTURAL SITE PLAN DATED NOVEMBER 7, 2022 PREPARED BY KARAKUSEVIC CARSON ARCHITECTS.
- GENERAL LEGEND**
- EXISTING SIGNALIZED INTERSECTION
  - PROPOSED CURB
  - PROPOSED CENTRELINE
  - PEDESTRIAN CLEARWAY EASEMENT

NO.	DATE	BY	REVISION
03	11-28-22	KIC	RE-ISSUED TO CLIENT FOR SUBMISSION
02	11-17-22	KIC	RE-ISSUED TO CLIENT FOR SUBMISSION
01	11-10-22	KIC	RE-ISSUED TO CLIENT FOR SUBMISSION
00	11-07-22	KIC	ISSUED TO CLIENT FOR SUBMISSION



**REGENT PARK PHASES 4 & 5**



**FUNCTIONAL ROAD PLAN**

Date: April 13, 2022  
 Project No.: 7575-46  
 Scale: 1:500



## **Appendix C: Minimum and Proposed Vehicle Parking Supply Requirements**



Site Specific Minimum Vehicle Parking Requirement

**BLOCKS 1A - TCHC** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
Social	202 units	0.40 spaces per unit	81	100%	100%	100%
Resident Sub-total	202 units	0.40 spaces per unit (blended)	81	81	81	81
Non-Residential						
Social Residential Visitors	202 units	0.00 spaces per unit	0	0%	0%	0%
Community	1358 sm GFA	1 spaces / 175 sm GFA	8	2	8	8
Office	237 sm GFA	1 spaces / 300 sm GFA	1	100%	60%	0%
Retail	711 sm GFA	1 spaces / 100 sm GFA	7	1	7	7
Non-Residential Sub-total			16	4	16	15
<b>TOTAL</b>			<b>97</b>	<b>85</b>	<b>97</b>	<b>96</b>

**BLOCKS 1B & 1C - MARKET** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
1-bedroom	496 units	0.41 spaces per unit	203			
2-bedroom	172 units	0.58 spaces per unit	100	100%	100%	100%
3-bedroom	47 units	1.04 spaces per unit	49			
Resident Sub-total	<b>715 units</b>	0.49 spaces per unit (blended)	352	352	352	352
Non-Residential						
Residential Visitors	713 units	0.06 spaces per unit	43	0	15	43
Office	330 sm GFA	1 spaces / 300 sm GFA	1	100%	60%	0%
Retail	989 sm GFA	1 spaces / 100 sm GFA	10	2	10	10
Non-Residential Sub-total			54	3	26	53
<b>TOTAL</b>			<b>406</b>	<b>355</b>	<b>378</b>	<b>405</b>

**BLOCKS 2D & 2E - MARKET** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
1-bedroom	352 units	0.41 spaces per unit	144			
2-bedroom	122 units	0.58 spaces per unit	71	100%	100%	100%
3-bedroom	47 units	1.04 spaces per unit	49			
Resident Sub-total	<b>599 units</b>	0.49 spaces per unit (blended)	248	248	248	248
Non-Residential						
Residential Visitors	505 units	0.06 spaces per unit	30	0	11	30
Office	511 sm GFA	1 spaces / 300 sm GFA	2	100%	60%	0%
Retail	1532 sm GFA	1 spaces / 100 sm GFA	15	3	15	15
Non-Residential Sub-total			47	6	27	45
<b>TOTAL</b>			<b>295</b>	<b>253</b>	<b>275</b>	<b>293</b>

**BLOCKS 2F & 2G - TCHC / MARKET** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Non-Residential						
Community	759 sm GFA	1 spaces / 175 sm GFA	4	0	1	4
Library	2277 sm GFA	1 spaces / 175 sm GFA	13	13	13	13
Culture	405 sm GFA	1 spaces / 175 sm GFA	2	2	2	2
Office	380 sm GFA	1 spaces / 300 sm GFA	1	100%	60%	0%
Retail	1139 sm GFA	1 spaces / 100 sm GFA	11	2	11	11
Non-Residential Sub-total			19	18	28	30
<b>TOTAL</b>			<b>19</b>	<b>18</b>	<b>28</b>	<b>30</b>

**BLOCKS 2H & 2I - TCHC** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
Social	333 units	0.40 spaces per unit	133	100%	100%	100%
Resident Sub-total	333 units	0.40 spaces per unit (blended)	133	133	133	133
Non-Residential						
Social Residential Visitors	333 units	0.00 spaces per unit	0	0%	0%	0%
Community	572 sm GFA	1 spaces / 175 sm GFA	3	25%	100%	3
Office	376 sm GFA	1 spaces / 300 sm GFA	1	100%	60%	0%
Retail	1127 sm GFA	1 spaces / 100 sm GFA	11	2	11	11
Non-Residential Sub-total			16	4	16	14
<b>TOTAL</b>			<b>149</b>	<b>137</b>	<b>149</b>	<b>147</b>

**BLOCKS 3I & 3K - MARKET** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
1-bedroom	456 units	0.41 spaces per unit	187			
2-bedroom	159 units	0.58 spaces per unit	92	100%	100%	100%
3-bedroom	42 units	1.04 spaces per unit	44			
Resident Sub-total	657 units	0.49 spaces per unit (blended)	323	323	323	323
Non-Residential						
Residential Visitors	657 units	0.06 spaces per unit	39	0	14	39
Community	0 sm GFA	1 spaces / 175 sm GFA	0	100%	100%	100%
Office	270 sm GFA	1 spaces / 300 sm GFA	1	100%	60%	0%
Retail	809 sm GFA	1 spaces / 100 sm GFA	8	2	8	8
Non-Residential Sub-total			48	3	23	47
<b>TOTAL</b>			<b>371</b>	<b>326</b>	<b>346</b>	<b>370</b>

**BLOCKS 4L & 4M - TCHC** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
Social	659 units	0.40 spaces per unit	264	100%	100%	100%
Resident Sub-total	659 units	0.40 spaces per unit (blended)	264	264	264	264
Non-Residential						
Social Residential Visitors	659 units	0.00 spaces per unit	0	0%	0%	0%
Community	916 sm GFA	1 spaces / 175 sm GFA	5	1	5	5
Office	199 sm GFA	1 spaces / 300 sm GFA	1	100%	60%	0%
Retail	596 sm GFA	1 spaces / 100 sm GFA	6	1	6	6
Non-Residential Sub-total			12	3	12	11
<b>TOTAL</b>			<b>276</b>	<b>267</b>	<b>276</b>	<b>275</b>

**SITE TOTAL** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
1-bedroom (Market)	848 units	0.41 spaces per unit	347 spaces			
2-bedroom (Market)	294 units	0.58 spaces per unit	171 spaces	100%	100%	100%
3-bedroom (Market)	79 units	1.04 spaces per unit	82 spaces			
Social	202 units	0.40 spaces per unit	81 spaces			
Residential Sub-total	<b>3670 units</b>	0.22 spaces per unit (blended)	681	681	681	681
Non-Residential						
Market Residential Visitors	1678 units	0.06 spaces per unit	73 spaces	0 spaces	26 spaces	73 spaces
Social Residential Visitors	202 units	0.00 spaces per unit	0 spaces	0 spaces	0 spaces	0 spaces
Community	2117 sm GFA	1 spaces / 175 sm GFA	12 spaces	0 spaces	4 spaces	12 spaces
Library	2277 sm GFA	1 spaces / 175 sm GFA	13 spaces	13 spaces	13 spaces	13 spaces
Culture	405 sm GFA	1 spaces / 175 sm GFA	2 spaces	2 spaces	2 spaces	2 spaces
Office	2301 sm GFA	1 spaces / 300 sm GFA	5 spaces	5 spaces	3 spaces	0 spaces
Retail	6902 sm GFA	1 spaces / 100 sm GFA	43 spaces	9 spaces	43 spaces	43 spaces
Non-Residential Sub-total			148	25	91	143
<b>TOTAL</b>			<b>829</b>	<b>710</b>	<b>772</b>	<b>824</b>

**BUILDING 1A**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum	Maximum Parking Provided (Ratio)	Maximum Parking Provided <sup>2</sup> (Spaces)	Effective Parking Required	Effective Parking Required
<b>Residential</b>							
Bachelor		None	0	0	0	0.30	0
1-Bedroom		None	0	0.50	0	0.50	0
2-Bedroom		None	0	0.80	0	0.80	0
3-Bedroom		None	0	1.00	0	1.00	0
Affordable Housing	202	None	0	0.50	101	0.20	40
<b>Resident Sub-Total</b>	<b>202</b>	<b>None</b>	<b>0</b>	<b>0.50</b>	<b>101</b>	<b>0.20</b>	<b>40</b>
<b>Non-Residential</b>							
Market Residential	0	2 plus 0.01 spaces per unit	0	5 plus 0.10 spaces per unit	0	0.10 spaces per unit	0
Affordable Housing	202	None	0	None	0	None	0
Residential Visitor <sup>3</sup>	0	None	0	None	0	None	0
Community	1,358	None	0	0.80 spaces per 100 sqm GFA	10	0.40 spaces per 100 sqm GFA	5
Library		None	0	3.50 spaces per 100 sqm GFA	0	1.00 spaces per 100 sqm GFA	0
Culture		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Office	237	None	0	0.80 spaces per 100 sqm GFA	1	0.40 spaces per 100 sqm GFA	0
Retail	711	None	0	3.50 spaces per 100 sqm GFA	24	1.00 spaces per 100 sqm GFA	7
<b>Non-Resident Sub-Total</b>	<b>--</b>	<b>0</b>	<b>--</b>	<b>--</b>	<b>35</b>	<b>--</b>	<b>12</b>
<b>TOTAL</b>	<b>--</b>	<b>0</b>	<b>0</b>	<b>--</b>	<b>136</b>	<b>--</b>	<b>52</b>
<b>Accessible Parking Requirement<sup>4</sup></b>							
B1 If the number of effective parking spaces is 13 to 100, a minimum of 1 accessible parking space for every 25 effective parking spaces or part thereof must comply with all regulations for an accessible parking space in Section 200.15.							3
<b>Adjusted Minimum Parking Requirement<sup>5</sup></b>							
2 accessible parking spaces.							3

**BUILDINGS 1B & 1C**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum	Maximum Parking Provided (Ratio)	Maximum Parking Provided <sup>2</sup> (Spaces)	Effective Parking Required	Effective Parking Required
<b>Residential</b>							
Bachelor		None	0	0	0	0.30	0
1-Bedroom	496	None	0	0.50	248	0.50	248
2-Bedroom	172	None	0	0.80	137	0.80	137
3-Bedroom	47	None	0	1.00	47	1.00	47
Affordable Housing		None	0	0.50	0	0.20	0
<b>Resident Sub-Total</b>	<b>718</b>	<b>None</b>	<b>0</b>	<b>0.60</b>	<b>432</b>	<b>0.20</b>	<b>432</b>
<b>Non-Residential</b>							
Market Residential	718	2 plus 0.01 spaces per unit	9	5 plus 0.10 spaces per unit	76	0.10 spaces per unit	71
Affordable Housing		None	0	None	0	None	0
Residential Visitor <sup>3</sup>	0	None	0	None	0	None	0
Community		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Library		None	0	3.50 spaces per 100 sqm GFA	0	1.00 spaces per 100 sqm GFA	0
Culture		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Office	330	None	0	0.80 spaces per 100 sqm GFA	2	0.40 spaces per 100 sqm GFA	1
Retail	869	None	0	3.50 spaces per 100 sqm GFA	34	1.00 spaces per 100 sqm GFA	9
<b>Non-Resident Sub-Total</b>	<b>--</b>	<b>0</b>	<b>--</b>	<b>--</b>	<b>112</b>	<b>--</b>	<b>81</b>
<b>TOTAL</b>	<b>--</b>	<b>0</b>	<b>0</b>	<b>--</b>	<b>544</b>	<b>--</b>	<b>513</b>
<b>Accessible Parking Requirement<sup>4</sup></b>							
C1 If the number of effective parking spaces is more than 100, a minimum of 5 accessible parking spaces plus 1 accessible parking space for every 50 effective parking spaces or part thereof in excess of 100 parking spaces must comply with all regulations for an accessible parking space in Section 200.15.							14
<b>Adjusted Minimum Parking Requirement<sup>5</sup></b>							
14 accessible parking spaces.							14

**BUILDINGS 2D & 2E**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum	Maximum Parking Provided (Ratio)	Maximum Parking Provided <sup>2</sup> (Spaces)	Effective Parking Required	Effective Parking Required
<b>Residential</b>							
Bachelor		None	0	0	0	0.30	0
1-Bedroom	352	None	0	0.50	176	0.50	176
2-Bedroom	122	None	0	0.80	97	0.80	97
3-Bedroom	32	None	0	1.00	32	1.00	32
Affordable Housing		None	0	0.50	0	0.20	0
<b>Resident Sub-Total</b>	<b>506</b>	<b>None</b>	<b>0</b>	<b>0.60</b>	<b>305</b>	<b>0.60</b>	<b>305</b>
<b>Non-Residential</b>							
Market Residential	606	2 plus 0.01 spaces per unit	7	5 plus 0.10 spaces per unit	55	0.10 spaces per unit	50
Affordable Housing		None	0	None	0	None	0
Residential Visitor <sup>3</sup>	0	None	0	None	0	None	0
Community		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Library		None	0	3.50 spaces per 100 sqm GFA	0	1.00 spaces per 100 sqm GFA	0
Culture		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Office	511	None	0	0.80 spaces per 100 sqm GFA	4	0.40 spaces per 100 sqm GFA	2
Retail	1532	None	0	3.50 spaces per 100 sqm GFA	53	1.00 spaces per 100 sqm GFA	15
<b>Non-Resident Sub-Total</b>	<b>--</b>	<b>0</b>	<b>--</b>	<b>--</b>	<b>112</b>	<b>--</b>	<b>67</b>
<b>TOTAL</b>	<b>--</b>	<b>0</b>	<b>0</b>	<b>--</b>	<b>417</b>	<b>--</b>	<b>372</b>
<b>Accessible Parking Requirement<sup>4</sup></b>							
C1 If the number of effective parking spaces is more than 100, a minimum of 5 accessible parking spaces plus 1 accessible parking space for every 50 effective parking spaces or part thereof in excess of 100 parking spaces must comply with all regulations for an accessible parking space in Section 200.15.							11
<b>Adjusted Minimum Parking Requirement<sup>5</sup></b>							
11 accessible parking spaces.							11

**BUILDINGS 2F & 2G**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum	Maximum Parking Provided (Ratio)	Maximum Parking Provided <sup>2</sup> (Spaces)	Effective Parking Required	Effective Parking Required
<b>Residential</b>							
Bachelor		None	0	0	0	0.30	0
1-Bedroom		None	0	0.50	0	0.50	0
2-Bedroom		None	0	0.80	0	0.80	0
3-Bedroom		None	0	1.00	0	1.00	0
Affordable Housing		None	0	0.50	0	0.20	0
<b>Resident Sub-Total</b>	<b>0</b>	<b>None</b>	<b>0</b>	<b>0.50</b>	<b>0</b>	<b>0.20</b>	<b>0</b>
<b>Non-Residential</b>							
Market Residential	0	2 plus 0.01 spaces per unit	0	5 plus 0.10 spaces per unit	0	0.10 spaces per unit	0
Affordable Housing	0	None	0	None	0	None	0
Residential Visitor <sup>3</sup>	0	None	0	None	0	None	0
Community	759	None	0	0.80 spaces per 100 sqm GFA	6	0.40 spaces per 100 sqm GFA	3
Library	2277	None	0	3.50 spaces per 100 sqm GFA	70	1.00 spaces per 100 sqm GFA	22
Culture	405	None	0	0.80 spaces per 100 sqm GFA	3	0.40 spaces per 100 sqm GFA	1
Office	380	None	0	0.80 spaces per 100 sqm GFA	3	0.40 spaces per 100 sqm GFA	1
Retail	1139	None	0	3.50 spaces per 100 sqm GFA	39	1.00 spaces per 100 sqm GFA	11
<b>Non-Resident Sub-Total</b>	<b>--</b>	<b>0</b>	<b>--</b>	<b>--</b>	<b>180</b>	<b>--</b>	<b>38</b>
<b>TOTAL</b>	<b>--</b>	<b>0</b>	<b>0</b>	<b>--</b>	<b>180</b>	<b>--</b>	<b>38</b>
<b>Accessible Parking Requirement<sup>4</sup></b>							
B1 If the number of effective parking spaces is 13 to 100, a minimum of 1 accessible parking space for every 25 effective parking spaces or part thereof must comply with all regulations for an accessible parking space in Section 200.15.							2
<b>Adjusted Minimum Parking Requirement<sup>5</sup></b>							
2 accessible parking spaces.							2

**BUILDINGS 2H & 2I**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum	Maximum Parking Provided (Ratio)	Maximum Parking Provided <sup>2</sup> (Spaces)	Effective Parking Required	Effective Parking Required
<b>Residential</b>							
Bachelor		None	0	0	0	0.30	0
1-Bedroom		None	0	0.50	0	0.50	0
2-Bedroom		None	0	0.80	0	0.80	0
3-Bedroom		None	0	1.00	0	1.00	0
Affordable Housing	333	None	0	0.50	166	0.20	66
<b>Resident Sub-Total</b>	<b>333</b>	<b>None</b>	<b>0</b>	<b>0.50</b>	<b>166</b>	<b>0.20</b>	<b>66</b>
<b>Non-Residential</b>							
Market Residential	0	2 plus 0.01 spaces per unit	0	5 plus 0.10 spaces per unit	0	0.10 spaces per unit	0
Affordable Housing	333	None	0	None	0	None	0
Residential Visitor <sup>3</sup>	0	None	0	None	0	None	0
Community	672	None	0	0.80 spaces per 100 sqm GFA	4	0.40 spaces per 100 sqm GFA	2
Library		None	0	3.50 spaces per 100 sqm GFA	0	1.00 spaces per 100 sqm GFA	0
Culture		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Office	376	None	0	0.80 spaces per 100 sqm GFA	3	0.40 spaces per 100 sqm GFA	1
Retail	1127	None	0	3.50 spaces per 100 sqm GFA	39	1.00 spaces per 100 sqm GFA	11
<b>Non-Resident Sub-Total</b>	<b>--</b>	<b>0</b>	<b>--</b>	<b>--</b>	<b>46</b>	<b>--</b>	<b>14</b>
<b>TOTAL</b>	<b>--</b>	<b>0</b>	<b>0</b>	<b>--</b>	<b>212</b>	<b>--</b>	<b>80</b>
<b>Accessible Parking Requirement<sup>4</sup></b>							
B1 If the number of effective parking spaces is 13 to 100, a minimum of 1 accessible parking space for every 25 effective parking spaces or part thereof must comply with all regulations for an accessible parking space in Section 200.15.							4
<b>Adjusted Minimum Parking Requirement<sup>5</sup></b>							
4 accessible parking spaces.							4

**BUILDINGS 3J & 3K**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum	Maximum Parking Provided (Ratio)	Maximum Parking Provided <sup>2</sup> (Spaces)	Effective Parking Required	Effective Parking Required
<b>Residential</b>							
Bachelor		None	0	0	0	0.30	0
1-Bedroom	456	None	0	0.50	228	0.50	228
2-Bedroom	159	None	0	0.80	127	0.80	127
3-Bedroom	45	None	0	1.00	45	1.00	45
Affordable Housing		None	0	0.50	0	0.20	0
<b>Resident Sub-Total</b>	<b>657</b>	<b>None</b>	<b>0</b>	<b>0.60</b>	<b>397</b>	<b>0.60</b>	<b>397</b>
<b>Non-Residential</b>							
Market Residential	657	2 plus 0.01 spaces per unit	8	5 plus 0.10 spaces per unit	70	0.10 spaces per unit	65
Affordable Housing		None	0	None	0	None	0
Residential Visitor <sup>3</sup>	0	None	0	None	0	None	0
Community		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Library		None	0	3.50 spaces per 100 sqm GFA	0	1.00 spaces per 100 sqm GFA	0
Culture		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Office	270	None	0	0.80 spaces per 100 sqm GFA	2	0.40 spaces per 100 sqm GFA	1
Retail	800	None	0	3.50 spaces per 100 sqm GFA	28	1.00 spaces per 100 sqm GFA	8
<b>Non-Resident Sub-Total</b>	<b>--</b>	<b>0</b>	<b>--</b>	<b>--</b>	<b>100</b>	<b>--</b>	<b>74</b>
<b>TOTAL</b>	<b>--</b>	<b>0</b>	<b>0</b>	<b>--</b>	<b>497</b>	<b>--</b>	<b>471</b>
<b>Accessible Parking Requirement<sup>4</sup></b>							
C1 If the number of effective parking spaces is more than 100, a minimum of 5 accessible parking spaces plus 1 accessible parking space for every 50 effective parking spaces or part thereof in excess of 100 parking spaces must comply with all regulations for an accessible parking space in Section 200.15.							13
<b>Adjusted Minimum Parking Requirement<sup>5</sup></b>							
13 accessible parking spaces.							13

**BUILDINGS 4L & 4M**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum	Maximum Parking Provided (Ratio)	Maximum Parking Provided <sup>2</sup> (Spaces)	Effective Parking Required	Effective Parking Required
<b>Residential</b>							
Bachelor		None	0	0	0	0.30	0
1-Bedroom		None	0	0.50	0	0.50	0
2-Bedroom		None	0	0.80	0	0.80	0
3-Bedroom		None	0	1.00	0	1.00	0
Affordable Housing	659	None	0	0.50	329	0.20	131
<b>Resident Sub-Total</b>	<b>659</b>	<b>None</b>	<b>0</b>	<b>0.60</b>	<b>329</b>	<b>0.20</b>	<b>131</b>
<b>Non-Residential</b>							
Market Residential	0	2 plus 0.01 spaces per unit	0	5 plus 0.10 spaces per unit	0	0.10 spaces per unit	0
Affordable Housing	659	None	0	None	0	None	0
Residential Visitor <sup>3</sup>	0	None	0	None	0	None	0
Community	918	None	0	0.80 spaces per 100 sqm GFA	7	0.40 spaces per 100 sqm GFA	3
Library		None	0	3.50 spaces per 100 sqm GFA	0	1.00 spaces per 100 sqm GFA	0
Culture		None	0	0.80 spaces per 100 sqm GFA	0	0.40 spaces per 100 sqm GFA	0
Office	199	None	0	0.80 spaces per 100 sqm GFA	1	0.40 spaces per 100 sqm GFA	0
Retail	596	None	0	3.50 spaces per 100 sqm GFA	20	1.00 spaces per 100 sqm GFA	5
<b>Non-Resident Sub-Total</b>	<b>--</b>	<b>0</b>	<b>--</b>	<b>--</b>	<b>37</b>	<b>--</b>	<b>8</b>
<b>TOTAL</b>	<b>--</b>	<b>0</b>	<b>0</b>	<b>--</b>	<b>367</b>	<b>--</b>	<b>139</b>
<b>Accessible Parking Requirement<sup>4</sup></b>							
C1 If the number of effective parking spaces is more than 100, a minimum of 5 accessible parking spaces plus 1 accessible parking space for every 50 effective parking spaces or part thereof in excess of 100 parking spaces must comply with all regulations for an accessible parking space in Section 200.15.							6
<b>Adjusted Minimum Parking Requirement<sup>5</sup></b>							
6 accessible parking spaces.							6

**BUILDINGS 4L & 4M**

Use	Units / GFA	Minimum Parking Required (Ratio)	Minimum
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Proposed Vehicle Parking Supply

**BLOCKS 1A - TCHC** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
All TCHC Units	202 units	0.40 spaces per unit	81	100%	100%	100%
Resident Sub-total	202 units	0.40 spaces per unit (blended)	81	29%	91	81
Non-Residential						
Social Residential Visitors	202 units	0.00 spaces per unit	0	0%	0	0
Community	1358 sm GFA	0 spaces / 175 sm GFA	0	0	0	0
Office	237 sm GFA	0 spaces / 300 sm GFA	0	100%	60%	0%
Retail	711 sm GFA	0 spaces / 100 sm GFA	0	20%	100%	100%
Non-Residential Sub-total			0	0	0	0
<b>TOTAL</b>			<b>81</b>	<b>81</b>	<b>81</b>	<b>81</b>

**BLOCKS 1B & 1C - MARKET** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
All Market Units	713 units	0.40 spaces per unit	285	100%	100%	100%
Resident Sub-total	713 units	0.40 spaces per unit (blended)	285	28%	285	285
Non-Residential						
Residential Visitors	713 units	0.06 spaces per unit	43	0%	15	43
Office	330 sm GFA	0 spaces / 300 sm GFA	0	0	0	0
Retail	989 sm GFA	0 spaces / 100 sm GFA	0	20%	100%	100%
Non-Residential Sub-total			43	0	15	43
<b>TOTAL</b>			<b>328</b>	<b>28%</b>	<b>300</b>	<b>328</b>

**BLOCKS 2D & 2E - MARKET** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
All Market Units	505 units	0.40 spaces per unit	202	100%	100%	100%
Resident Sub-total	505 units	0.40 spaces per unit (blended)	202	20%	202	202
Non-Residential						
Residential Visitors	505 units	0.06 spaces per unit	30	0%	11	30
Office	511 sm GFA	0 spaces / 300 sm GFA	0	100%	60%	0%
Retail	1532 sm GFA	0 spaces / 100 sm GFA	0	20%	100%	100%
Non-Residential Sub-total			30	0	11	30
<b>TOTAL</b>			<b>232</b>	<b>20%</b>	<b>213</b>	<b>232</b>

**BLOCKS 2F & 2G - TCHC / MARKET** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Non-Residential						
Community	759 sm GFA	0 spaces / 175 sm GFA	0	0	0	0
Library	2277 sm GFA	0 spaces / 175 sm GFA	0	100%	100%	100%
Culture	405 sm GFA	0 spaces / 175 sm GFA	0	0	0	0
Office	380 sm GFA	0 spaces / 300 sm GFA	0	100%	100%	100%
Retail	1139 sm GFA	0 spaces / 100 sm GFA	0	20%	100%	100%
Non-Residential Sub-total			0	0	0	0
<b>TOTAL</b>			<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>

**BLOCKS 2H & 2I - TCHC** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
All TCHC Units	333 units	0.40 spaces per unit	133	100%	100%	100%
Resident Sub-total	333 units	0.40 spaces per unit (blended)	133	13%	133	133
Non-Residential						
Social Residential Visitors	333 units	0.00 spaces per unit	0	0	0	0
Community	572 sm GFA	0 spaces / 175 sm GFA	0	20%	100%	100%
Office	376 sm GFA	0 spaces / 300 sm GFA	0	100%	60%	0%
Retail	1127 sm GFA	0 spaces / 100 sm GFA	0	20%	100%	100%
Non-Residential Sub-total			0	0	0	0
<b>TOTAL</b>			<b>133</b>	<b>13%</b>	<b>133</b>	<b>133</b>

**BLOCKS 3I & 3K - MARKET** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
All Market Units	657 units	0.40 spaces per unit	263	100%	100%	100%
Resident Sub-total	657 units	0.40 spaces per unit (blended)	263	28%	263	263
Non-Residential						
Residential Visitors	657 units	0.06 spaces per unit	39	0%	14	39
Community	0 sm GFA	0 spaces / 175 sm GFA	0	100%	100%	100%
Office	270 sm GFA	0 spaces / 175 sm GFA	0	100%	60%	0%
Retail	809 sm GFA	0 spaces / 175 sm GFA	0	20%	100%	100%
Non-Residential Sub-total			39	0	14	39
<b>TOTAL</b>			<b>302</b>	<b>28%</b>	<b>277</b>	<b>302</b>

**BLOCKS 4L & 4M - TCHC** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
All TCHC Units	659 units	0.40 spaces per unit	264	100%	100%	100%
Resident Sub-total	659 units	0.40 spaces per unit (blended)	264	28%	264	264
Non-Residential						
Social Residential Visitors	659 units	0.00 spaces per unit	0	0	0	0
Community	916 sm GFA	0 spaces / 175 sm GFA	0	20%	100%	100%
Office	199 sm GFA	0 spaces / 300 sm GFA	0	100%	60%	0%
Retail	596 sm GFA	0 spaces / 100 sm GFA	0	20%	100%	100%
Non-Residential Sub-total			0	0	0	0
<b>TOTAL</b>			<b>264</b>	<b>28%</b>	<b>264</b>	<b>264</b>

**SITE TOTAL** \* bylaw uses normal rounding

Use / Type	# of Units / GFA	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)	Shared Parking		
				AM	PM	Evening
Residential						
All Market Units	<b>1876 units</b>	0.40 spaces per unit	487 spaces	100%	100%	100%
All TCHC Units	202 units	0.40 spaces per unit	81 spaces			
Residential Sub-total	2078 units	0.27 spaces per unit (blended)	568	56%	568	568
Non-Residential						
Market Residential Visitors	1876 units	0.06 spaces per unit	73 spaces	0 spaces	26 spaces	73 spaces
Social Residential Visitors	202 units	0.00 spaces per unit	0 spaces	0 spaces	0 spaces	0 spaces
Community	2117 sm GFA	0 spaces / 175 sm GFA	0 spaces	0 spaces	35%	0 spaces
Library	2277 sm GFA	0 spaces / 175 sm GFA	0 spaces	100%	100%	100%
Culture	405 sm GFA	0 spaces / 175 sm GFA	0 spaces	100%	100%	100%
Office	2201 sm GFA	0 spaces / 300 sm GFA	0 spaces	100%	60%	0 spaces
Retail	6602 sm GFA	0 spaces / 100 sm GFA	0 spaces	20%	100%	100%
Non-Residential Sub-total			73	0	26	73
<b>TOTAL</b>			<b>641</b>	<b>56%</b>	<b>594</b>	<b>641</b>

## **Appendix D: Minimum and Proposed Bicycle Parking Supply Requirements**



Minimum Bicycle Parking Requirement TGS v3 Zone 1

BLOCKS 1A - TCHC

Use / Type	# of Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Residential	202 units	Occupant	0.90 spaces per unit	182 spaces
		Visitor	0.10 spaces per unit	21 spaces
		Total	1.00 spaces per unit	203 spaces
Community	1358 sm GFA	Total	--	0 spaces
Office	237 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	1 spaces
		Short-Term	3 plus 0.2 spaces / 100 sqm GFA	4 spaces
		Total		5 spaces
Retail	711 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	2 spaces
		Short-Term	3 plus 0.3 spaces / 100 sqm GFA	6 spaces
		Total		8 spaces
<b>TOTAL</b>				<b>216 spaces</b>

BLOCKS 1B & 1C - MARKET

Use / Type	# of Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Residential	713 units	Occupant	0.90 spaces per unit	642 spaces
		Visitor	0.10 spaces per unit	72 spaces
		Total	1.00 spaces per unit	714 spaces
Community	0 sm GFA	Total	--	0 spaces
Office	330 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	1 spaces
		Short-Term	3 plus 0.2 spaces / 100 sqm GFA	4 spaces
		Total		5 spaces
Retail	989 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	2 spaces
		Short-Term	3 plus 0.3 spaces / 100 sqm GFA	6 spaces
		Total		8 spaces
<b>TOTAL</b>				<b>727 spaces</b>

BLOCKS 2D & 2E - MARKET

Use / Type	# of Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Residential	505 units	Occupant	0.90 spaces per unit	455 spaces
		Visitor	0.10 spaces per unit	51 spaces
		Total	1.00 spaces per unit	506 spaces
Community	0 sm GFA	Total	--	0 spaces
Office	511 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	2 spaces
		Short-Term	3 plus 0.2 spaces / 100 sqm GFA	5 spaces
		Total		7 spaces
Retail	1532 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	4 spaces
		Short-Term	3 plus 0.3 spaces / 100 sqm GFA	8 spaces
		Total		12 spaces
<b>TOTAL</b>				<b>525 spaces</b>

BLOCKS 2F & 2G - TCHC / MARKET

Use / Type	# of Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Community	759 sm GFA	Total	--	0 spaces
Library	2277 sm GFA	Total	--	0 spaces
Culture	405 sm GFA	Total	--	0 spaces
Office	380 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	1 spaces
		Short-Term	3 plus 0.2 spaces / 100 sqm GFA	4 spaces
		Total		5 spaces
Retail	1139 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	3 spaces
		Short-Term	3 plus 0.3 spaces / 100 sqm GFA	7 spaces
		Total		10 spaces
<b>TOTAL</b>				<b>15 spaces</b>

BLOCKS 2H & 2I - TCHC

Use / Type	# of Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Residential	333 units	Occupant	0.90 spaces per unit	300 spaces
		Visitor	0.10 spaces per unit	34 spaces
		Total	1.00 spaces per unit	334 spaces
Community	572 sm GFA	Total	--	0 spaces
Office	376 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	1 spaces
		Short-Term	3 plus 0.2 spaces / 100 sqm GFA	4 spaces
		Total		5 spaces
Retail	1127 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	3 spaces
		Short-Term	3 plus 0.3 spaces / 100 sqm GFA	7 spaces
		Total		10 spaces
<b>TOTAL</b>				<b>349 spaces</b>

BLOCKS 3J & 3K - MARKET

Use / Type	# of Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Residential	657 units	Occupant	0.90 spaces per unit	592 spaces
		Visitor	0.10 spaces per unit	66 spaces
		Total	1.00 spaces per unit	658 spaces
Community	0 sm GFA	Total	--	0 spaces
Office	270 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	1 spaces
		Short-Term	3 plus 0.2 spaces / 100 sqm GFA	4 spaces
		Total		5 spaces
Retail	809 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	2 spaces
		Short-Term	3 plus 0.3 spaces / 100 sqm GFA	6 spaces
		Total		8 spaces
<b>TOTAL</b>				<b>671 spaces</b>

BLOCKS 4L & 4M - TCHC

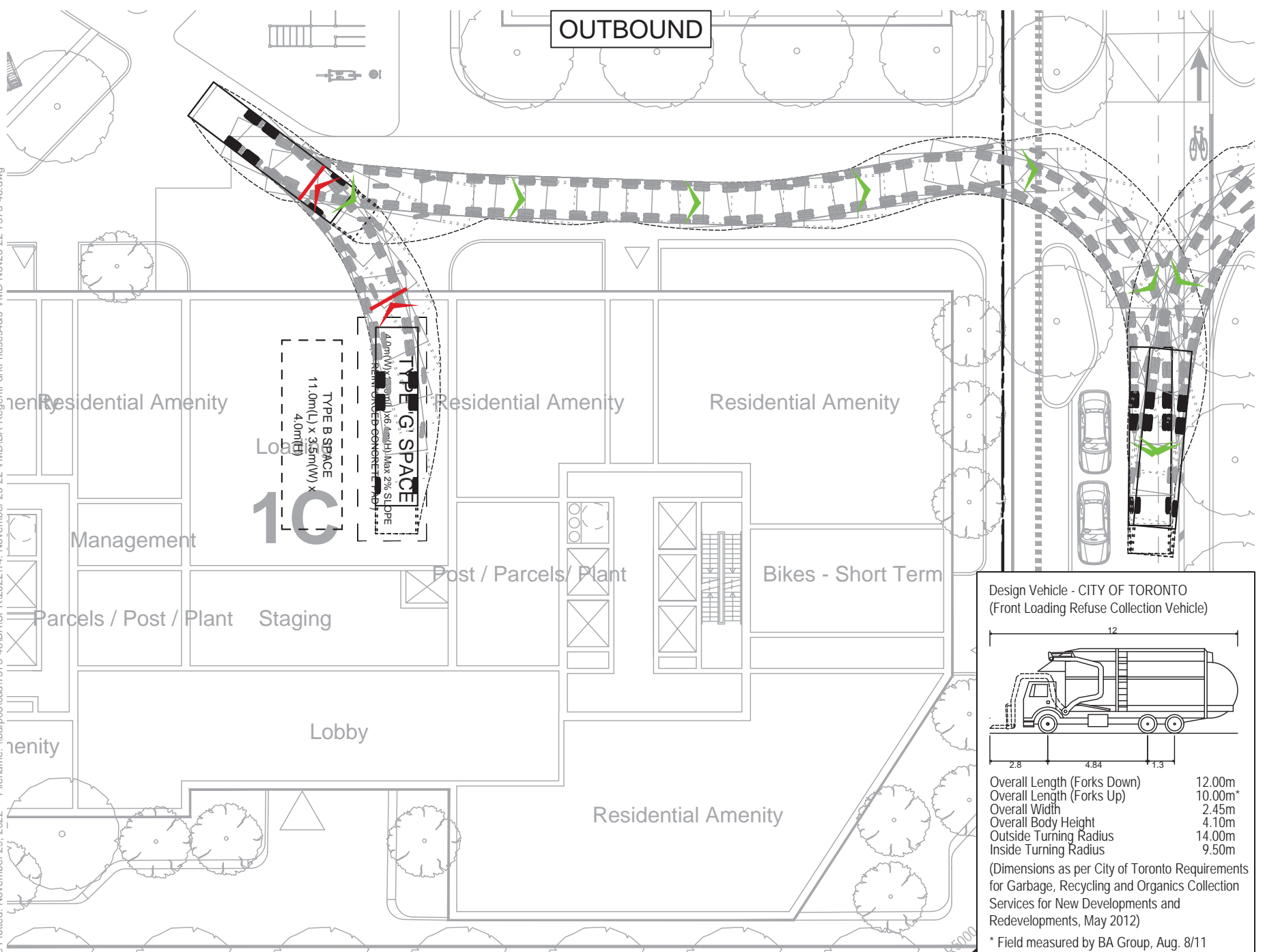
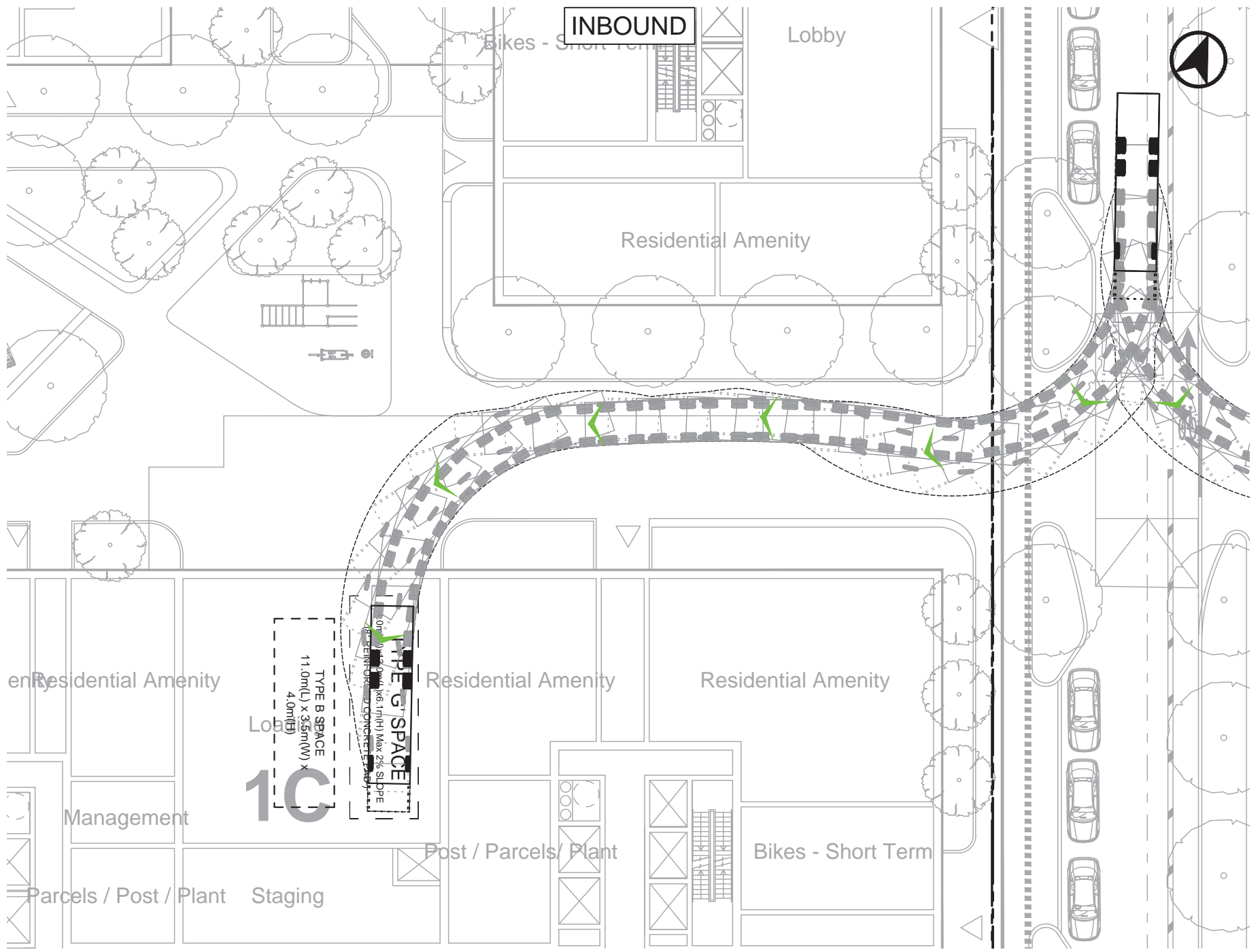
Use / Type	# of Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Residential	659 units	Occupant	0.90 spaces per unit	594 spaces
		Visitor	0.10 spaces per unit	66 spaces
		Total	1.00 spaces per unit	660 spaces
Community	916 sm GFA	Total	--	0 spaces
Office	199 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	1 spaces
		Short-Term	3 plus 0.2 spaces / 100 sqm GFA	4 spaces
		Total		5 spaces
Retail	596 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	2 spaces
		Short-Term	3 plus 0.3 spaces / 100 sqm GFA	5 spaces
		Total		7 spaces
<b>TOTAL</b>				<b>672 spaces</b>

SITE TOTAL

Use / Type	# of Units / GFA	Type	Minimum Parking Requirement (Ratio)	Minimum Parking Requirement (Spaces)
Residential	<b>3070 units</b>	Occupant	0.90 spaces per unit	1279 spaces
		Visitor	0.10 spaces per unit	144 spaces
		Total	1.00 spaces per unit	1423 spaces
Community	2117 sm GFA	Total	--	0 spaces
Library	2277 sm GFA	Total	--	0 spaces
Culture	405 sm GFA	Total	--	0 spaces
Office	2301 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	5 spaces
		Short-Term	3 plus 0.2 spaces / 100 sqm GFA	17 spaces
		Total		22 spaces
Retail	6902 sm GFA	Long-Term	0.2 spaces / 100 sqm GFA	11 spaces
		Short-Term	3 plus 0.3 spaces / 100 sqm GFA	27 spaces
		Total		38 spaces
Long-Term Subtotal				1295 spaces
Short-Term Subtotal				185 spaces
<b>TOTAL</b>				<b>1483 spaces</b>

## Appendix E: Vehicle Manoeuvring Diagrams





**Design Vehicle - CITY OF TORONTO**  
(Front Loading Refuse Collection Vehicle)

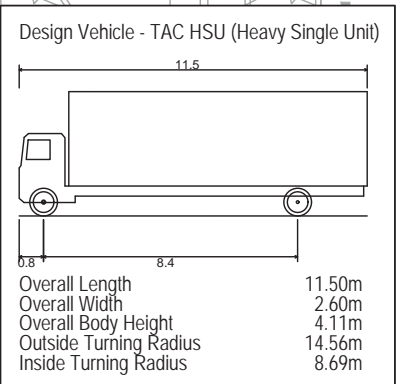
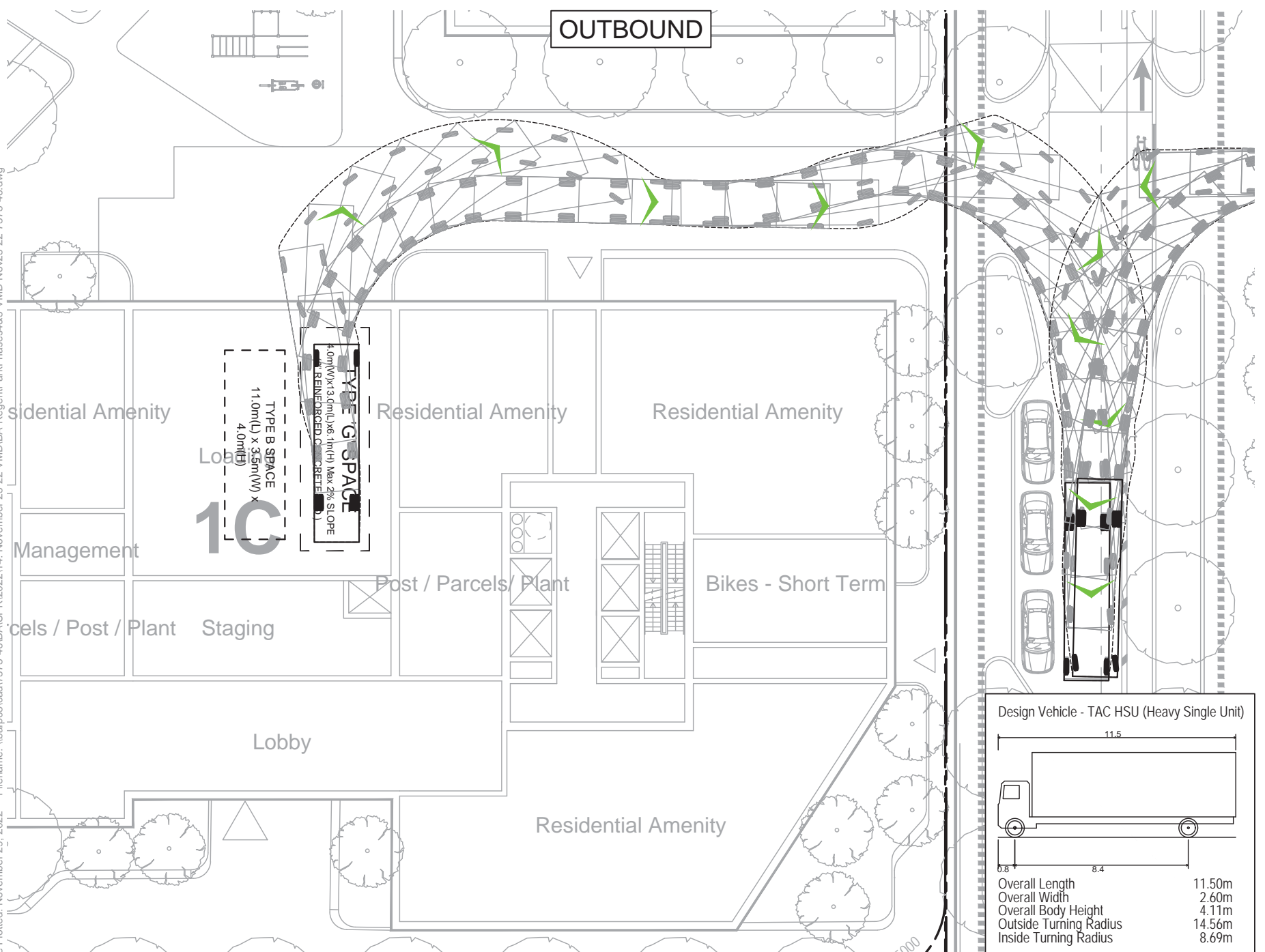
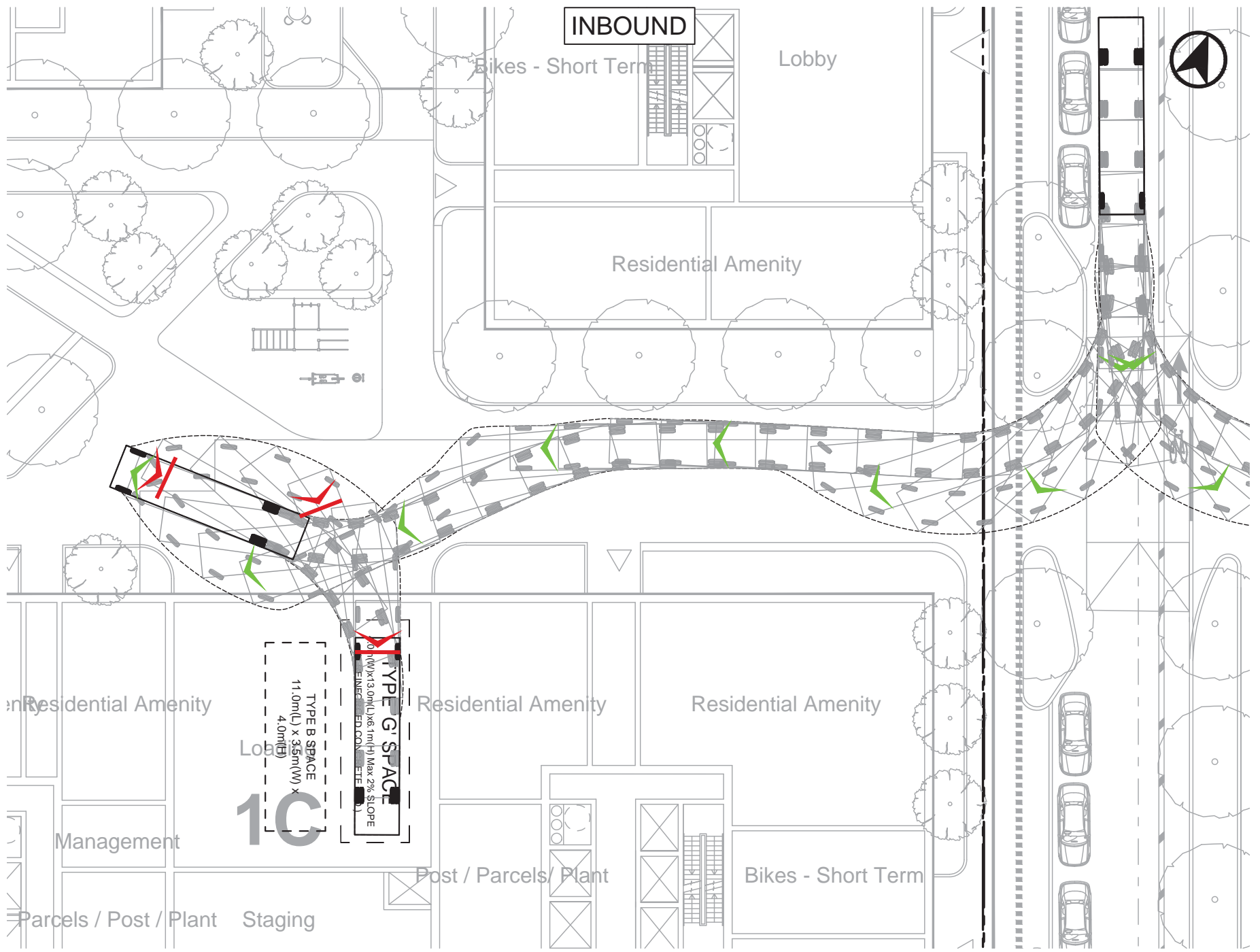
Overall Length (Forks Down)	12.00m
Overall Length (Forks Up)	10.00m*
Overall Width	2.45m
Overall Body Height	4.10m
Outside Turning Radius	14.00m
Inside Turning Radius	9.50m

(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments, May 2012)  
\* Field measured by BA Group, Aug. 8/11

Date Plotted: November 29, 2022 File name: \\baftp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

	<b>REGENT PARK PHASES 4 &amp; 5</b> VEHICULAR MANOEUVRING DIAGRAM Buildings 1B & 1C City of Toronto Front Loading Garbage Truck	Project: Regent Park Phases 4 & 5 Project No. 7575-46 Date: November 29, 2022 Revised: --	Scale: 1:300 
		Drawing No. <b>VMD-01A</b>	





Date Plotted: November 29, 2022 File name: \\bafp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

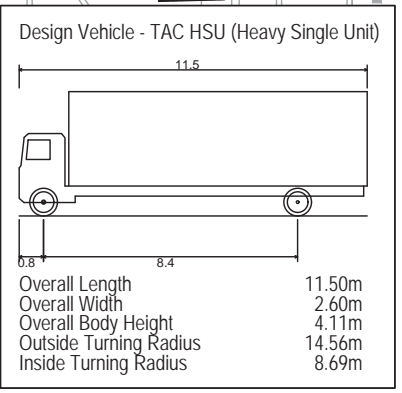
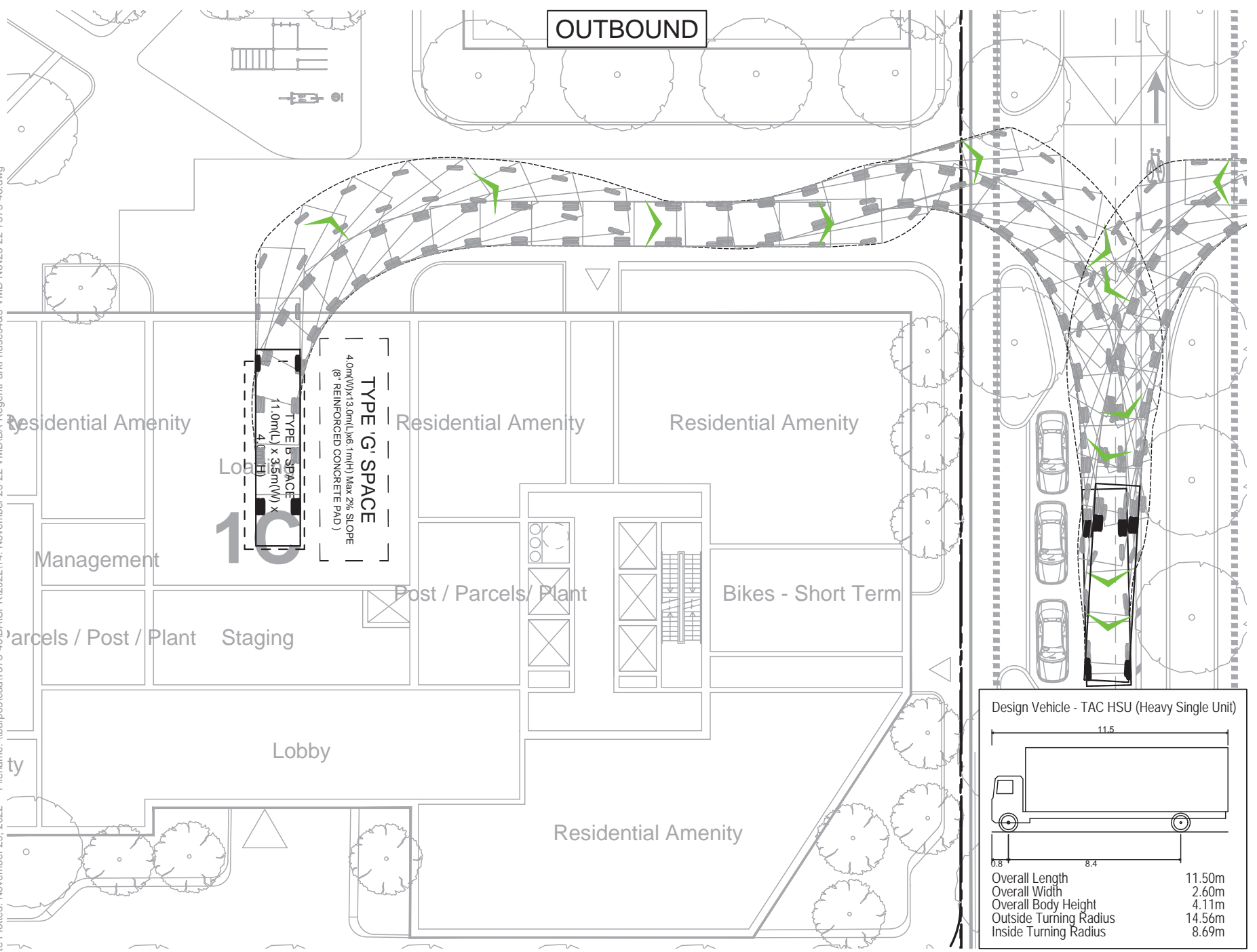
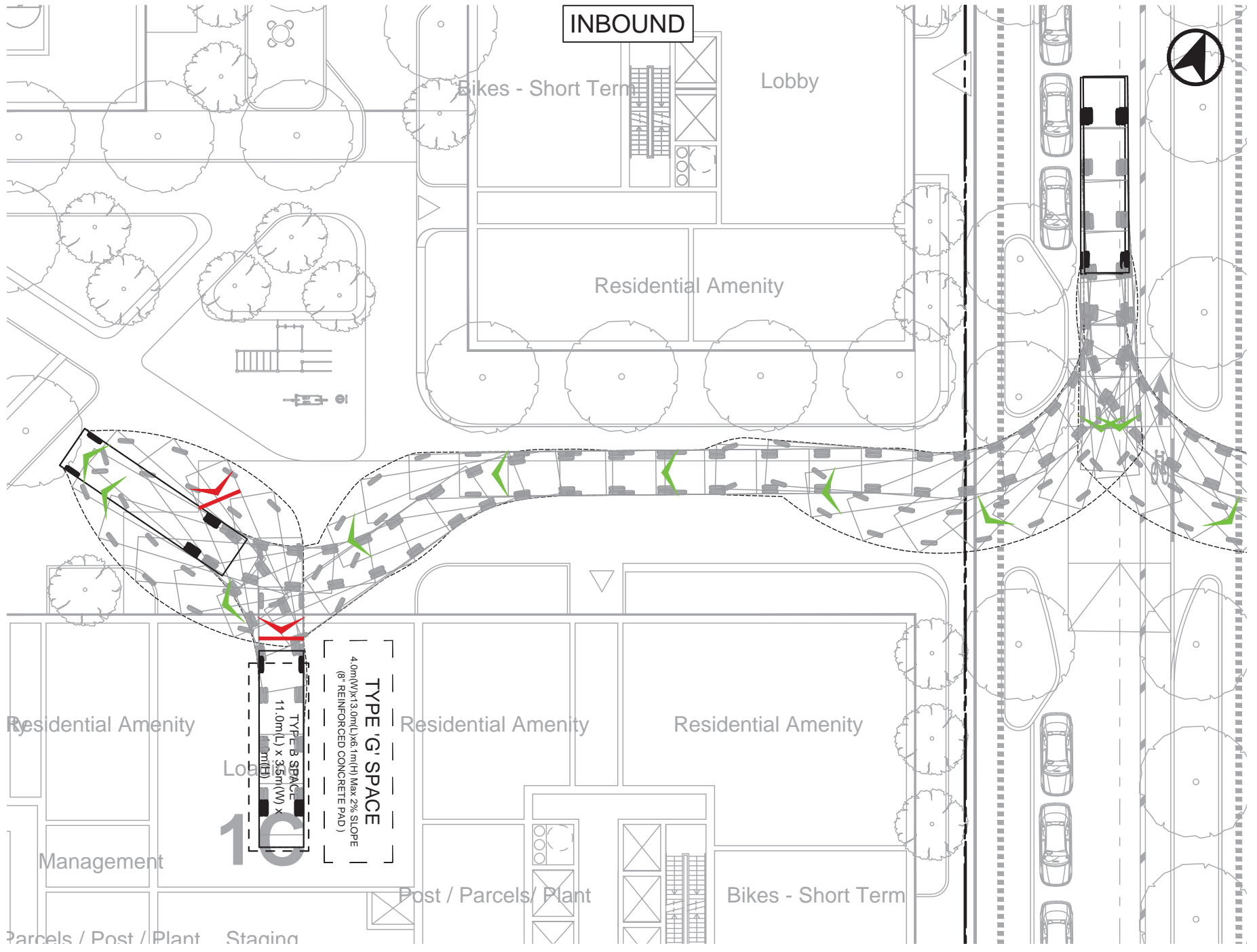


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Buildings 1B & 1C  
 TAC Heavy Single Unit (HSU) Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

Scale: 1:300

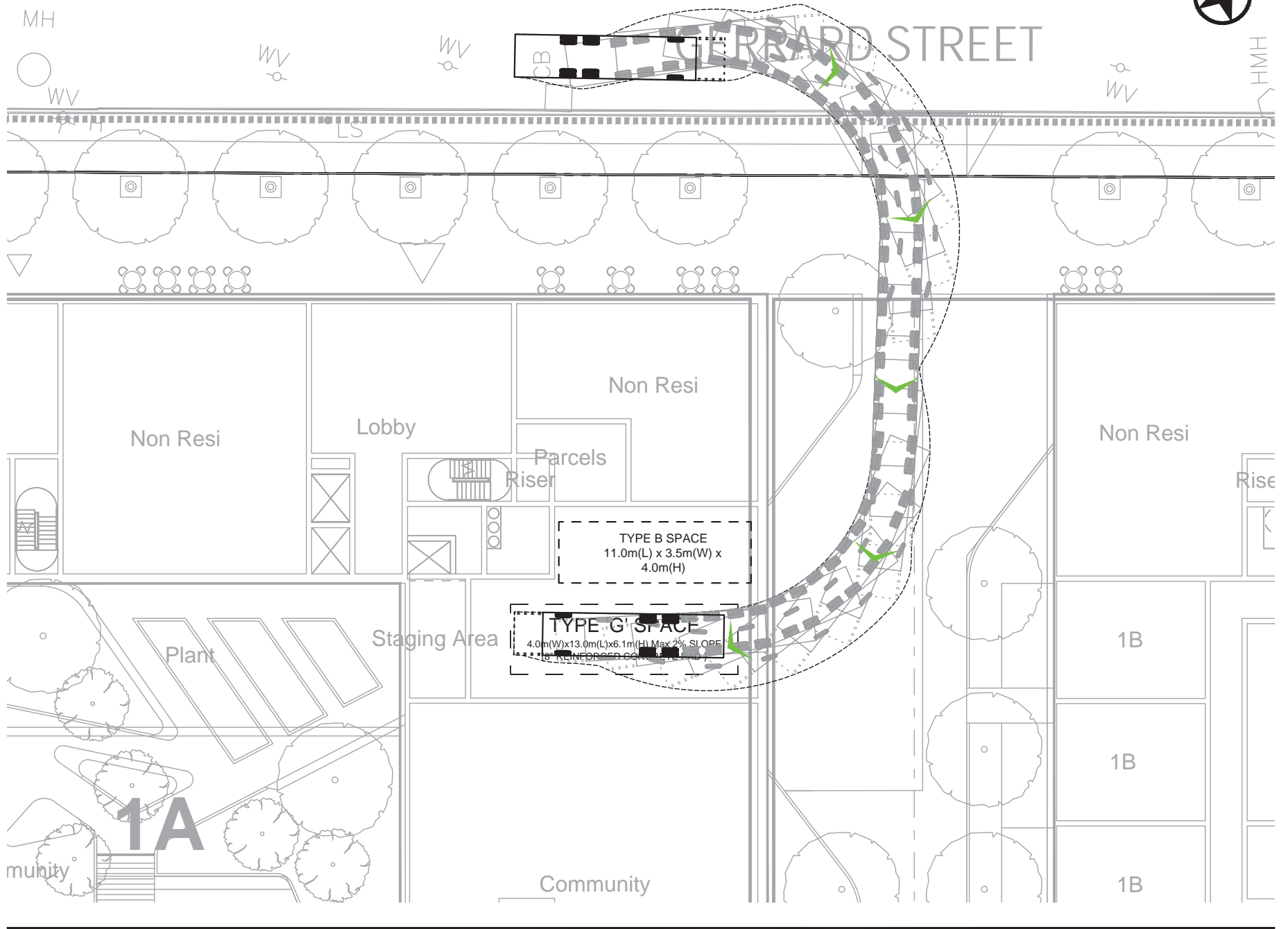
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	<b>REGENT PARK PHASES 4 &amp; 5</b> VEHICULAR MANOEUVRING DIAGRAM Buildings 1B & 1C TAC Heavy Single Unit (SU) Truck	Project: Regent Park Phases 4 & 5 Project No. 7575-46 Date: November 29, 2022 Revised: --	Scale: 1:300 
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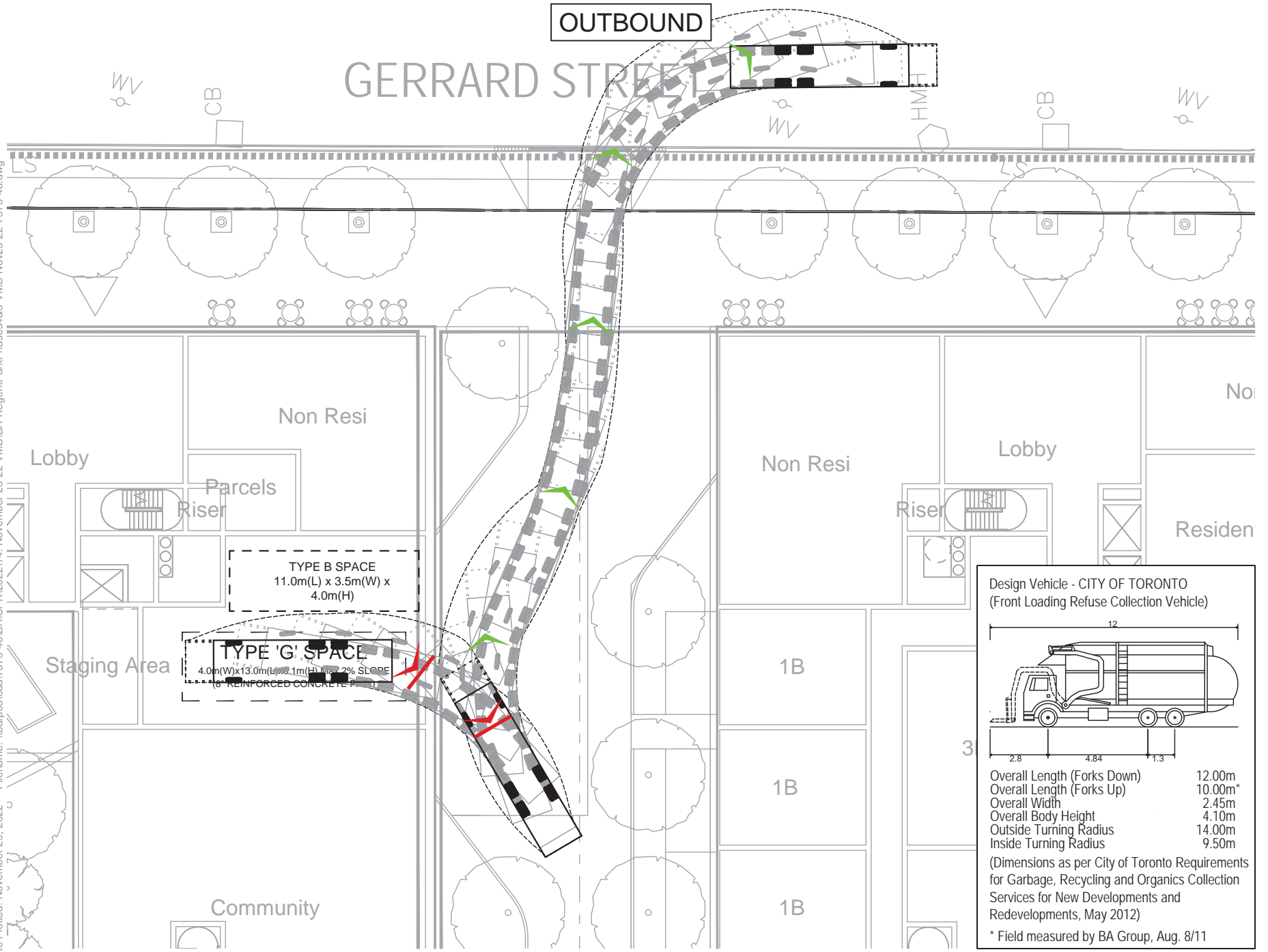
INBOUND



OUTBOUND

GERRARD STREET

Date Plotted: November 29, 2022. Filename: \\bafp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg



Design Vehicle - CITY OF TORONTO  
(Front Loading Refuse Collection Vehicle)

Overall Length (Forks Down)	12.00m
Overall Length (Forks Up)	10.00m*
Overall Width	2.45m
Overall Body Height	4.10m
Outside Turning Radius	14.00m
Inside Turning Radius	9.50m

(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments, May 2012)  
\* Field measured by BA Group, Aug. 8/11

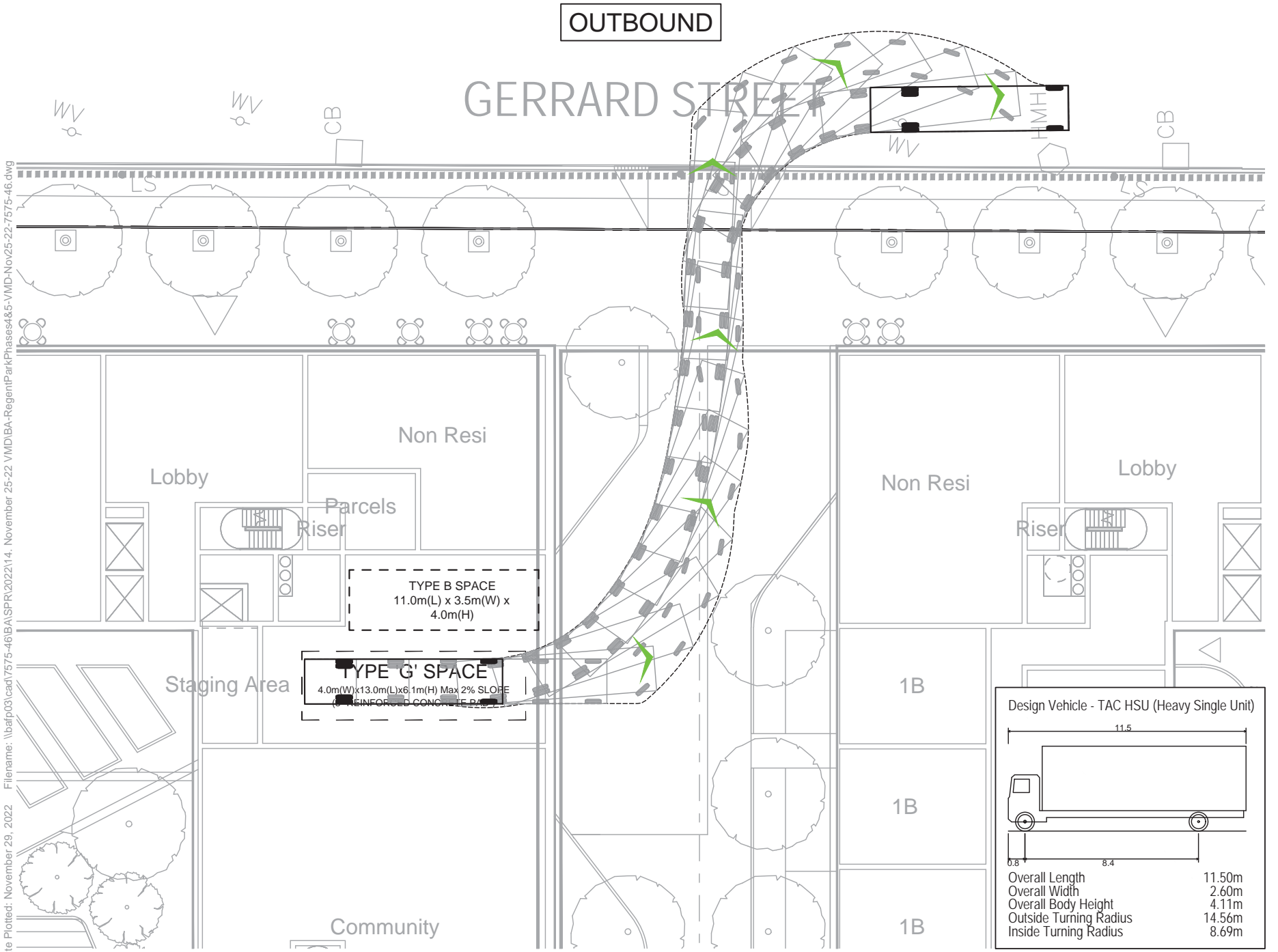
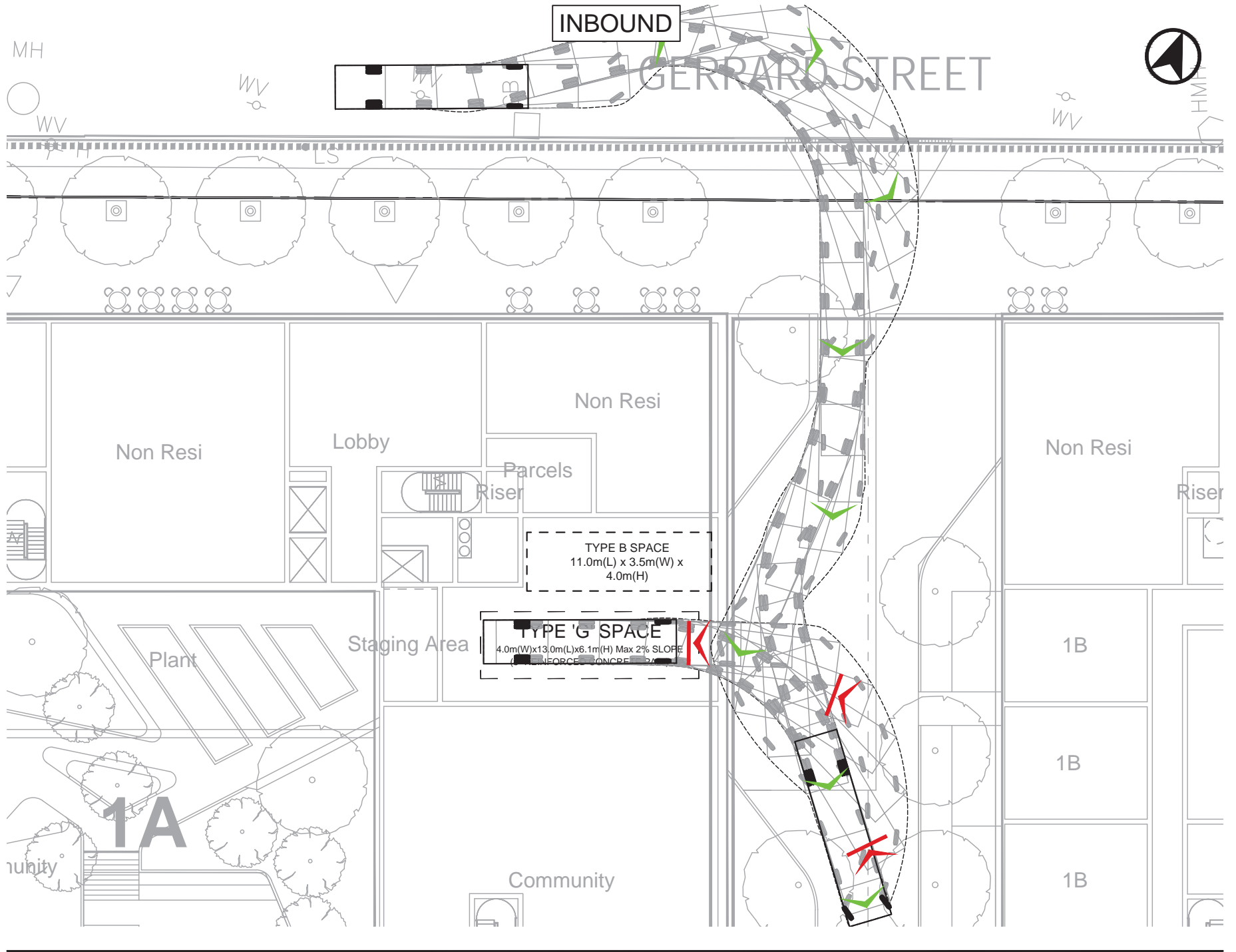


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Building 1A  
 City of Toronto Front Loading Garbage Truck

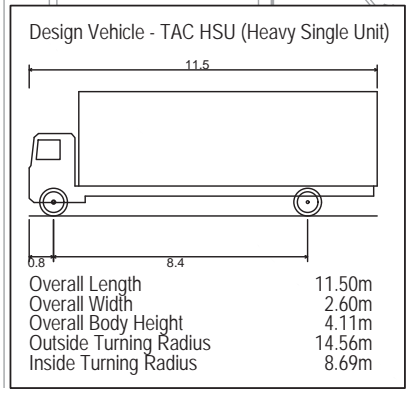
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 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

Scale: 1:300

Drawing No. **VMD-02A**



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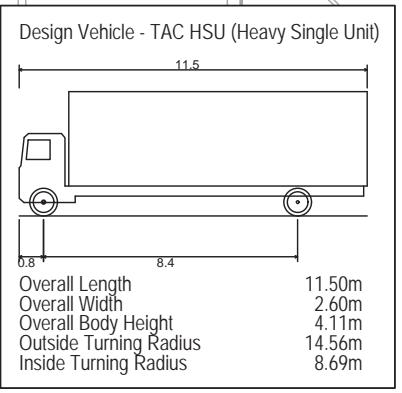
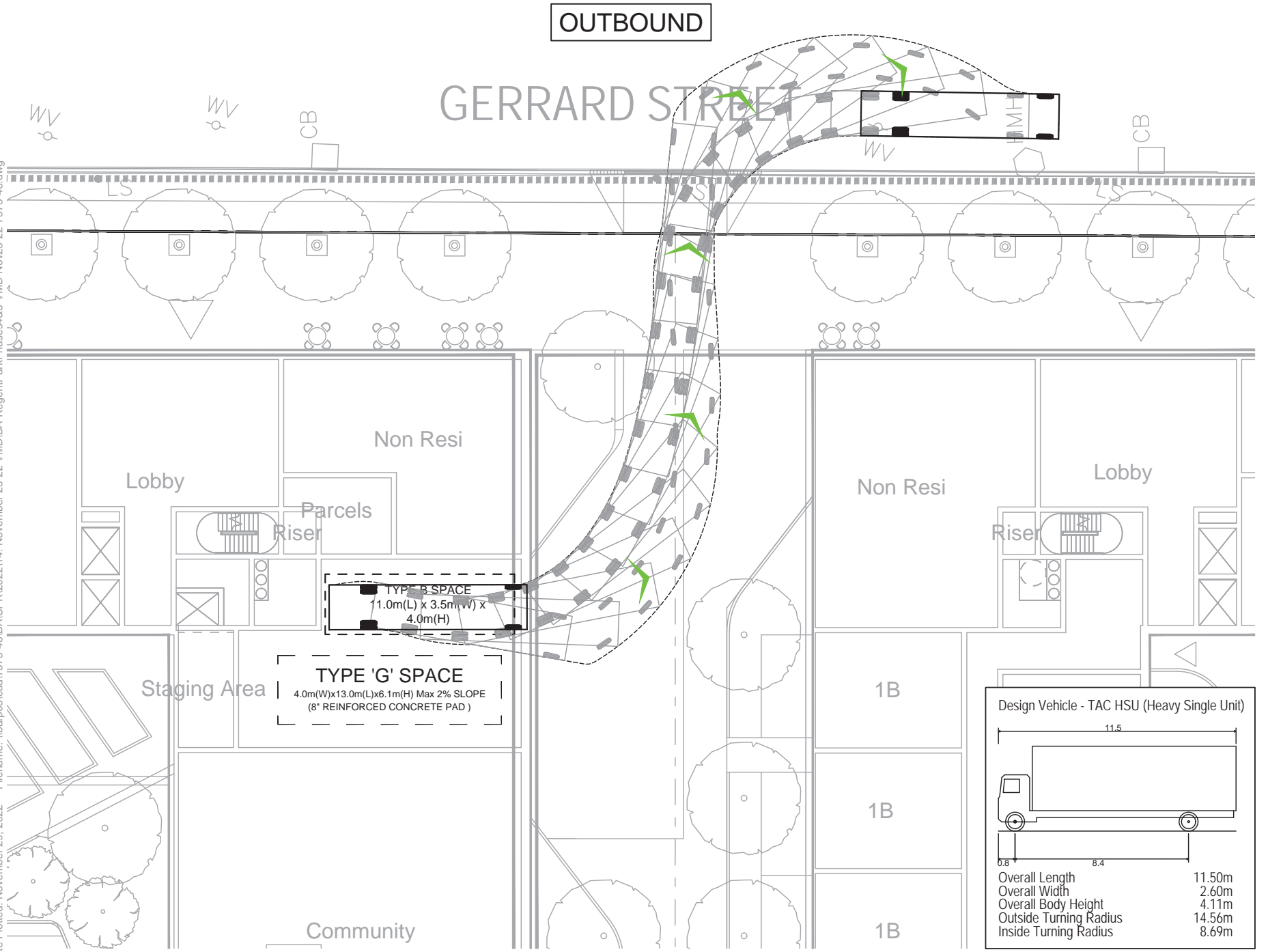
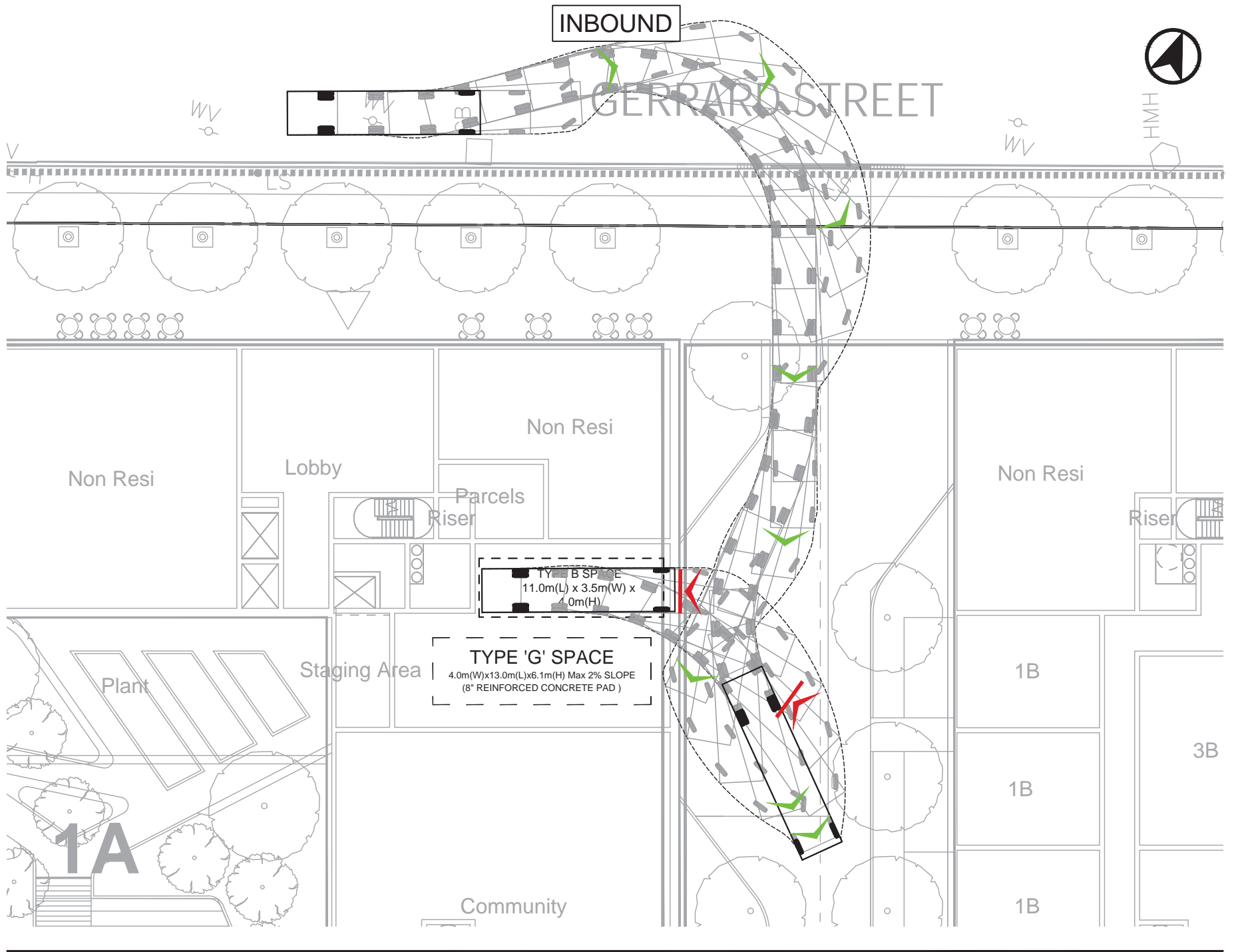


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Building 1A  
 TAC Heavy Single Unit (HSU) Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

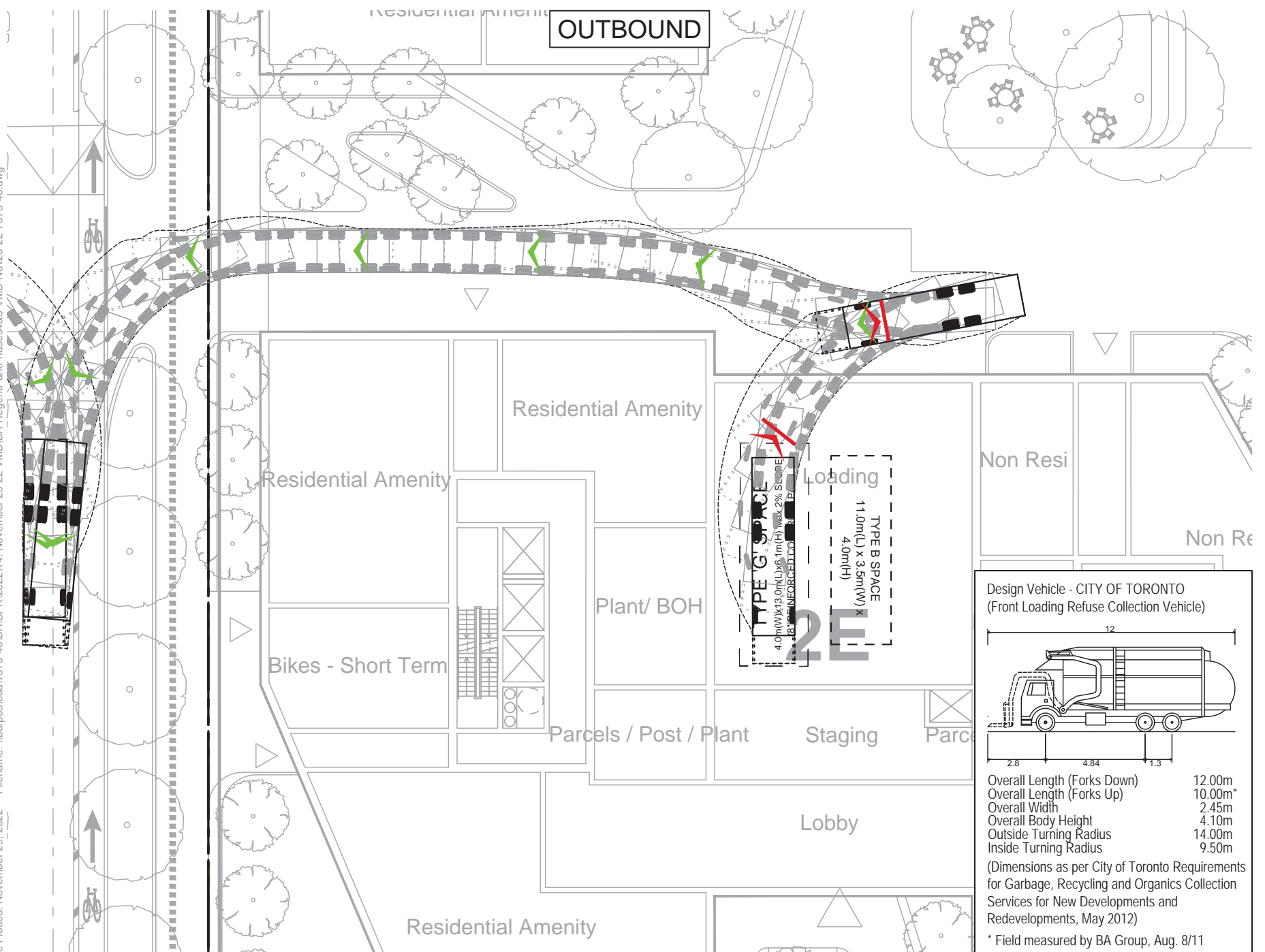
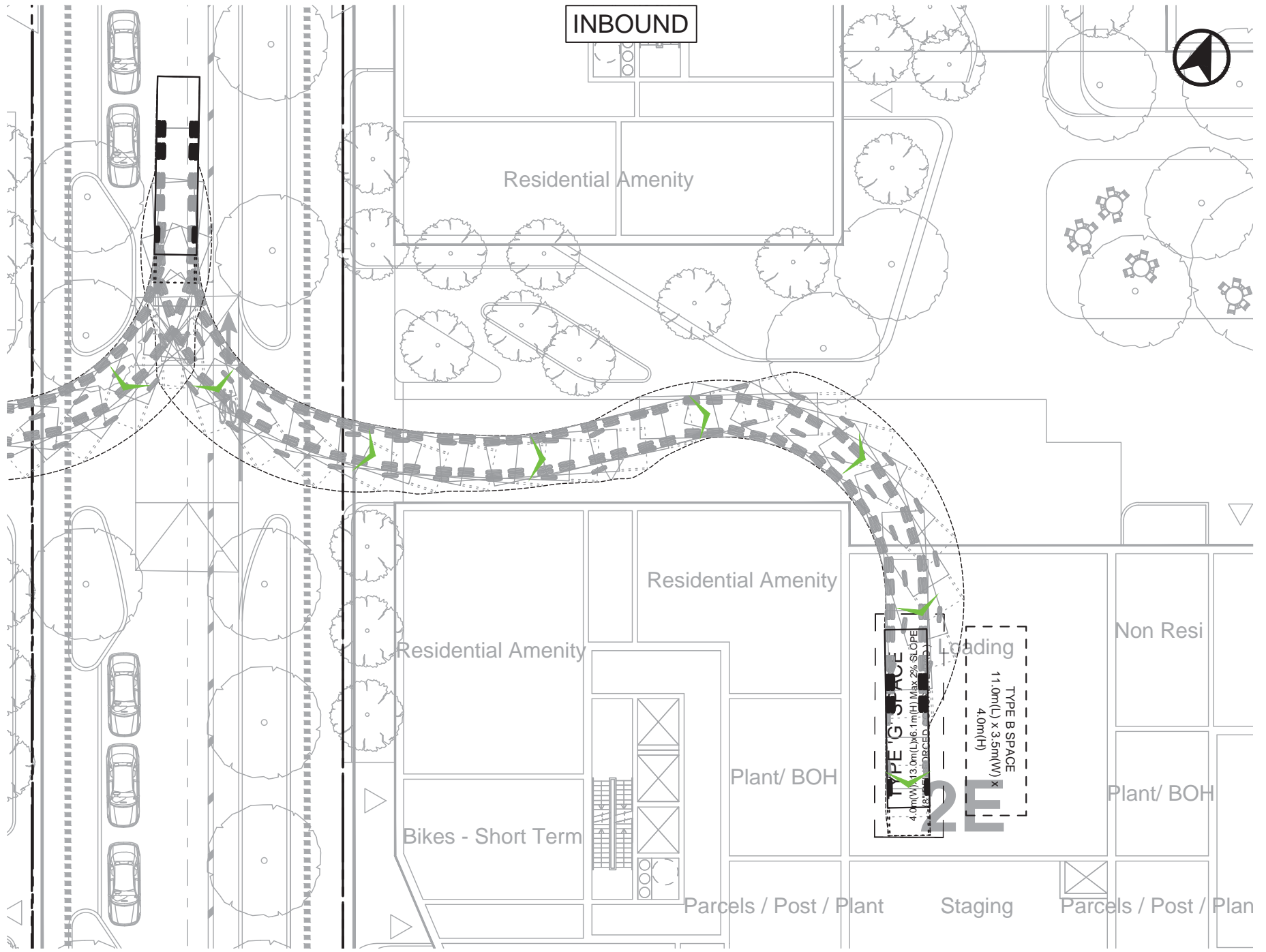
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Drawing No. **VMD-02B**



Date Plotted: November 29, 2022. Filename: \\bafp03\cad\7575-46\BA\SPR\2022\14 - November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

	<b>REGENT PARK PHASES 4 &amp; 5</b> VEHICULAR MANOEUVRING DIAGRAM Buildings 1A TAC Heavy Single Unit (HSU) Truck	Project: Regent Park Phases 4 & 5 Project No. 7575-46 Date: November 29, 2022 Revised: --	Scale 1:300
		Drawing No. <b>VMD-02C</b>	



Design Vehicle - CITY OF TORONTO  
(Front Loading Refuse Collection Vehicle)

Overall Length (Forks Down)	12.00m
Overall Length (Forks Up)	10.00m*
Overall Width	2.45m
Overall Body Height	4.10m
Outside Turning Radius	14.00m
Inside Turning Radius	9.50m

(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments, May 2012)  
\* Field measured by BA Group, Aug. 8/11

Date Plotted: November 29, 2022 File name: \\baftp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

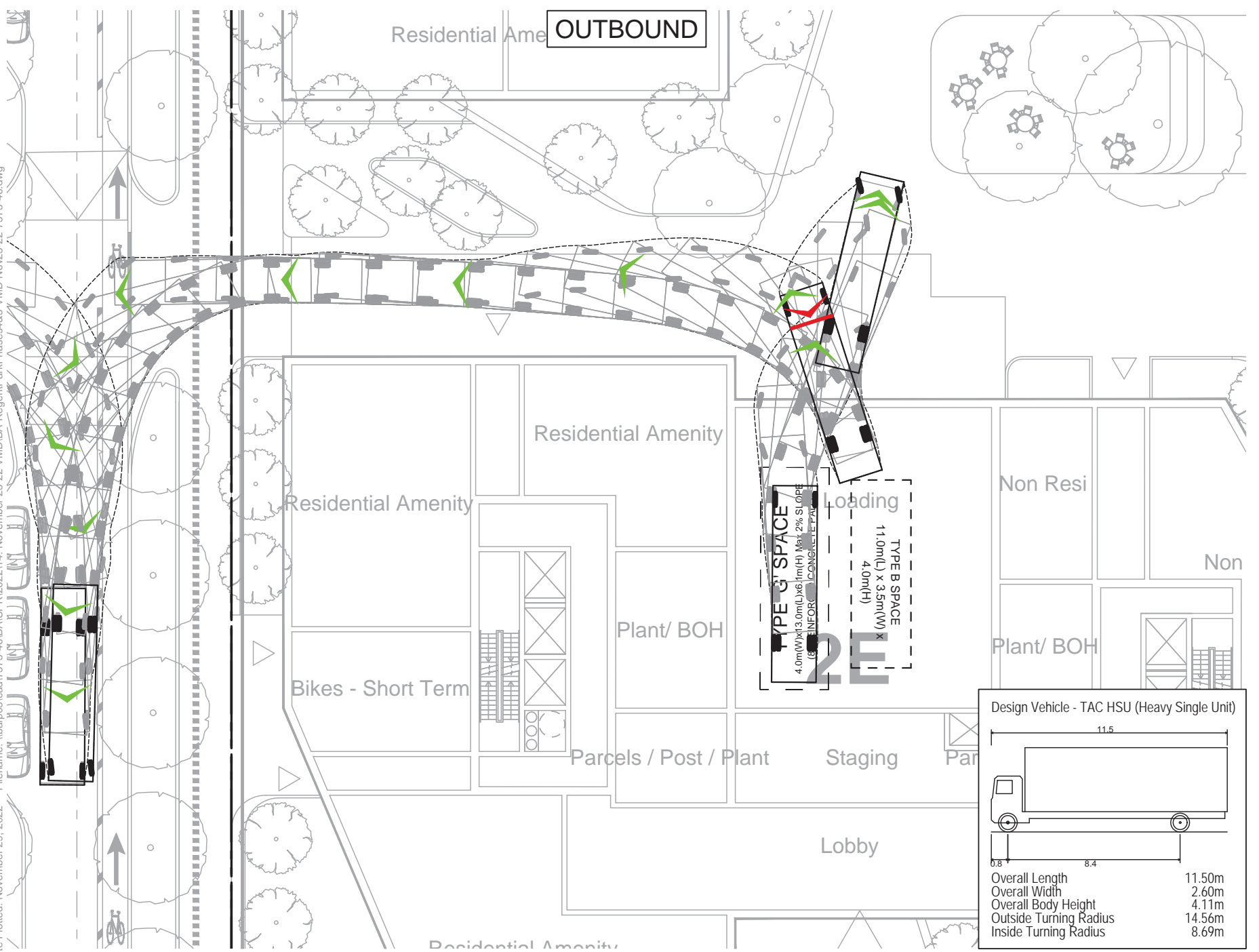
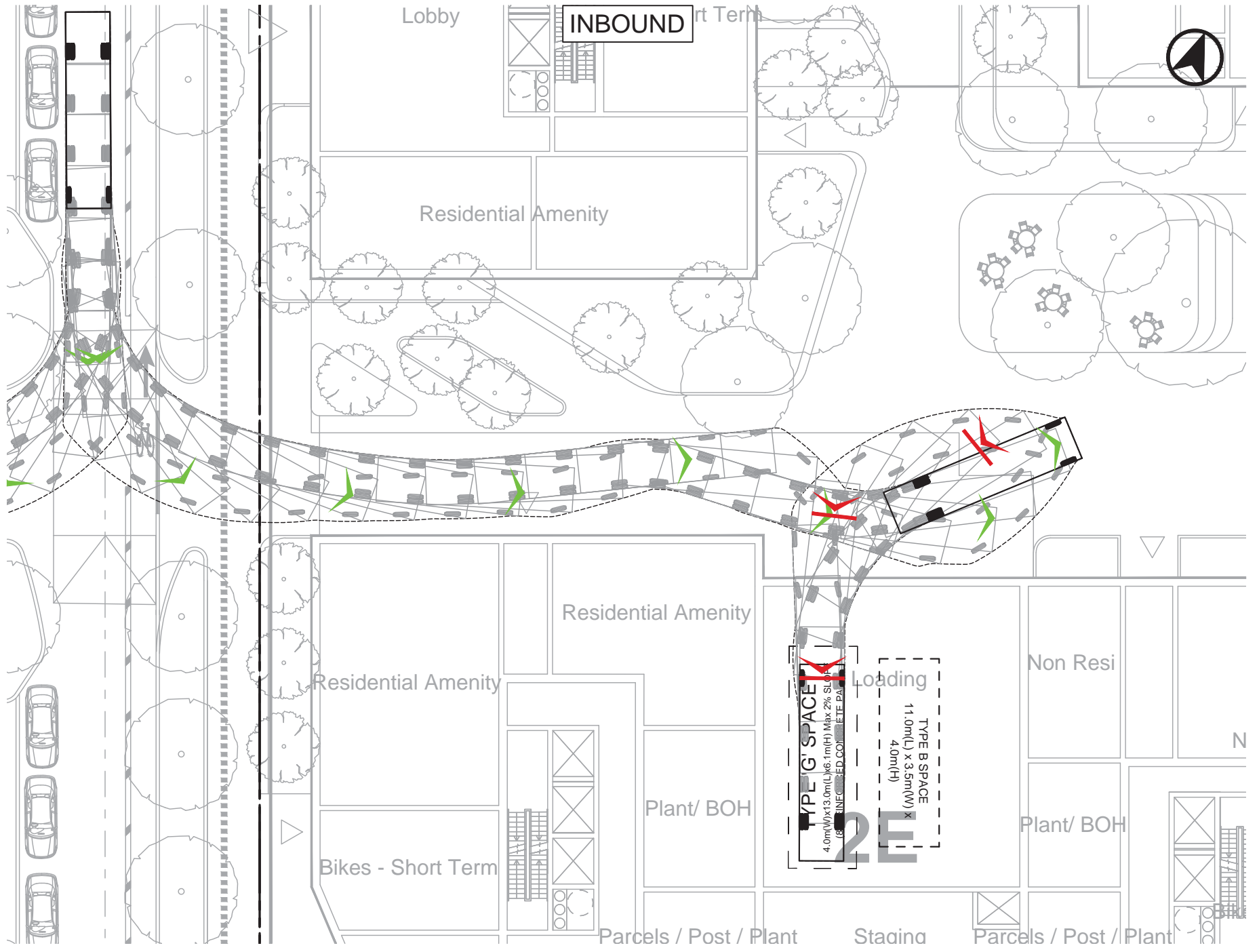


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Buildings 2D & 2E  
 City of Toronto Front Loading Garbage Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

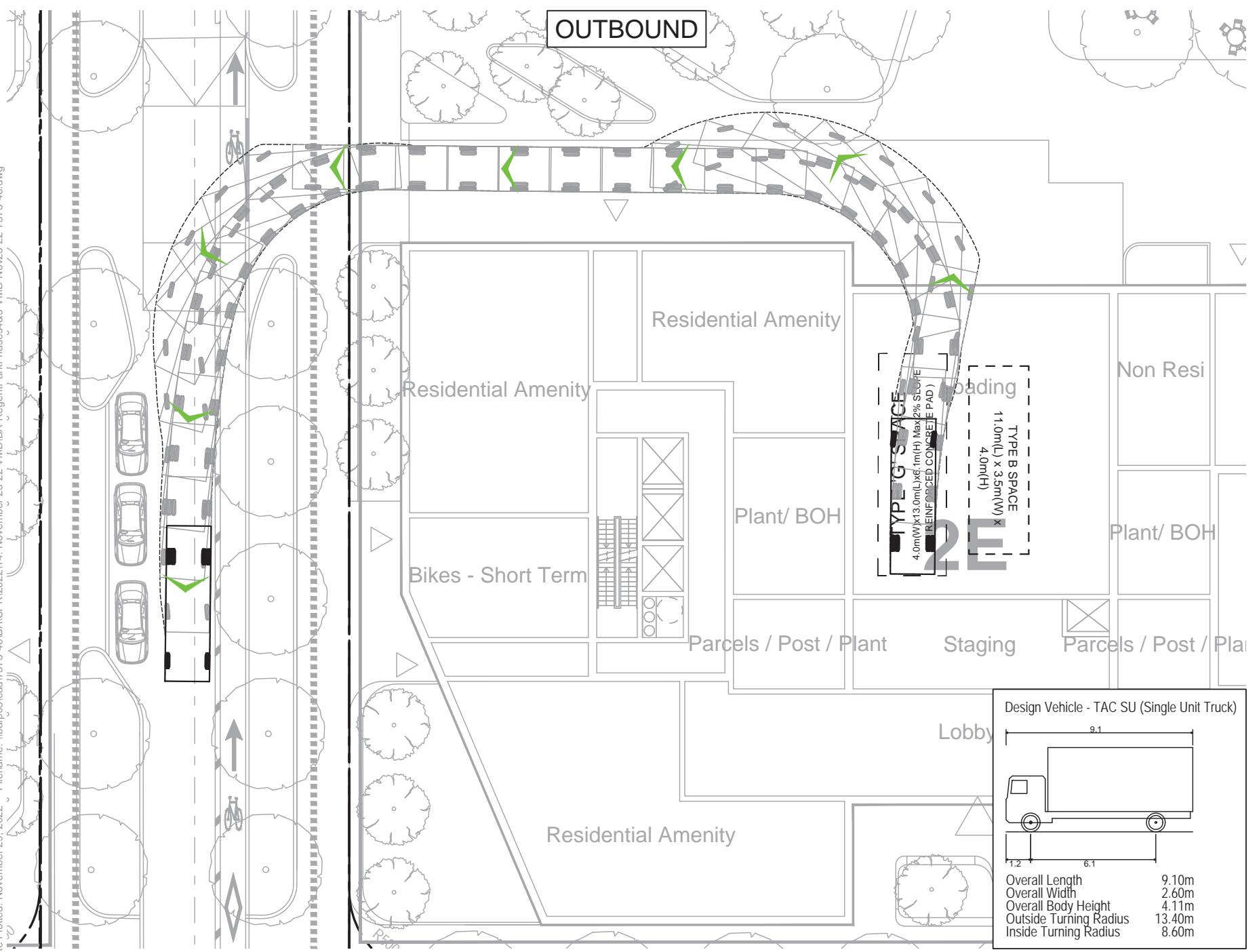
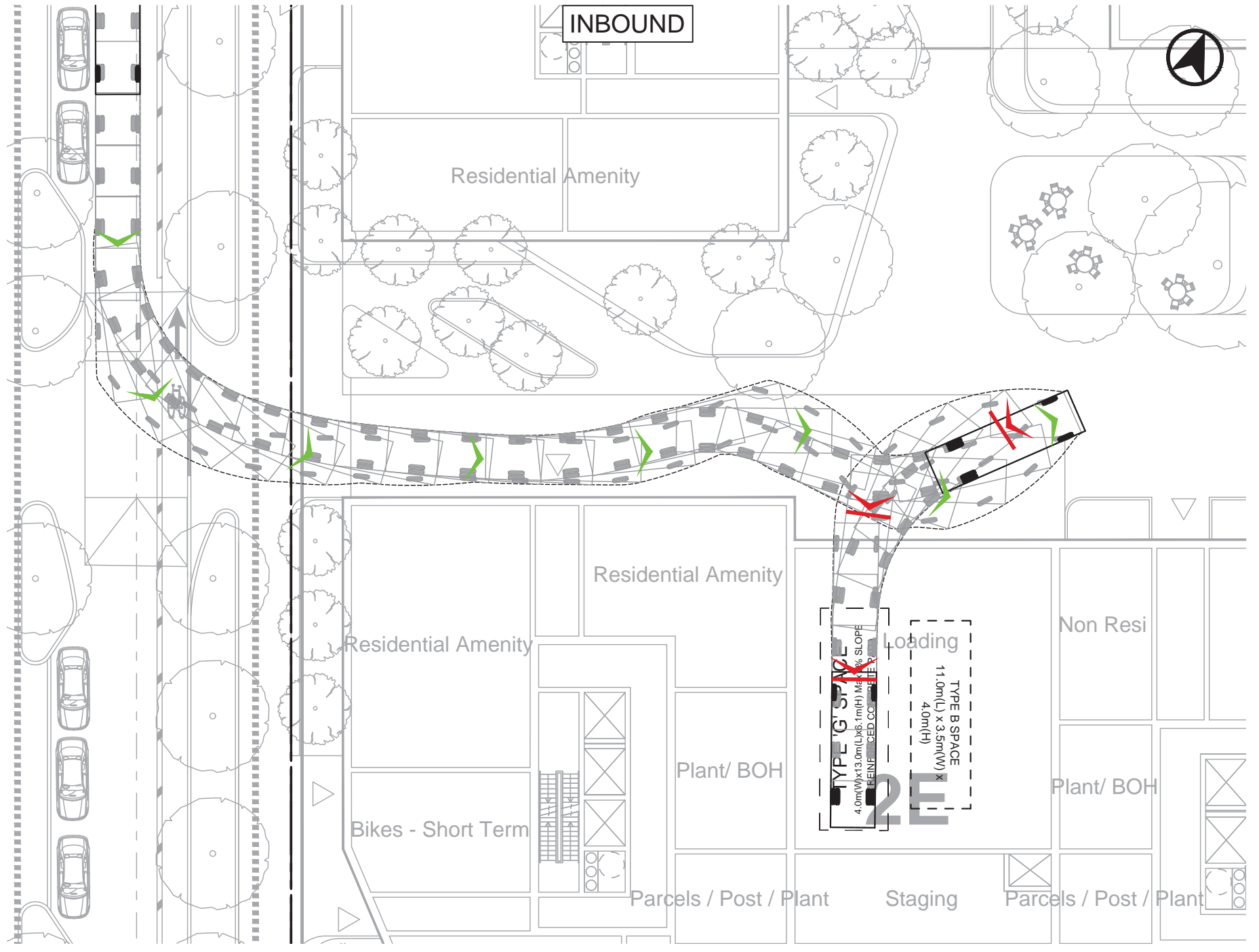
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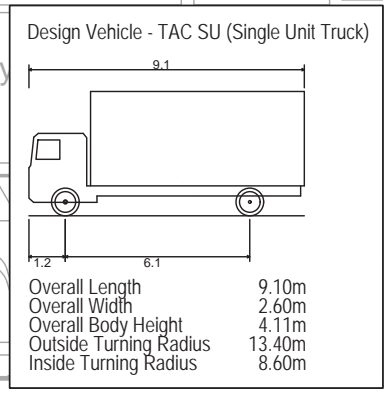


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	<b>REGENT PARK PHASES 4 &amp; 5</b> VEHICULAR MANOEUVRING DIAGRAM Buildings 2D & 2E TAC Heavy Single Unit (HSU) Truck	Project: Regent Park Phases 4 & 5 Project No. 7575-46 Date: November 29, 2022 Revised: --	Scale 1:300
		Drawing No. <b>VMD-03B</b>	



Date Plotted: November 29, 2022 File name: \\bafp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg



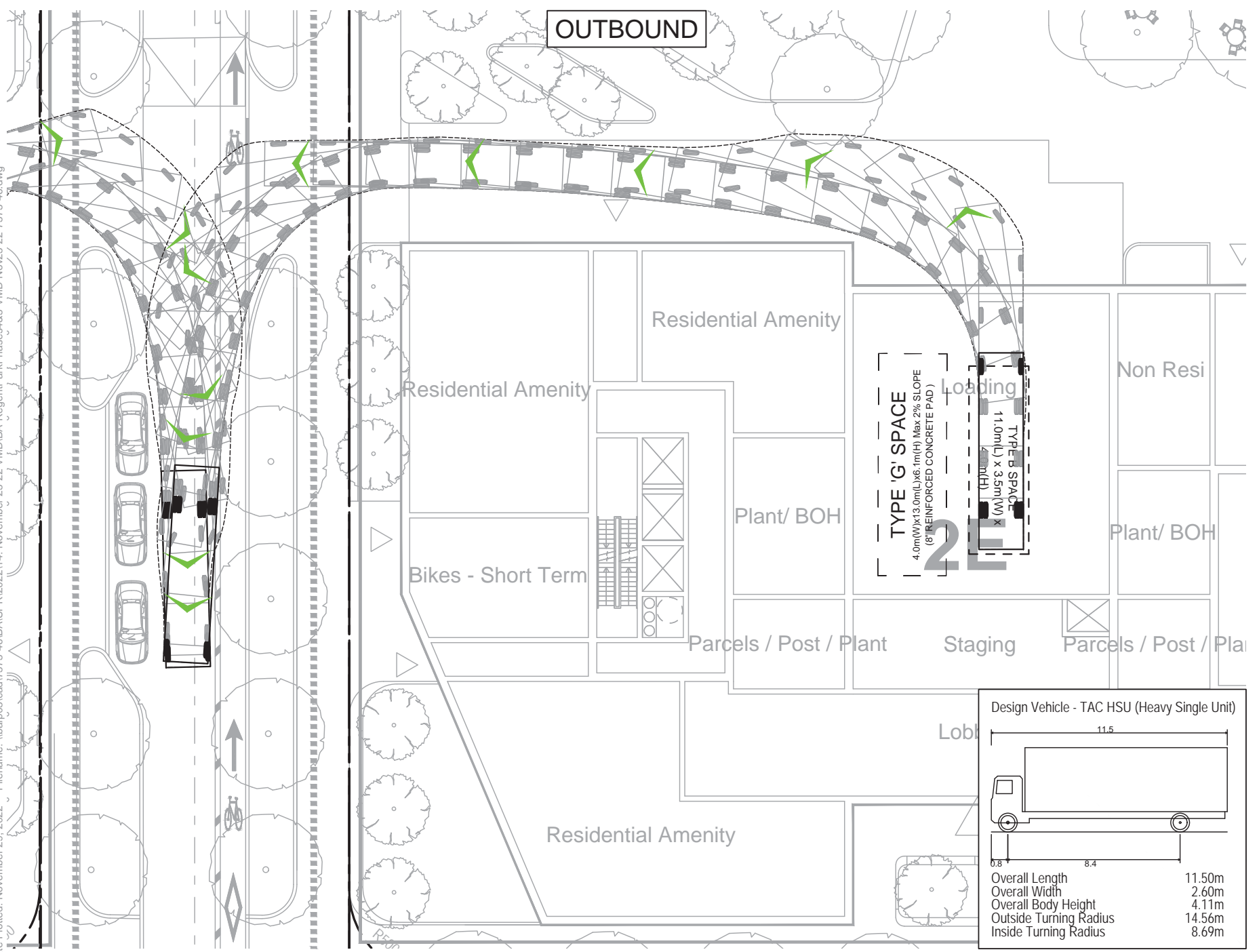
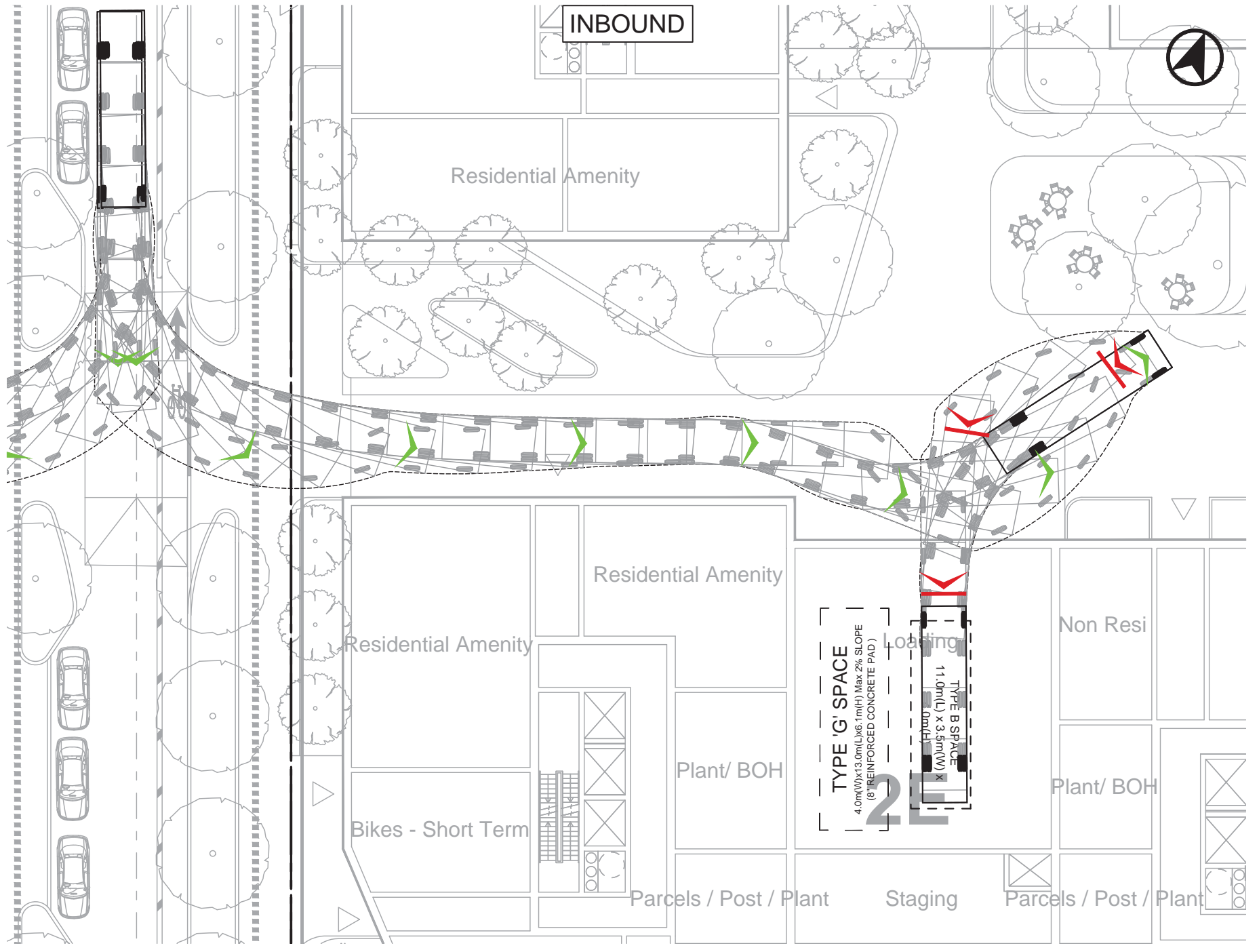
**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Buildings 2D & 2E  
 TAC Single Unit (SU) Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

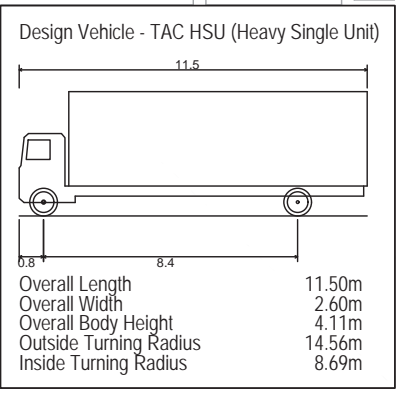
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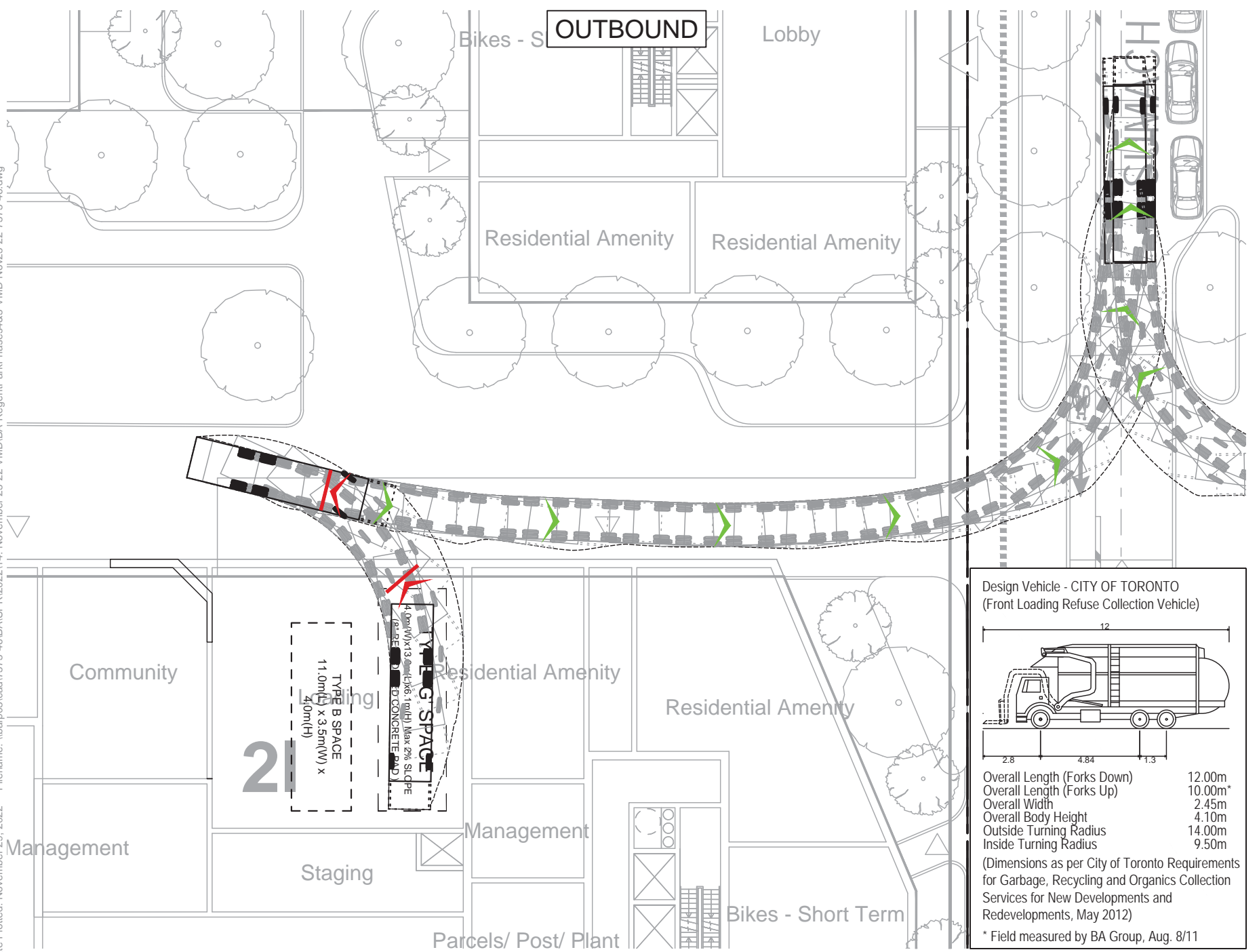
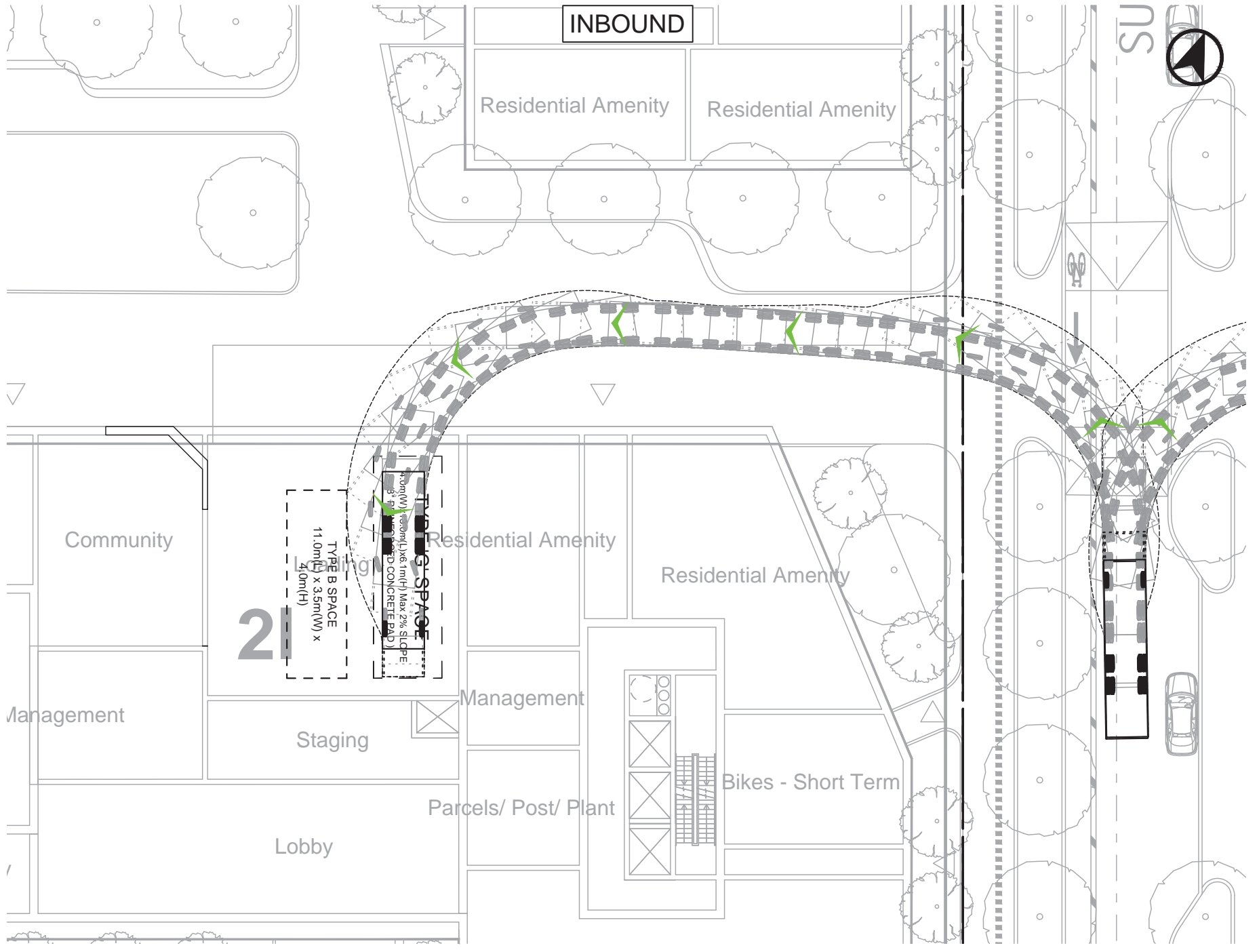


**REGENT PARK PHASES 4 & 5**  
VEHICULAR MANOEUVRING DIAGRAM  
Buildings 2D & 2E  
TAC Heavy Single Unit (HSU) Truck

Project: Regent Park Phases 4 & 5  
Project No. 7575-46  
Date: November 29, 2022  
Revised: --

Scale 1:300

Drawing No. **VMD-03C**



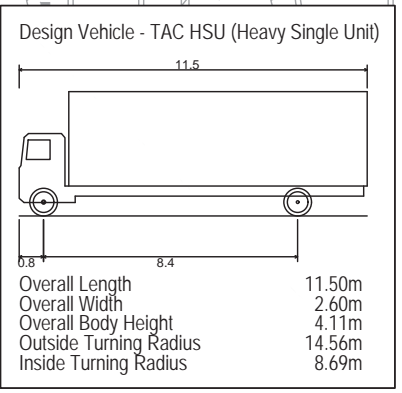
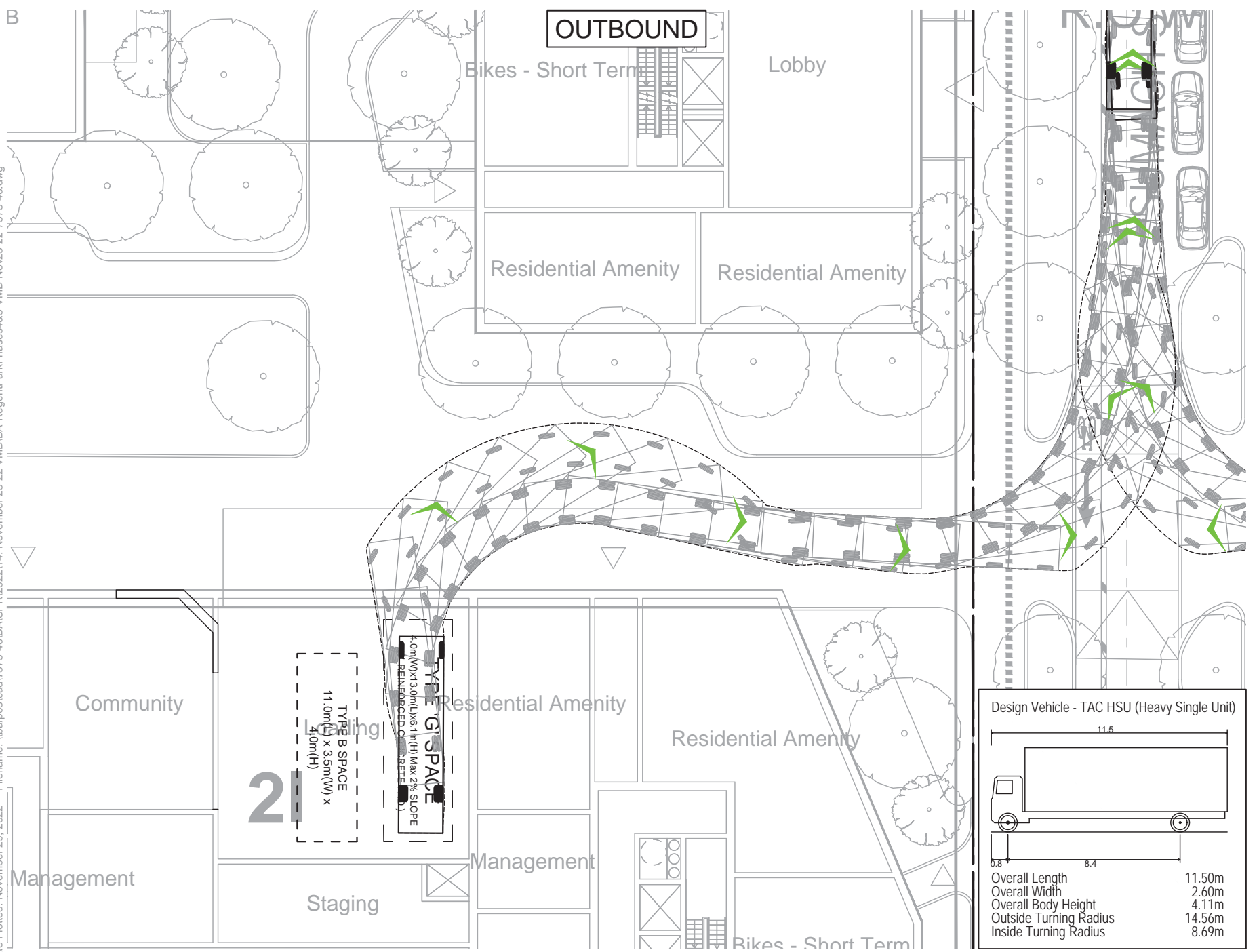
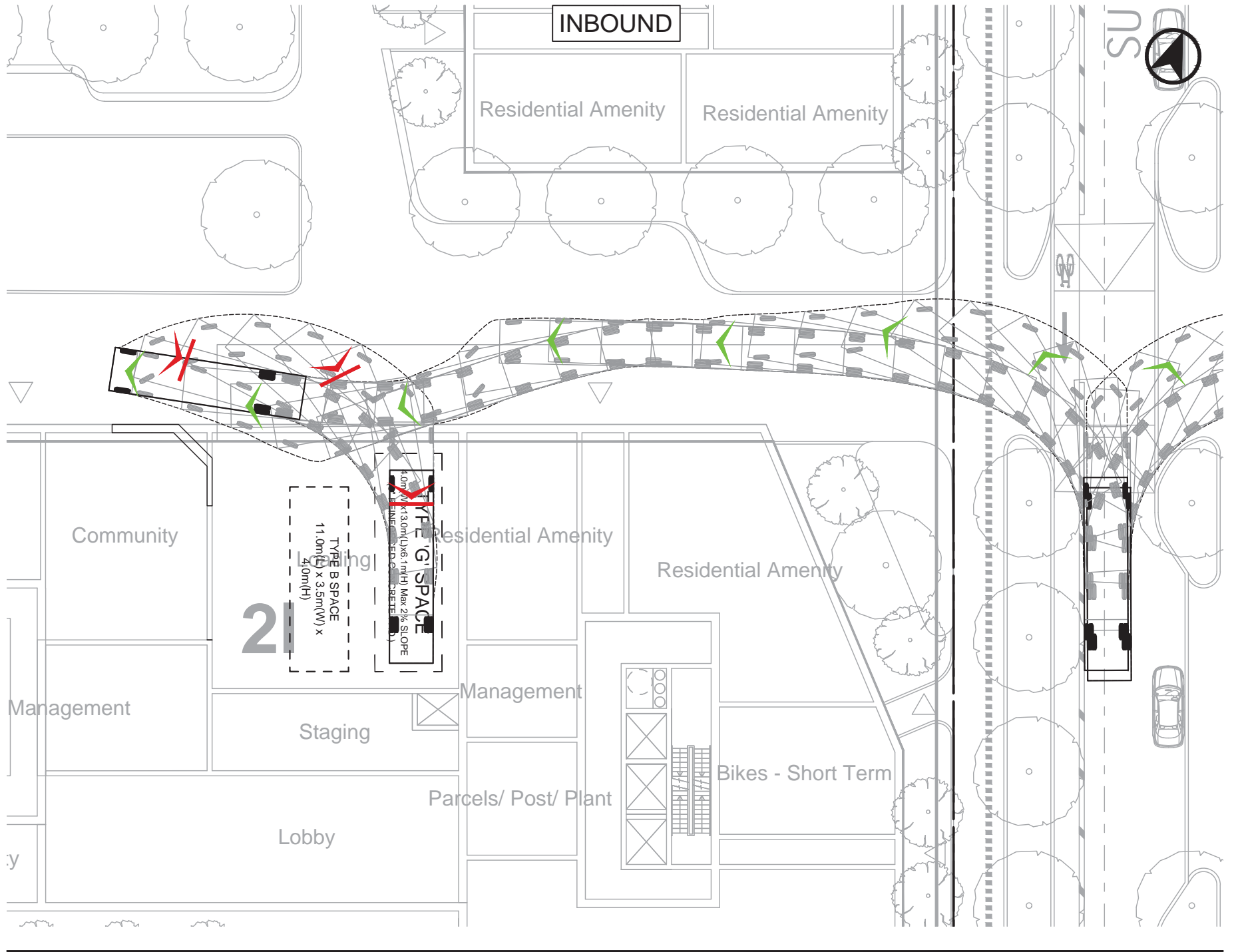
Design Vehicle - CITY OF TORONTO  
(Front Loading Refuse Collection Vehicle)

Overall Length (Forks Down)	12.00m
Overall Length (Forks Up)	10.00m*
Overall Width	2.45m
Overall Body Height	4.10m
Outside Turning Radius	14.00m
Inside Turning Radius	9.50m

(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments, May 2012)  
\* Field measured by BA Group, Aug. 8/11

Date Plotted: November 29, 2022 File name: \\baftp03\cad\7575-46\BA\SPR\2022\14 - November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

	<b>REGENT PARK PHASES 4 &amp; 5</b> VEHICULAR MANOEUVRING DIAGRAM Buildings 2H & 2I City of Toronto Front Loading Garbage Truck	Project: Regent Park Phases 4 & 5 Project No. 7575-46 Date: November 29, 2022 Revised: --	Scale: 1:300 
		Drawing No. <b>VMD-04A</b>	



Date Plotted: November 29, 2022 File name: \\ba\p03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

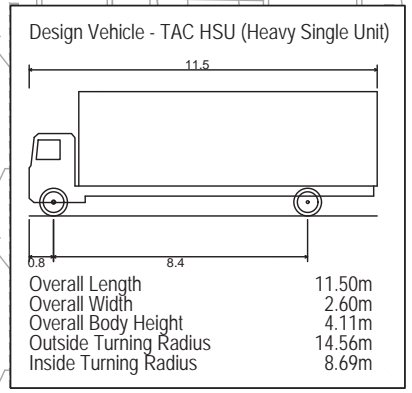
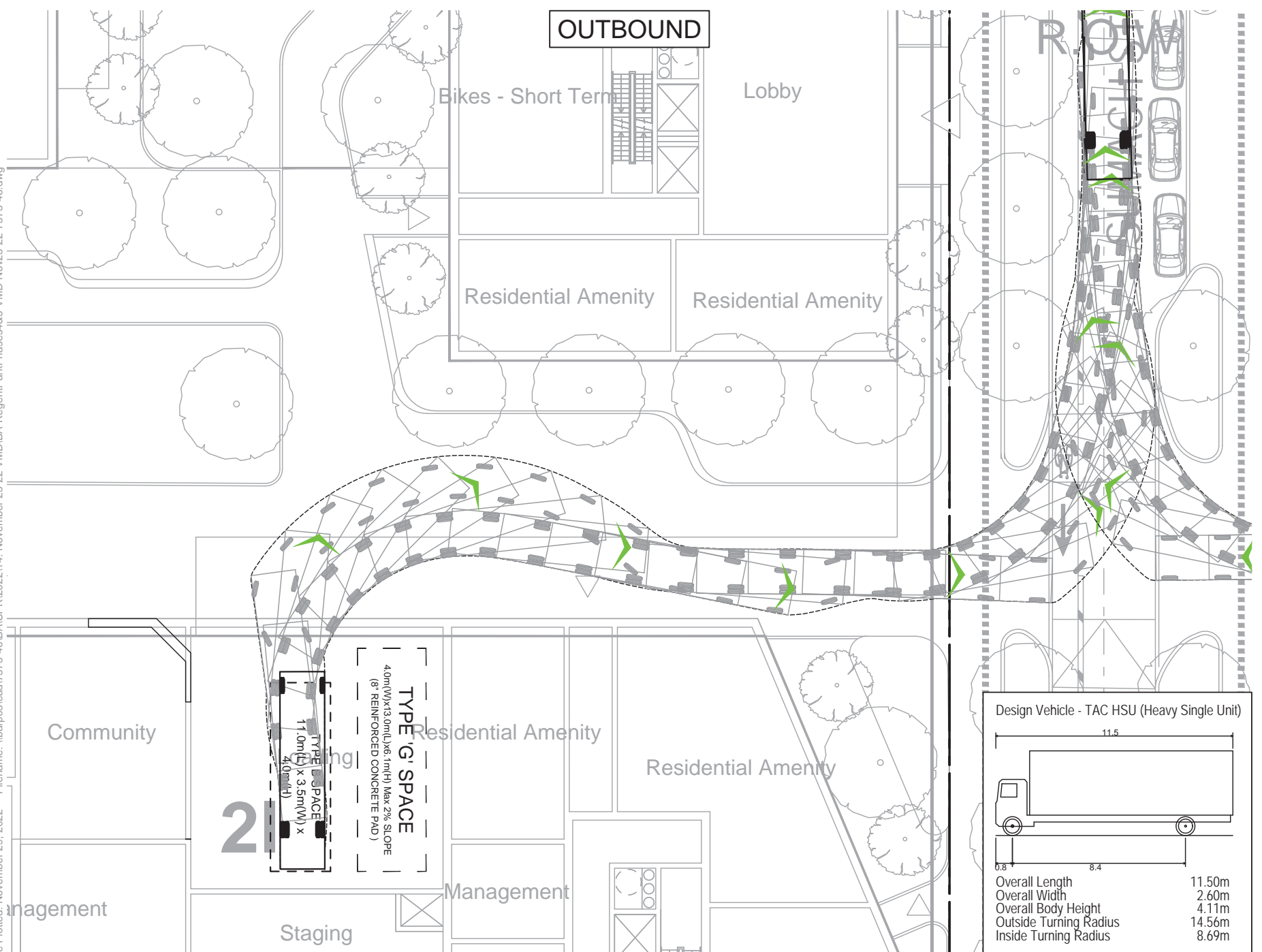
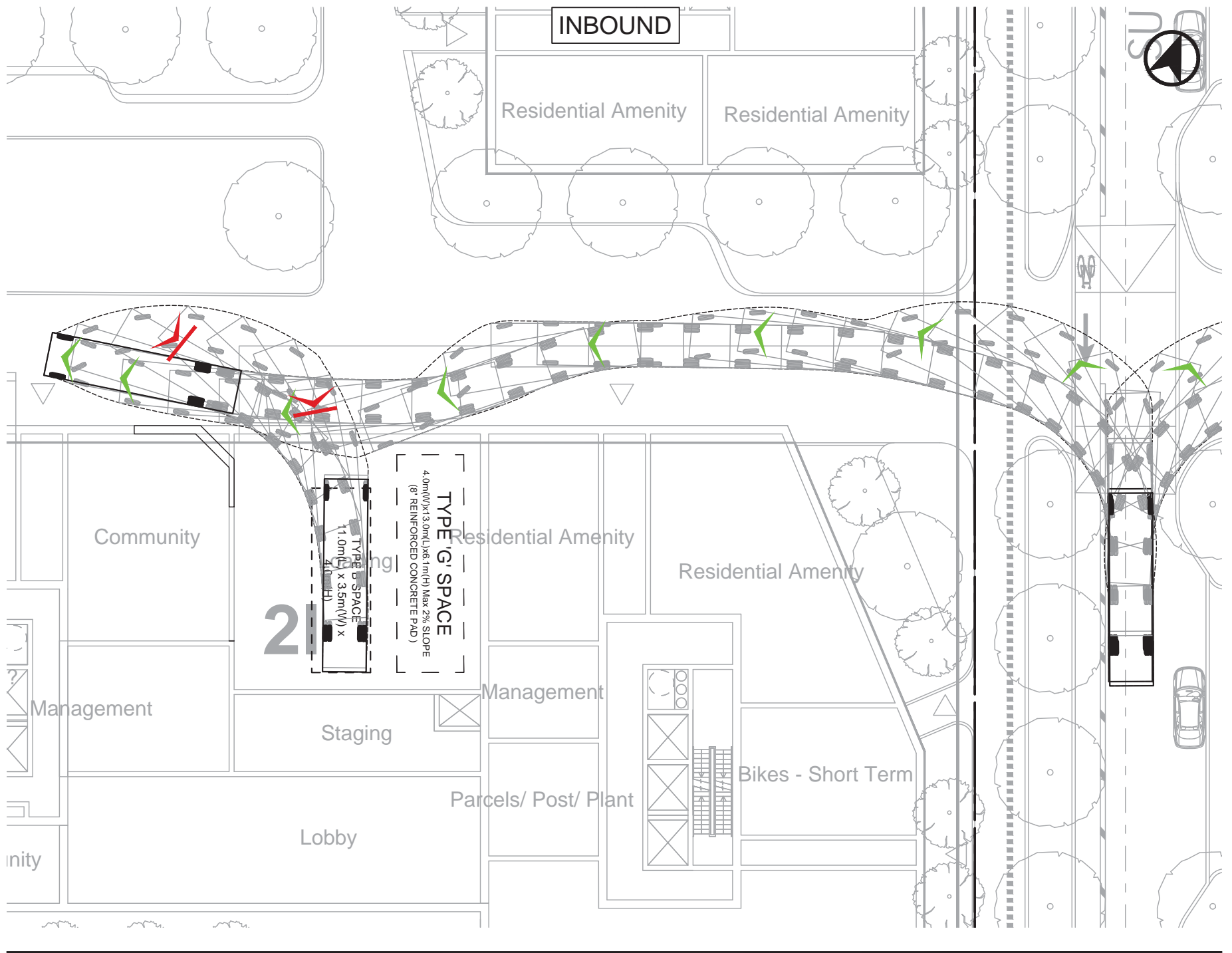


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Buildings 2H & 2I  
 TAC Heavy Single Unit (HSU) Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

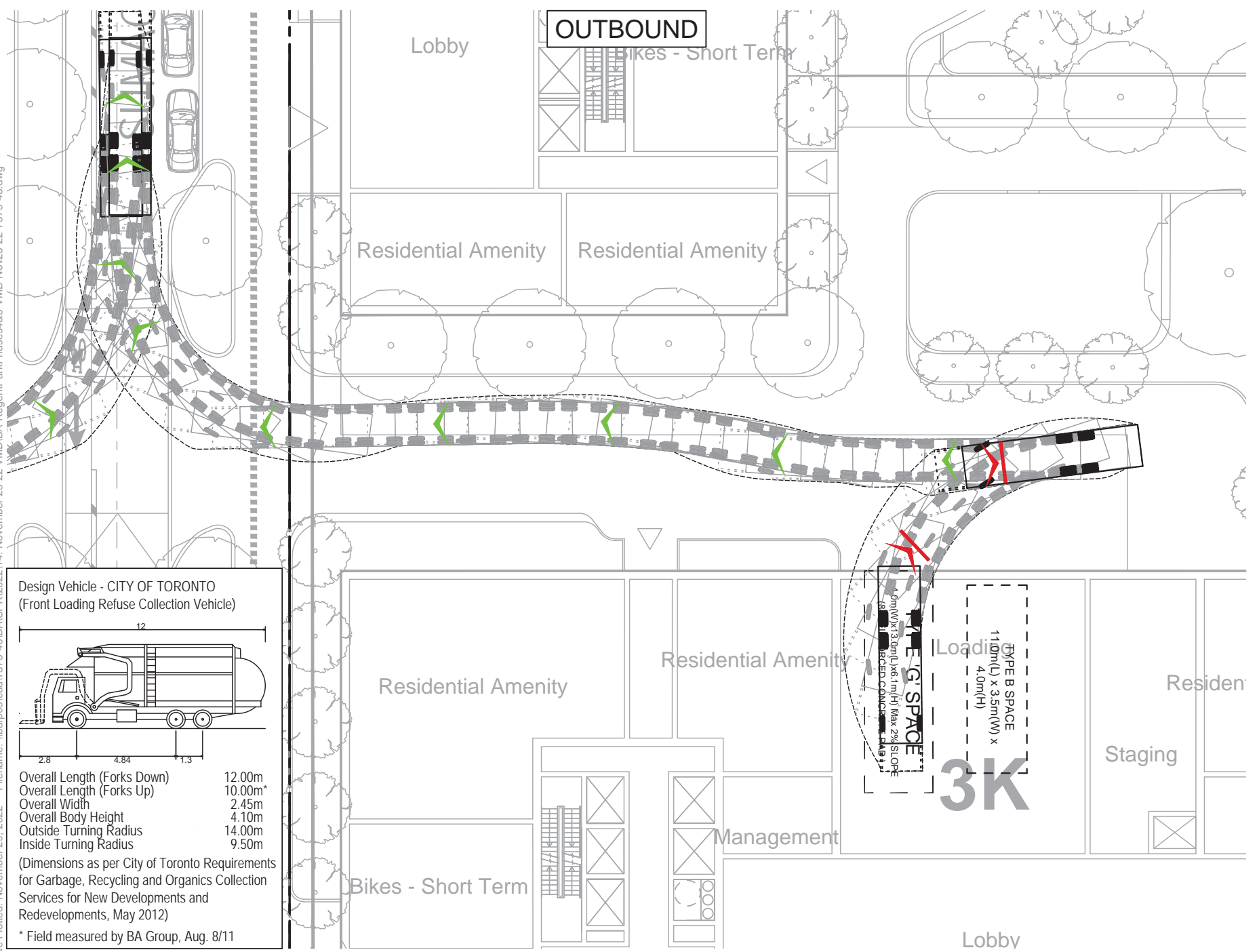
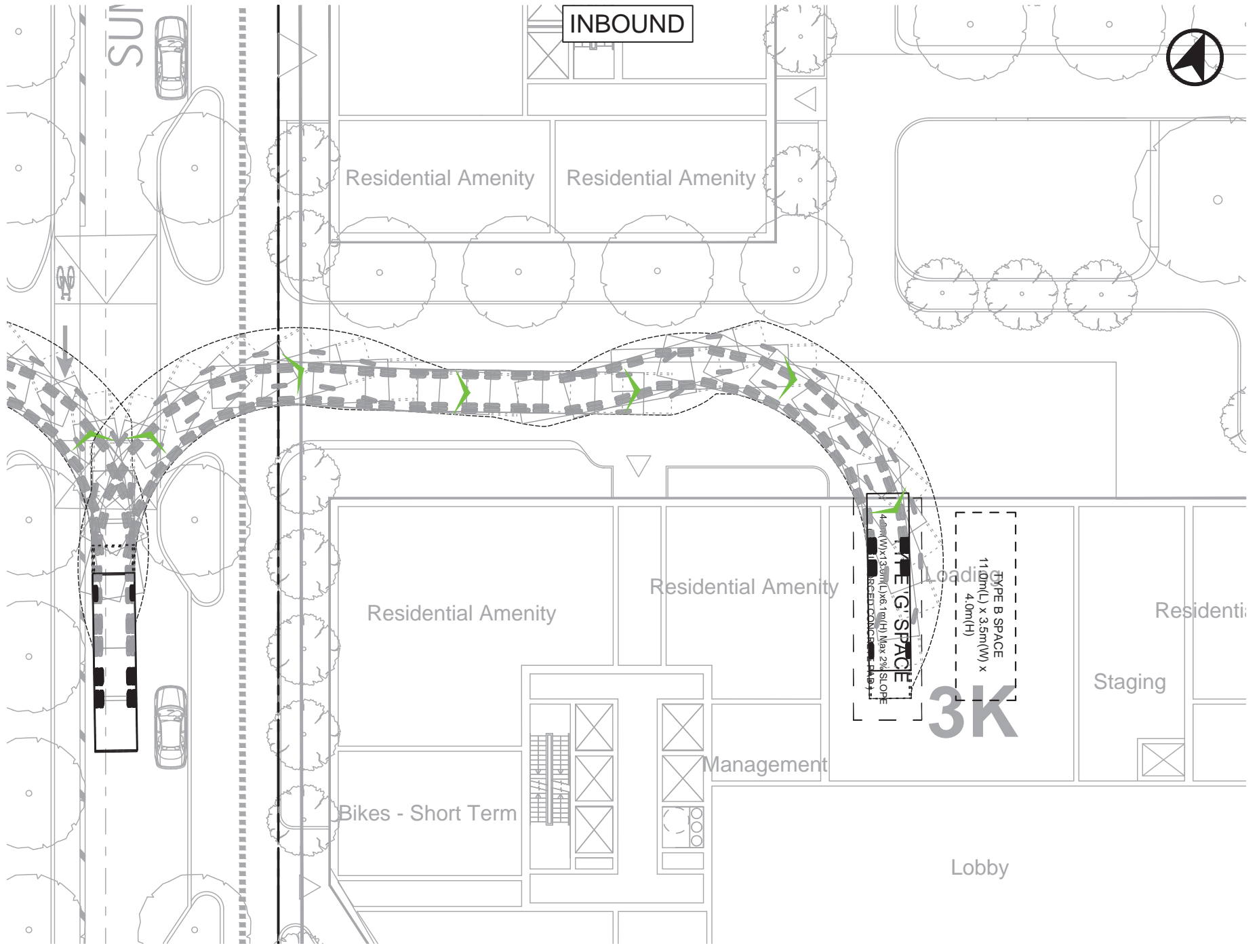
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Drawing No. **VMD-04B**



Date Plotted: November 29, 2022 File name: \\ba\p03\cad\7575-46\BA-RegentPark\Phases4&5-VMD-Nov25-22-7575-46.dwg

	<b>REGENT PARK PHASES 4 &amp; 5</b> VEHICULAR MANOEUVRING DIAGRAM Buildings 2H & 2I TAC Heavy Single Unit (HSU) Truck	Project: Regent Park Phases 4 & 5 Project No. 7575-46 Date: November 29, 2022 Revised: --	Scale: 1:300 
		Drawing No. <b>VMD-04C</b>	



Date Plotted: November 29, 2022 File name: \\bafp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

**Design Vehicle - CITY OF TORONTO**  
(Front Loading Refuse Collection Vehicle)

Overall Length (Forks Down)	12.00m
Overall Length (Forks Up)	10.00m*
Overall Width	2.45m
Overall Body Height	4.10m
Outside Turning Radius	14.00m
Inside Turning Radius	9.50m

(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments, May 2012)  
\* Field measured by BA Group, Aug. 8/11

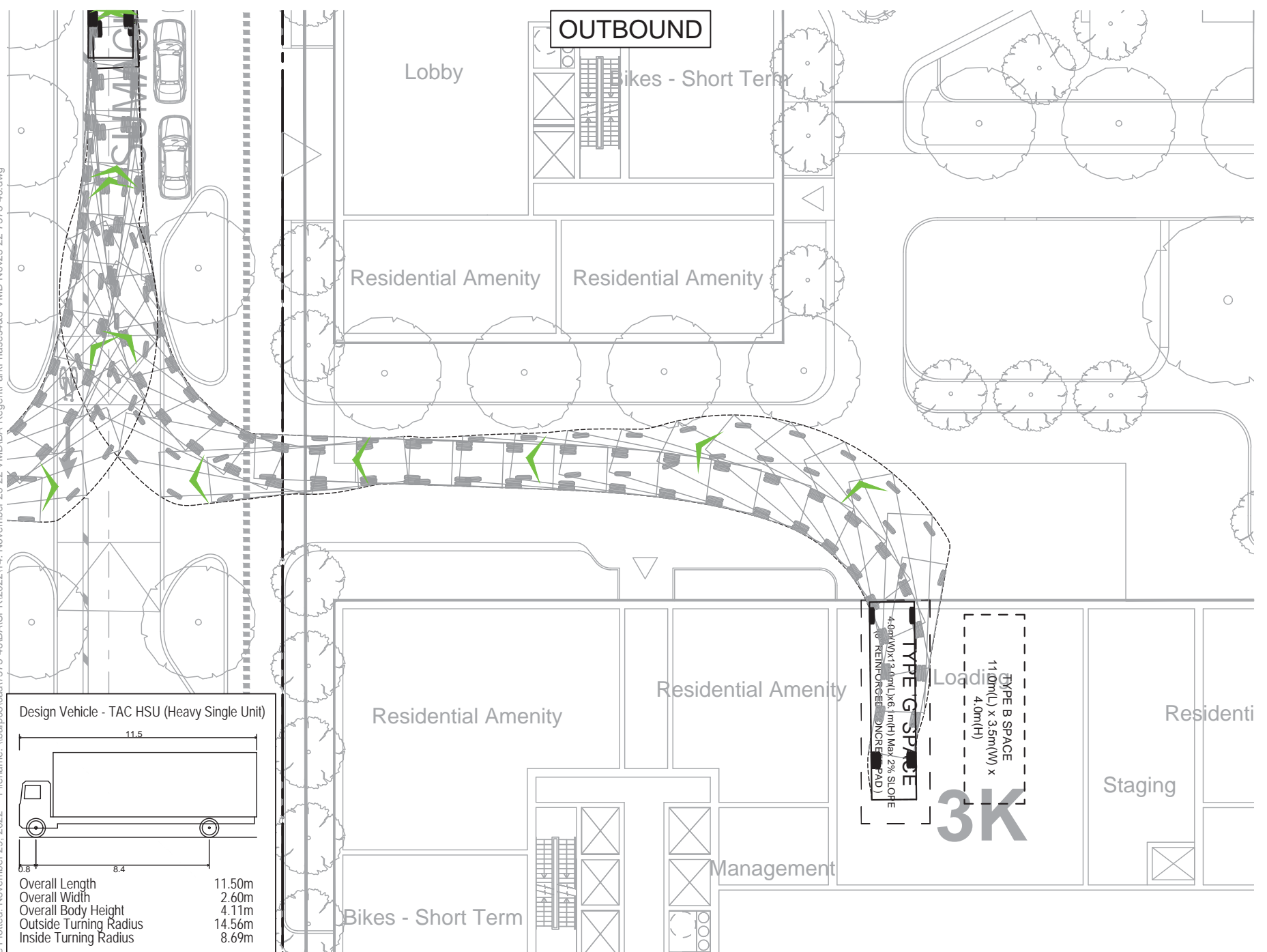
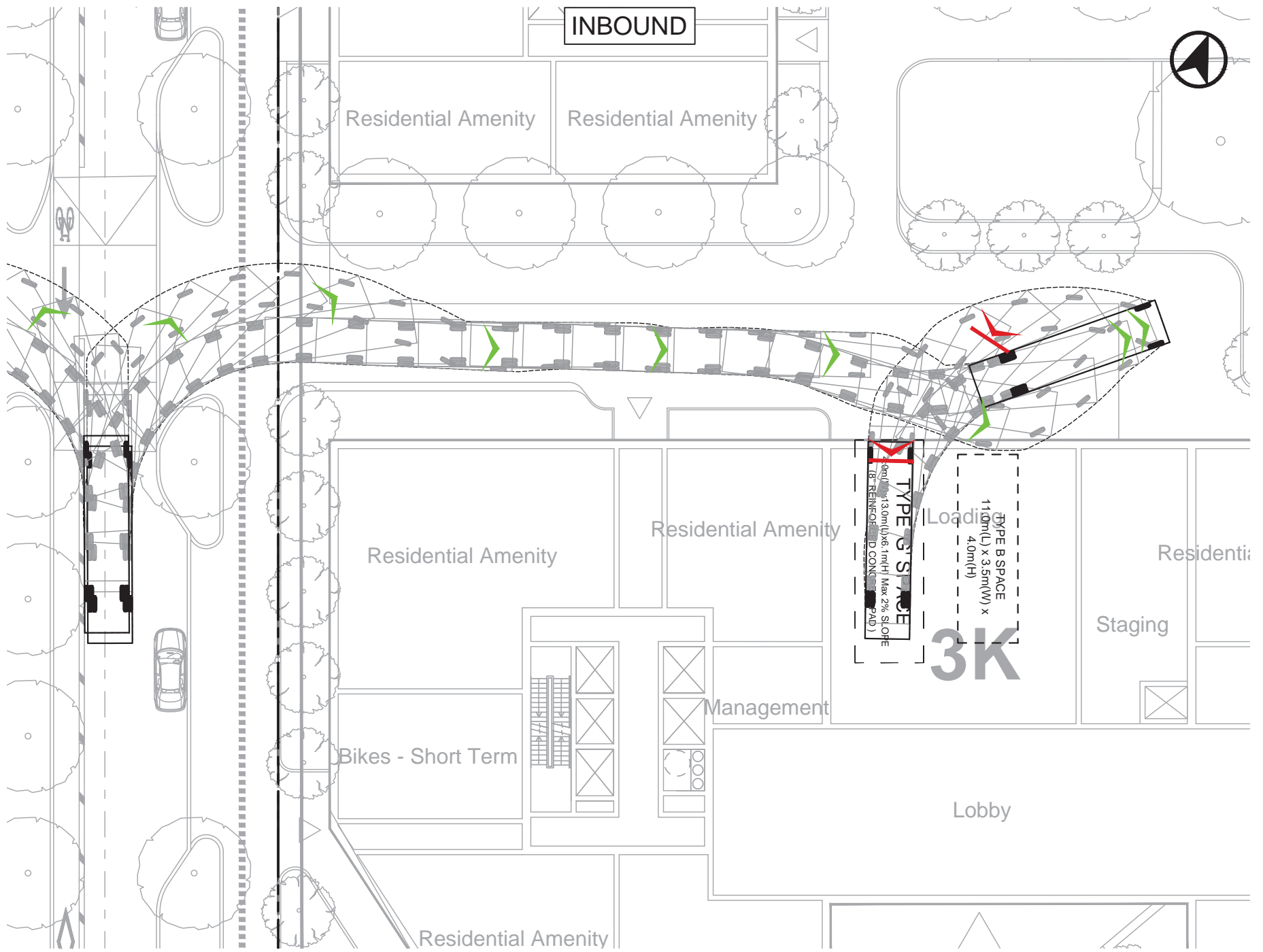


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
Buildings 3J & 3K  
City of Toronto Front Loading Garbage Truck

Project: Regent Park Phases 4 & 5  
Project No. 7575-46  
Date: November 29, 2022  
Revised: --

Scale: 1:300

Drawing No. **VMD-05A**



Date Plotted: November 29, 2022 File name: \\bafp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

Design Vehicle - TAC HSU (Heavy Single Unit)	
Overall Length	11.50m
Overall Width	2.60m
Overall Body Height	4.11m
Outside Turning Radius	14.56m
Inside Turning Radius	8.69m

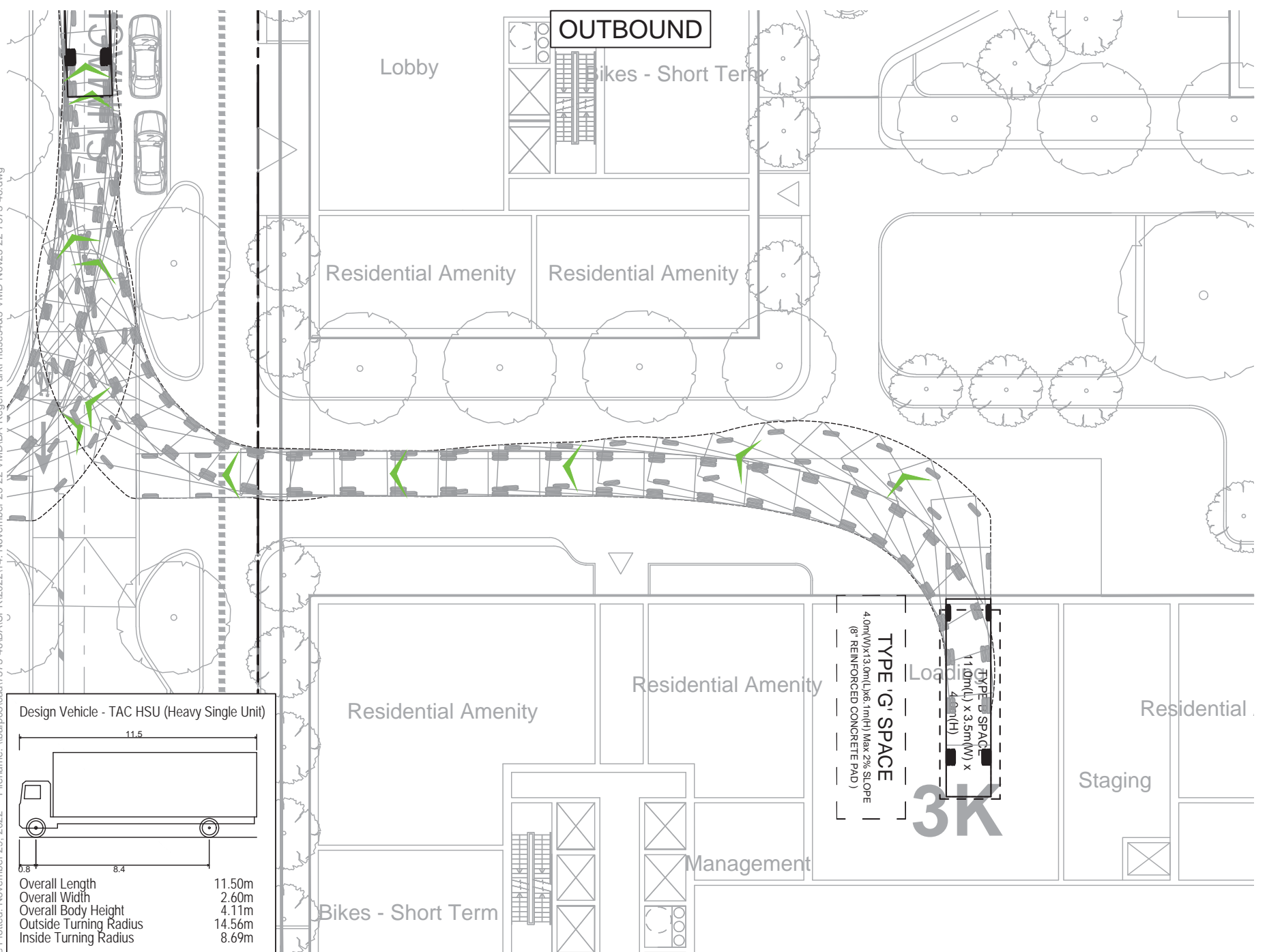
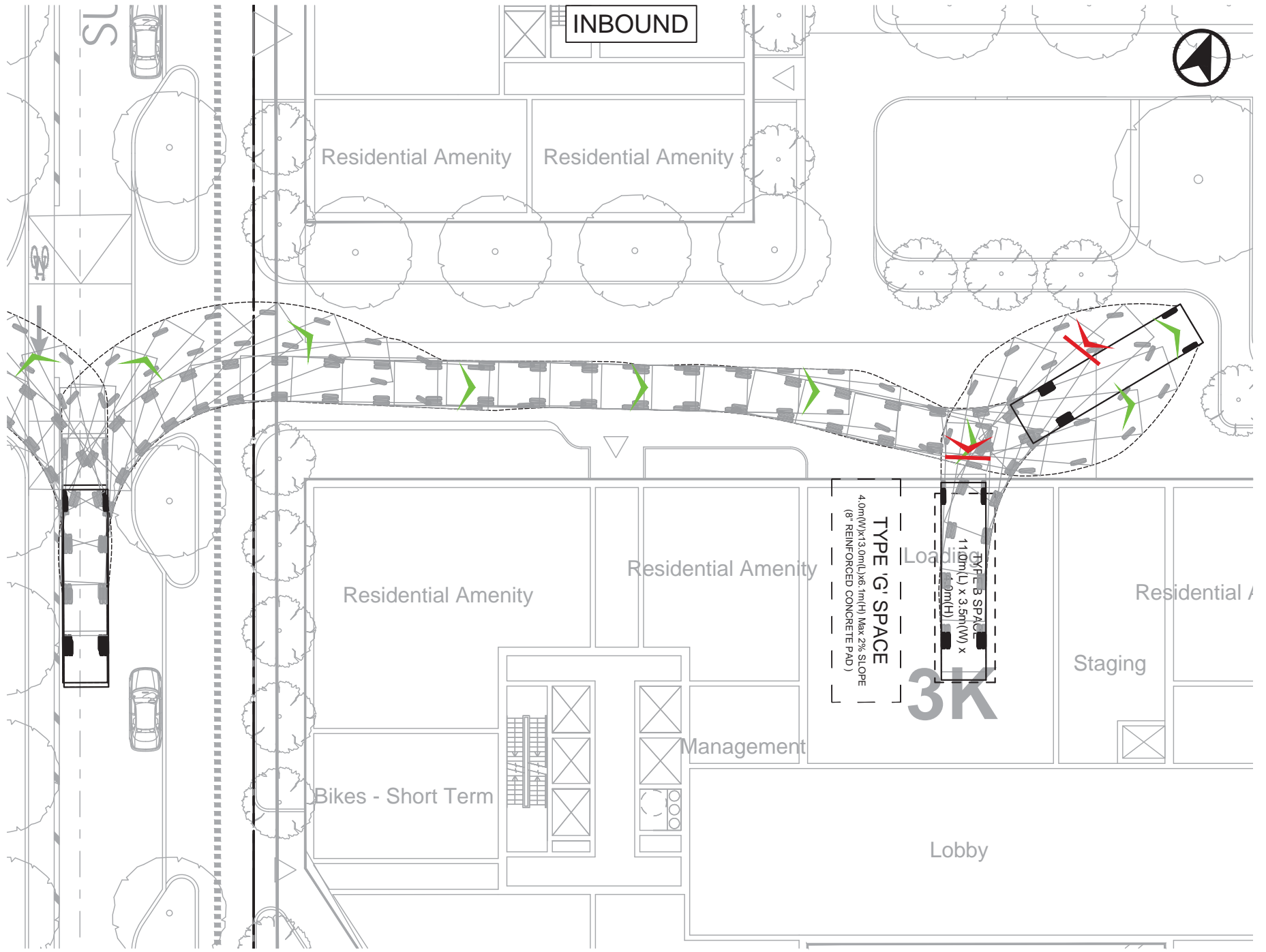


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Buildings 3J & 3K  
 TAC Heavy Single Unit (HSU) Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

Scale: 1:300

Drawing No. **VMD-05B**



Date Plotted: November 29, 2022 File name: \\bafp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

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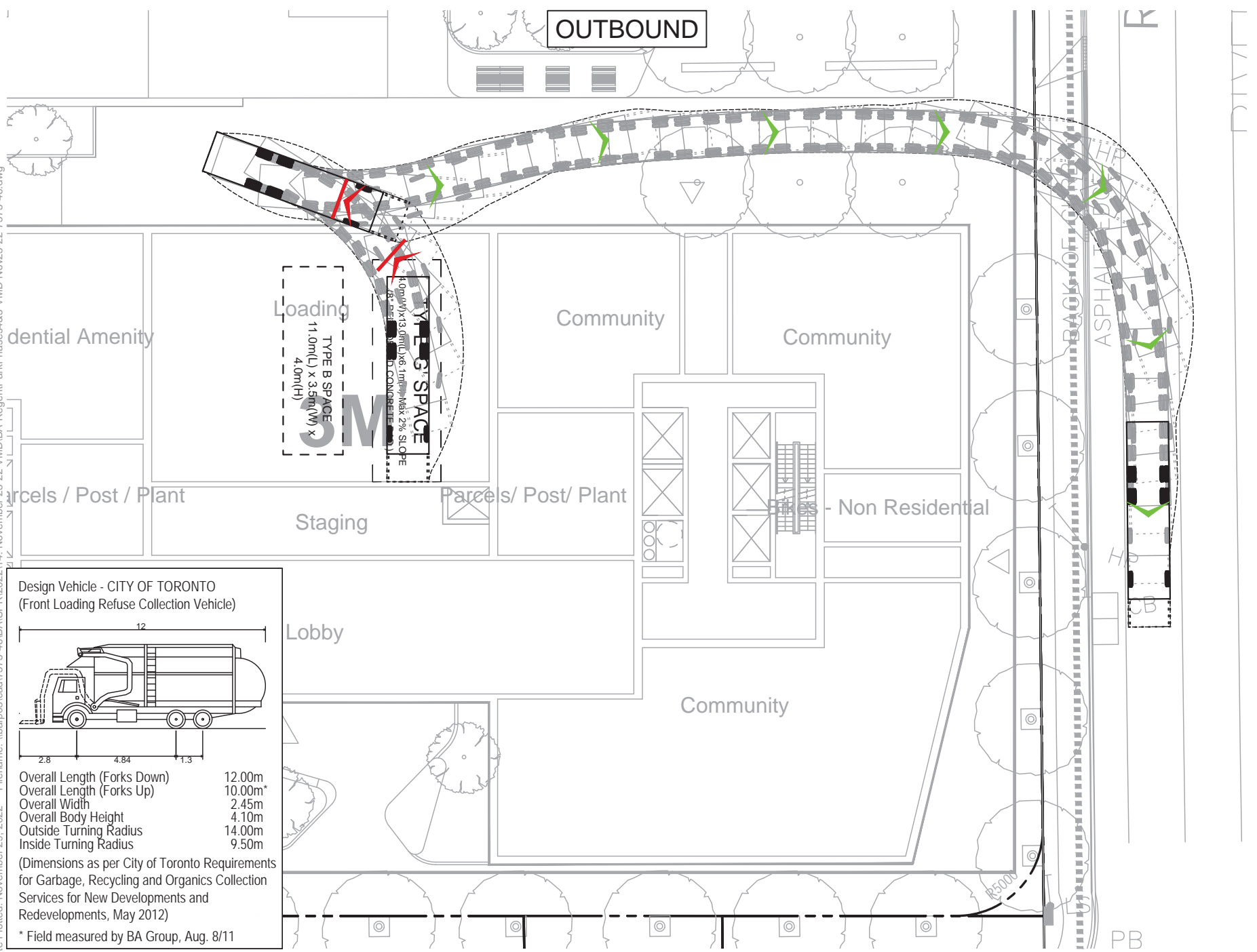
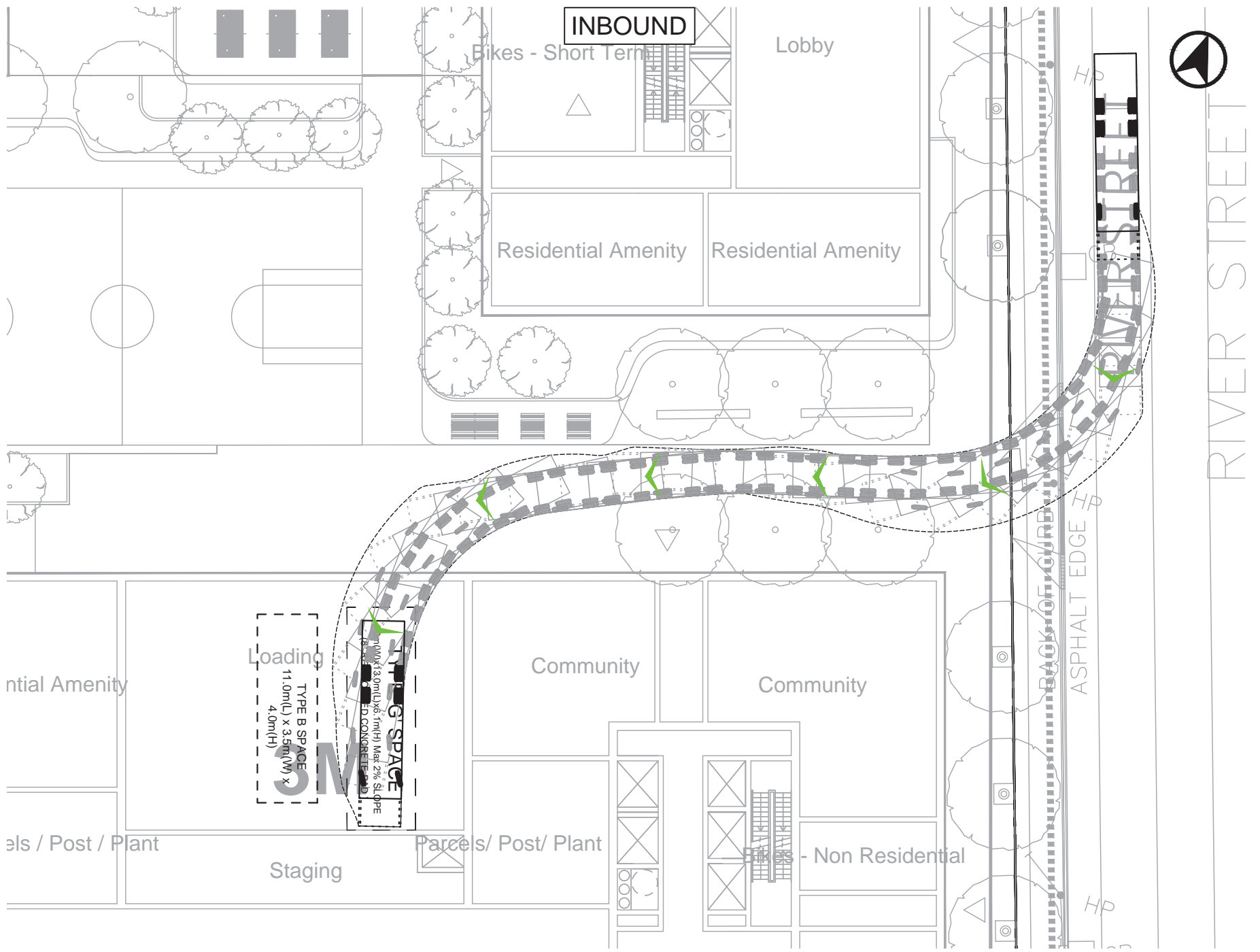


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Buildings 3J & 3K  
 TAC Heavy Single Unit (HSU) Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

Scale: 1:300

Drawing No. **VMD-05C**



Date Plotted: November 29, 2022 File name: \\baftp03\cad\7575-46\BA\SPR\2022\14 - November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

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(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments, May 2012)  
\* Field measured by BA Group, Aug. 8/11



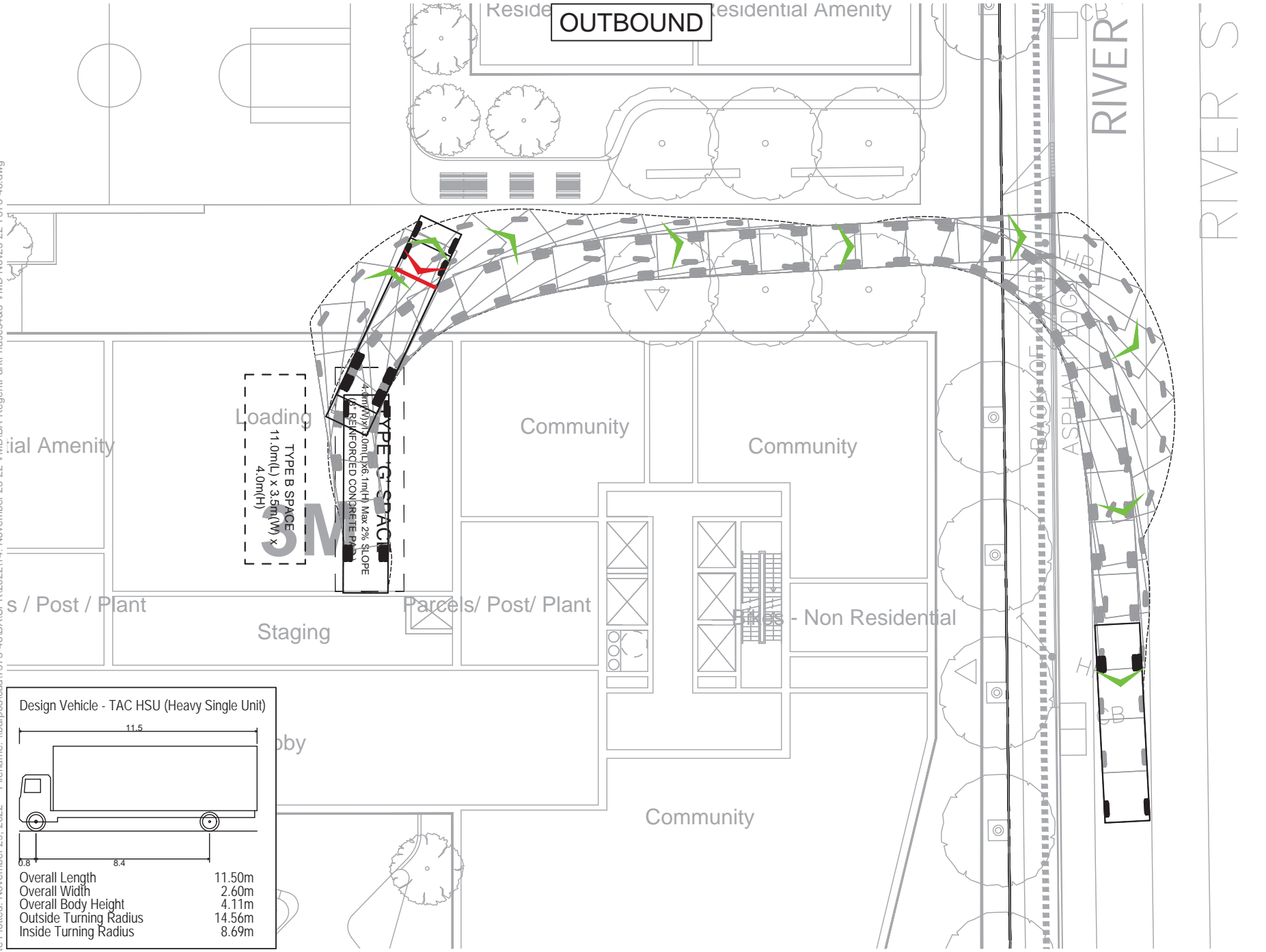
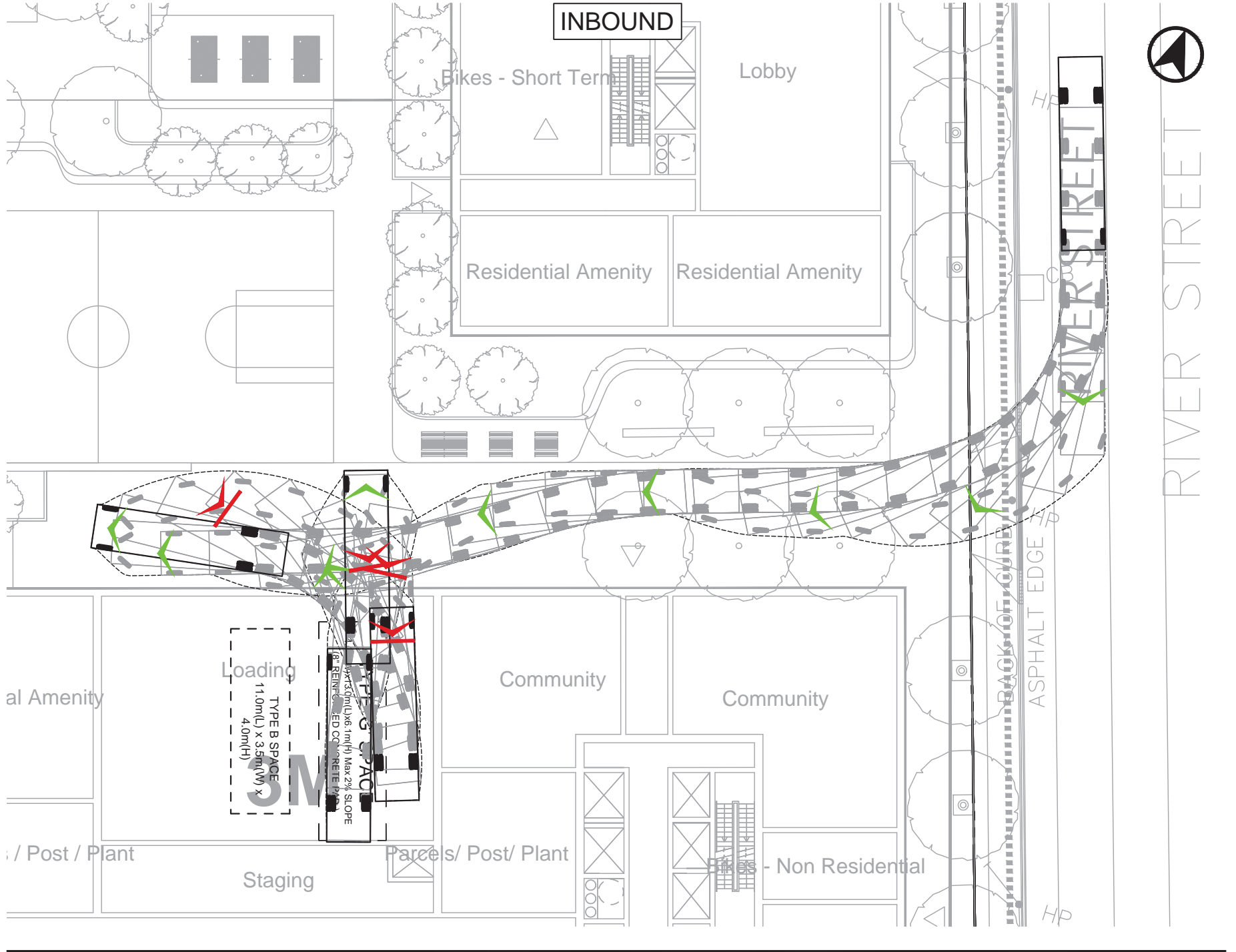
**REGENT PARK PHASES 4 & 5**  
VEHICULAR MANOEUVRING DIAGRAM  
Buildings 3L & 3M  
City of Toronto Front Loading Garbage Truck

Project: Regent Park Phases 4 & 5  
Project No. 7575-46  
Date: November 29, 2022  
Revised: --

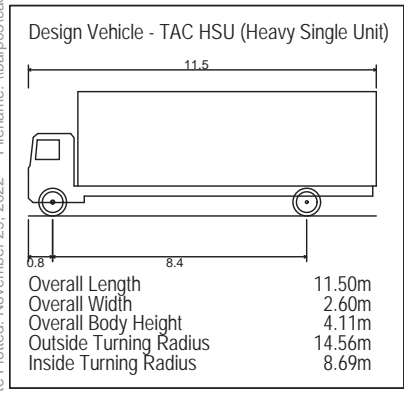
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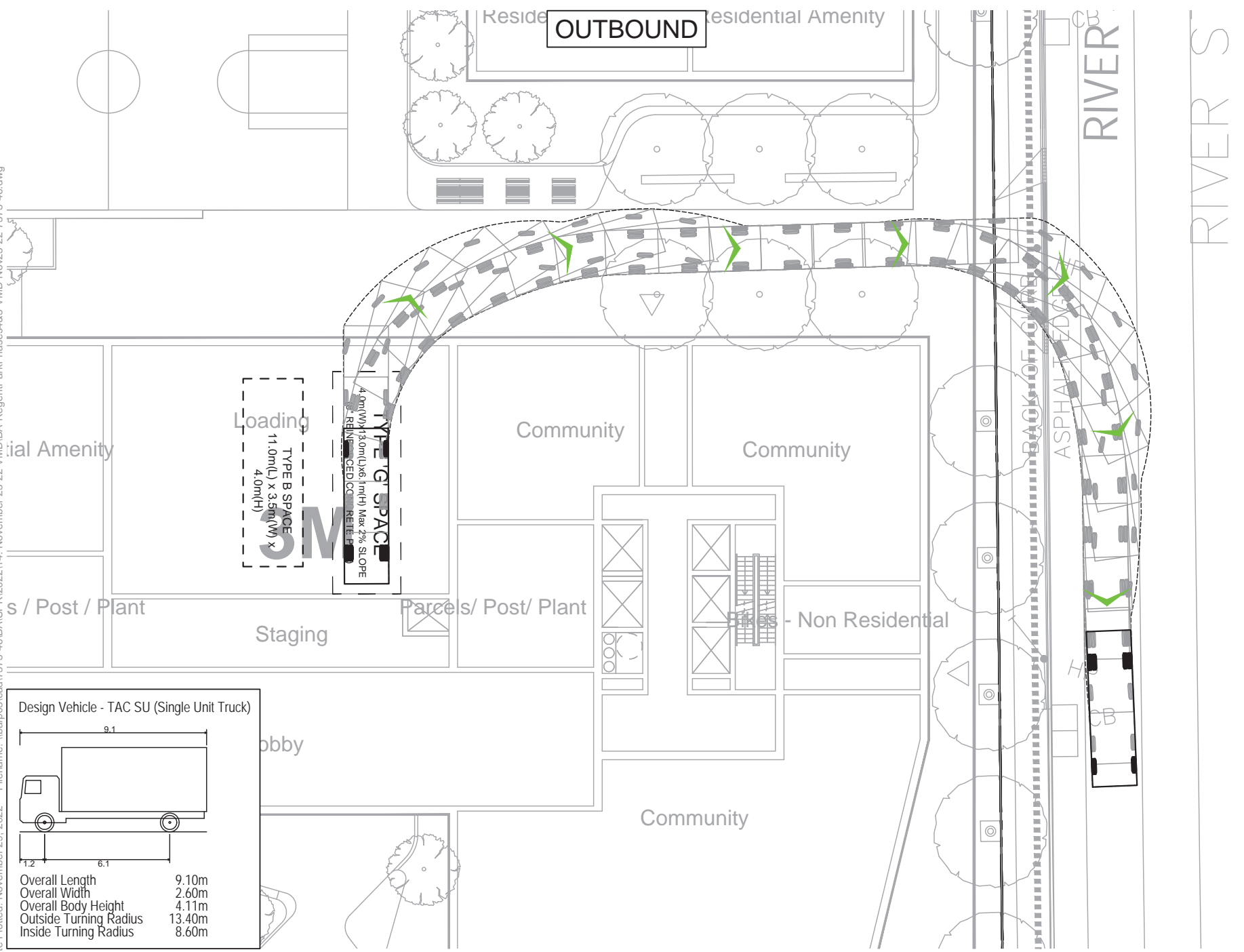
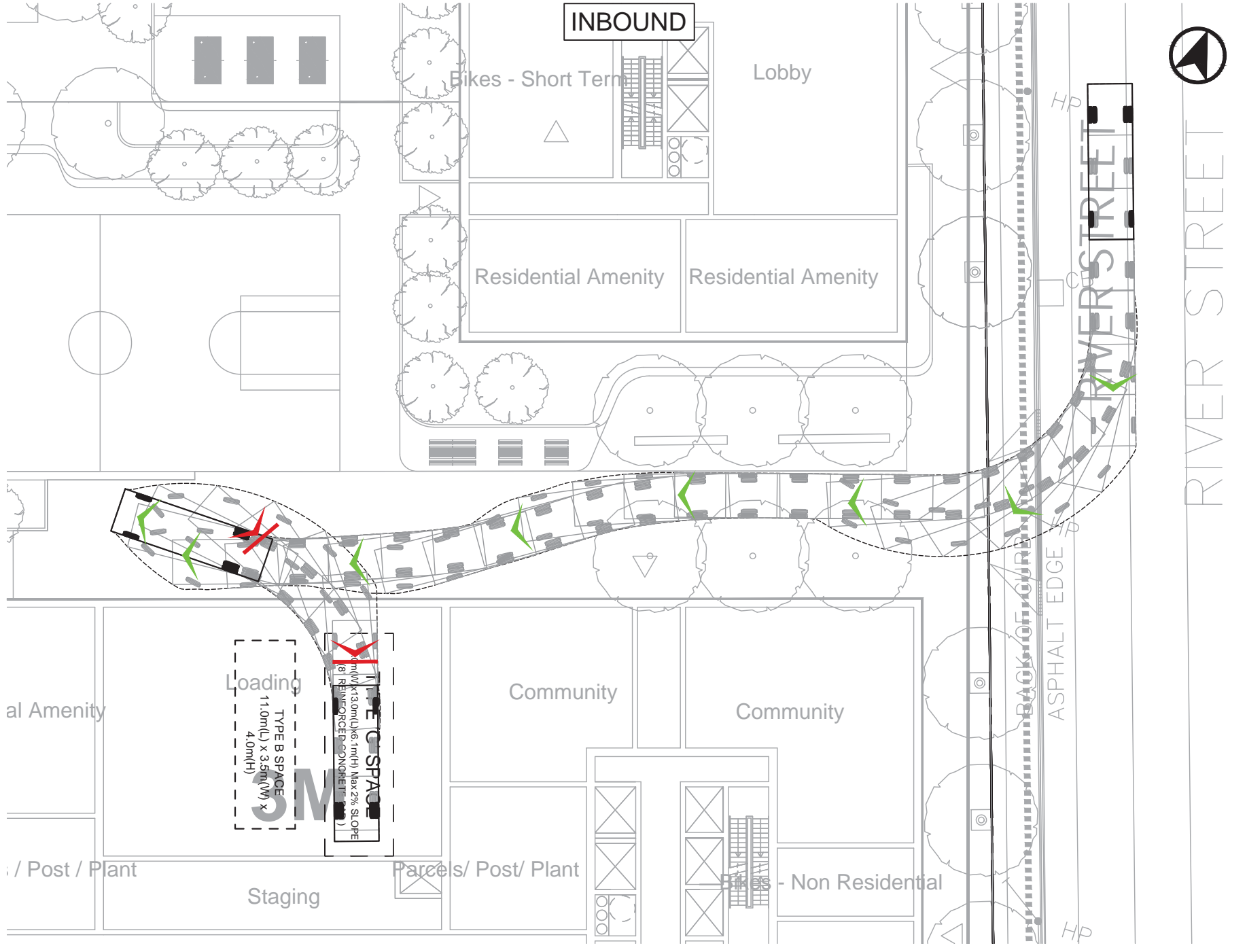


**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Buildings 3L & 3M  
 TAC Heavy Single Unit (HSU) Truck

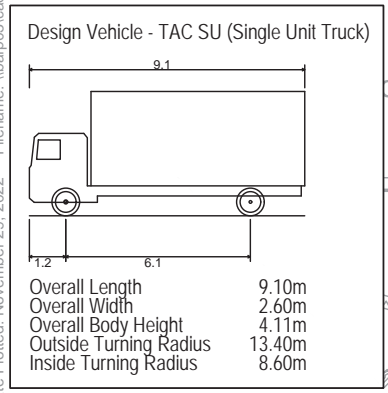
Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

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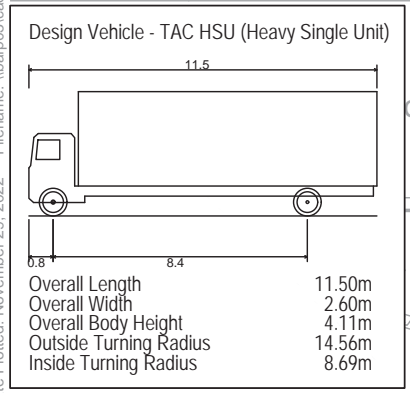
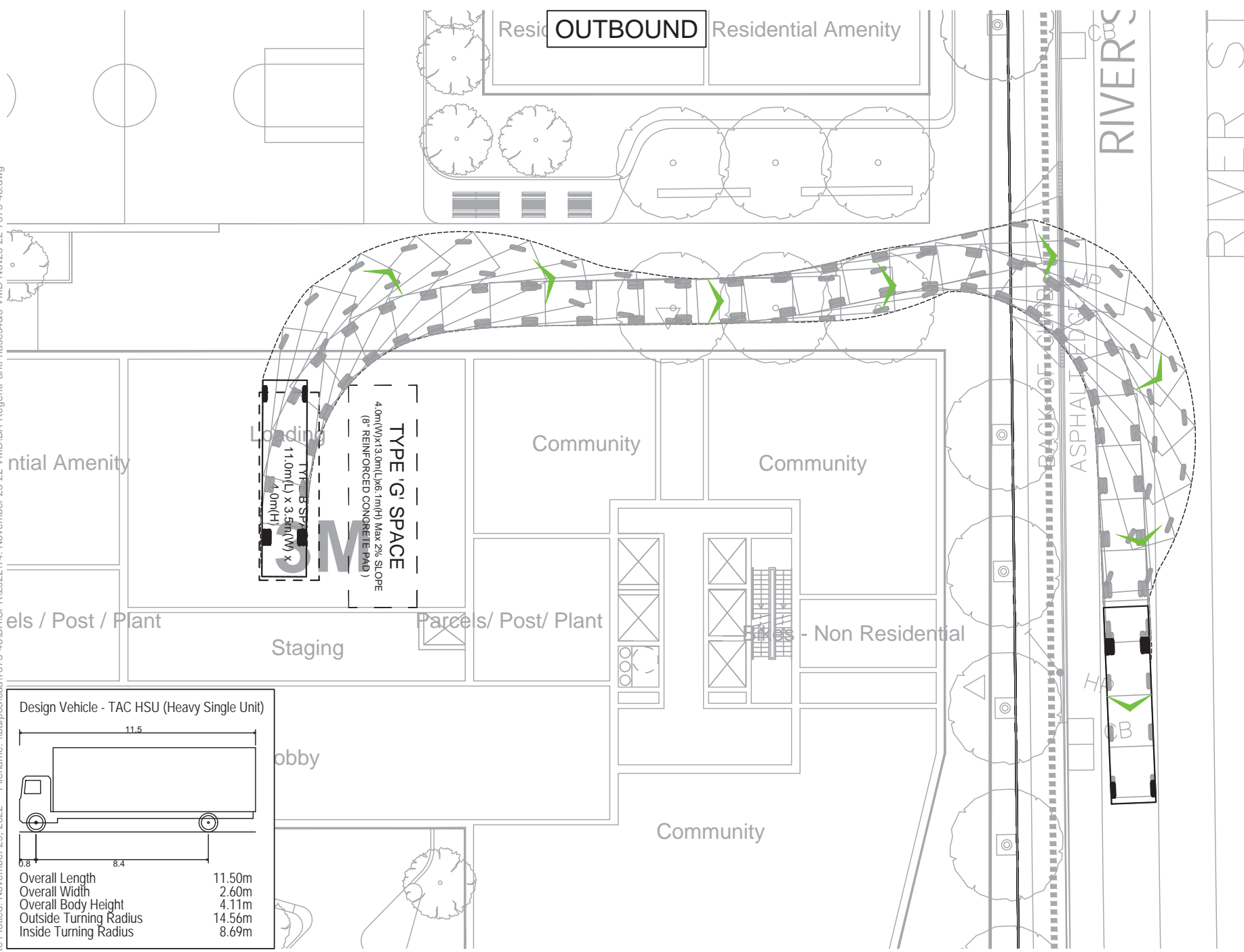
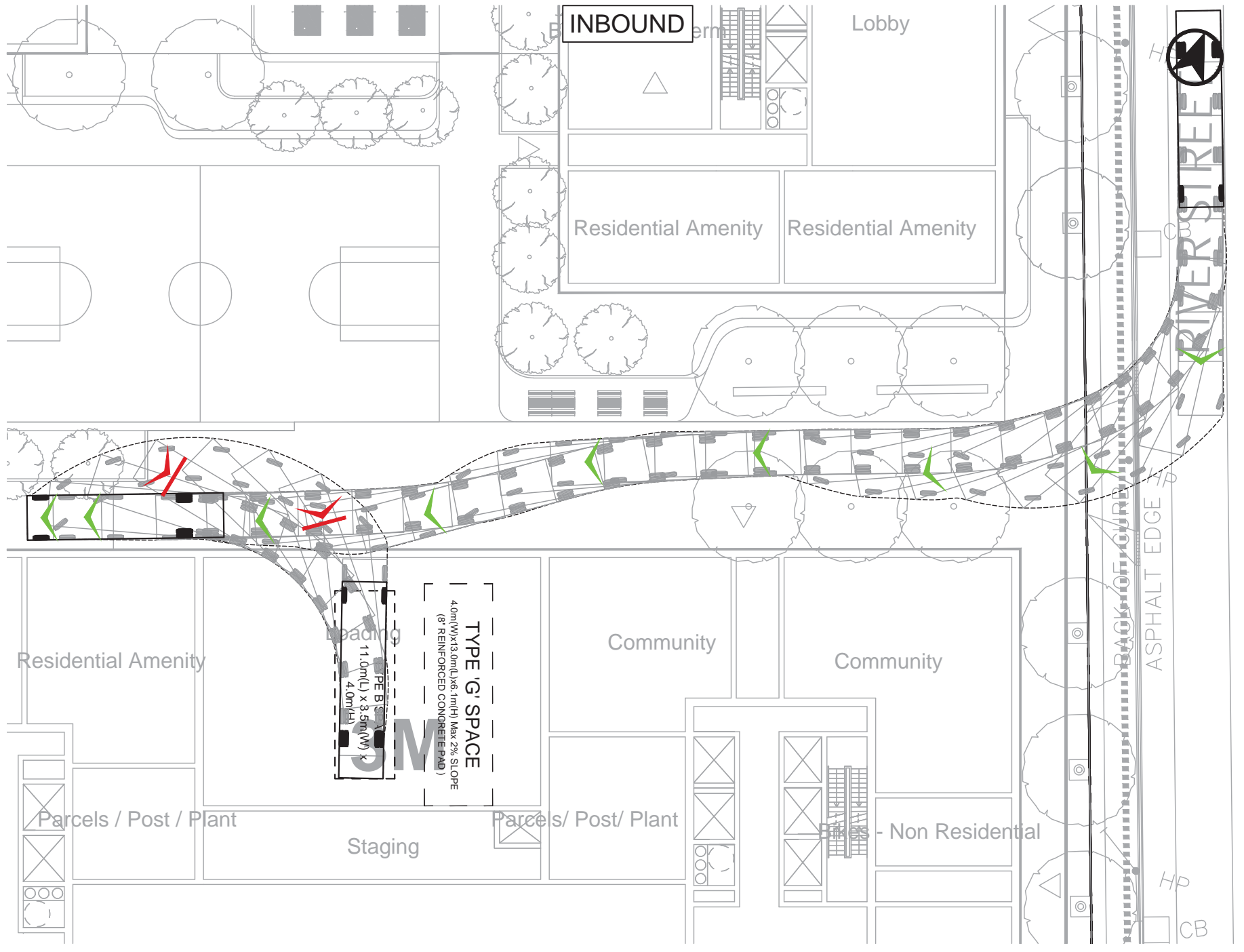


**REGENT PARK PHASES 4 & 5**  
 VEHICULAR MANOEUVRING DIAGRAM  
 Buildings 3L & 3M  
 TAC Single Unit (SU) Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

Scale: 1:300

Drawing No. **VMD-06B**



Date Plotted: November 29, 2022 File name: \\bafp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

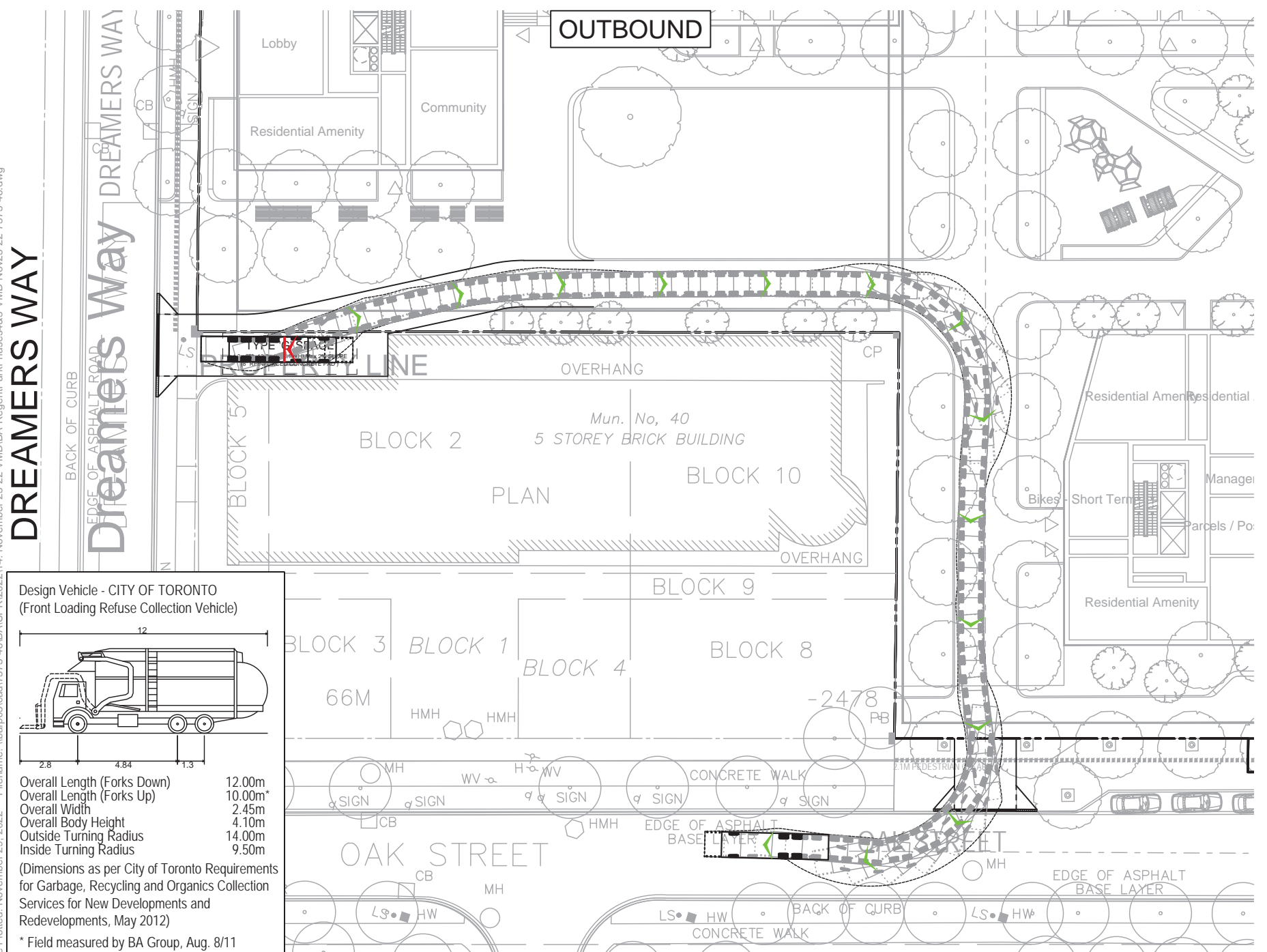
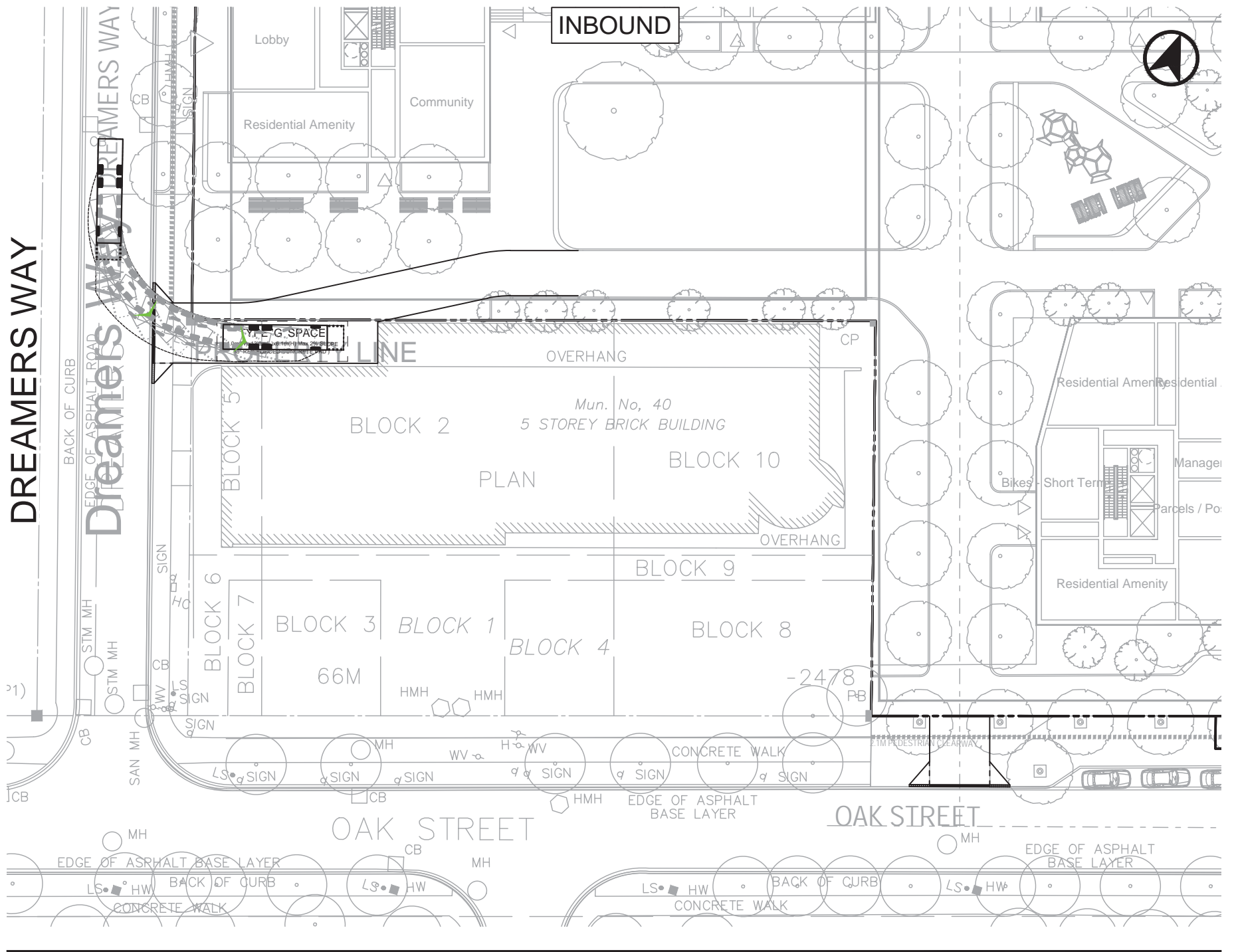


**REGENT PARK PHASES 4 & 5**  
 VEHICULAR MANOEUVRING DIAGRAM  
 Buildings 3L & 3M  
 TAC Heavy Single Unit (HSU) Truck

Project: Regent Park Phases 4 & 5  
 Project No. 7575-46  
 Date: November 29, 2022  
 Revised: --

Scale 1:300

Drawing No. **VMD-06C**



Date Plotted: November 29, 2022 File name: \\bafp03\cad\7575-46\BA\SPR\2022\14\_November 25-22 VMD\BA-RegentParkPhases4&5-VMD-Nov25-22-7575-46.dwg

**Design Vehicle - CITY OF TORONTO**  
(Front Loading Refuse Collection Vehicle)

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Overall Length (Forks Up)	10.00m*
Overall Width	2.45m
Overall Body Height	4.10m
Outside Turning Radius	14.00m
Inside Turning Radius	9.50m

(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments, May 2012)  
\* Field measured by BA Group, Aug. 8/11



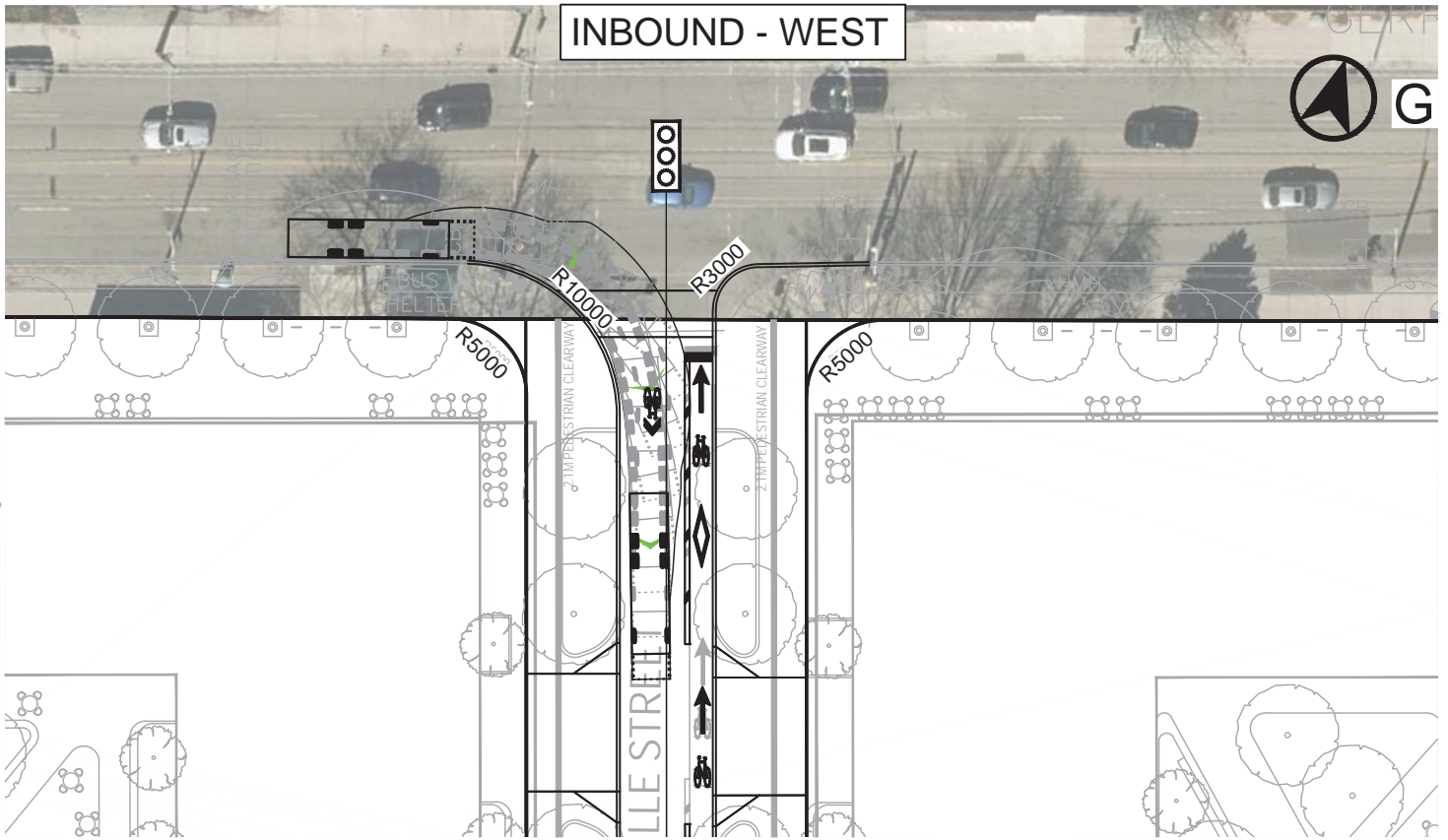
**REGENT PARK PHASES 4 & 5**  
VEHICULAR MANOEUVRING DIAGRAM  
Fred Victor Housing  
City of Toronto Front Loading Garbage Truck

Project: Regent Park Phases 4 & 5  
Project No. 7575-46  
Date: November 29, 2022  
Revised: --

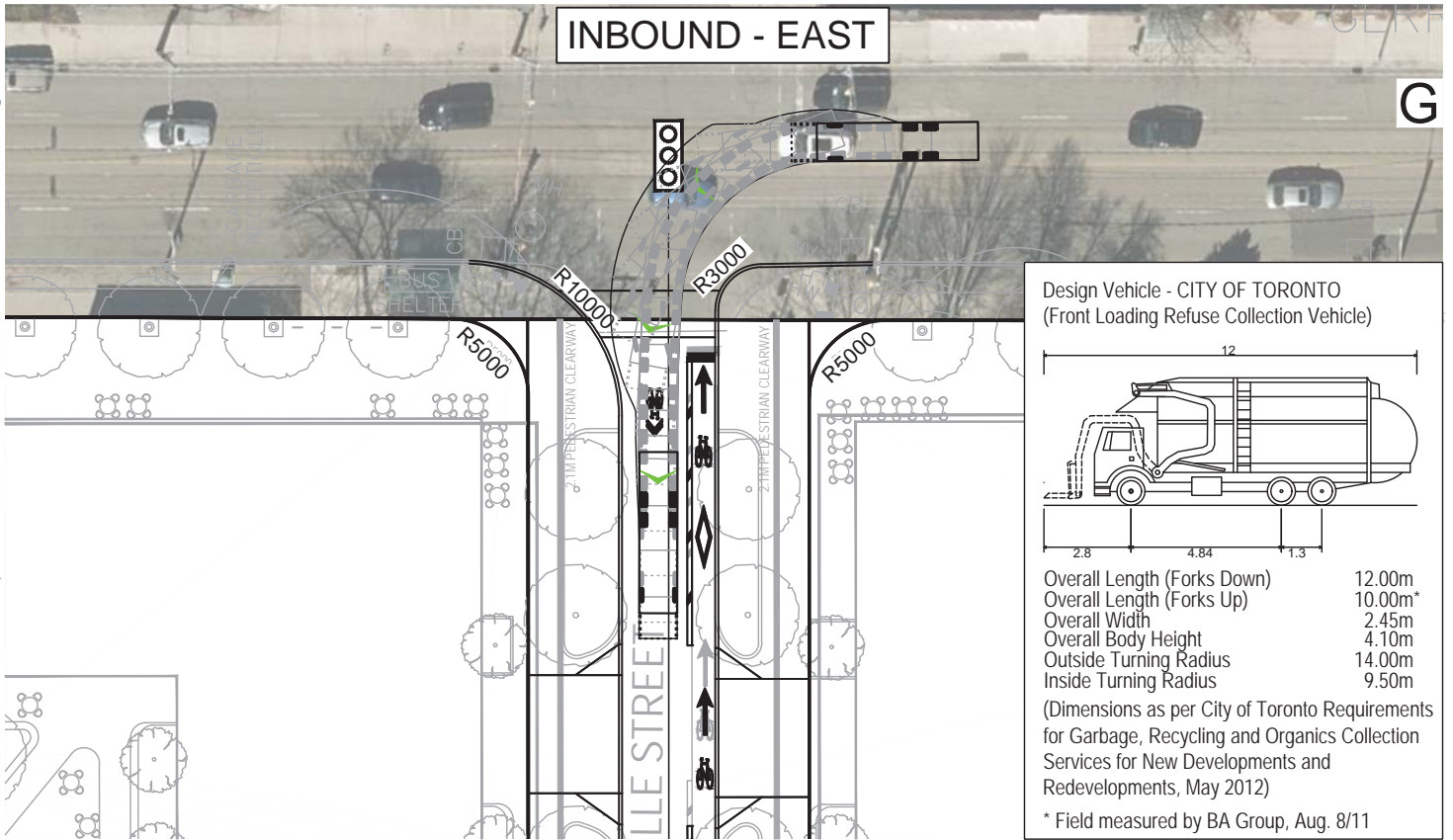
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Drawing No. **VMD-07**

# INBOUND - WEST



# INBOUND - EAST

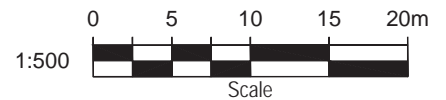


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(Front Loading Refuse Collection Vehicle)

Overall Length (Forks Down)	12.00m
Overall Length (Forks Up)	10.00m*
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Overall Body Height	4.10m
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(Dimensions as per City of Toronto Requirements for Garbage, Recycling and Organics Collection Services for New Developments and Redevelopments, May 2012)

\* Field measured by BA Group, Aug. 8/11



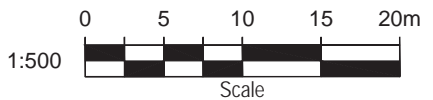
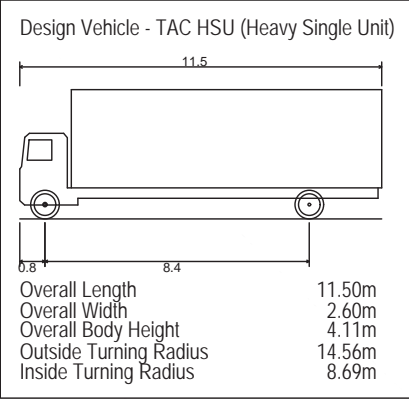
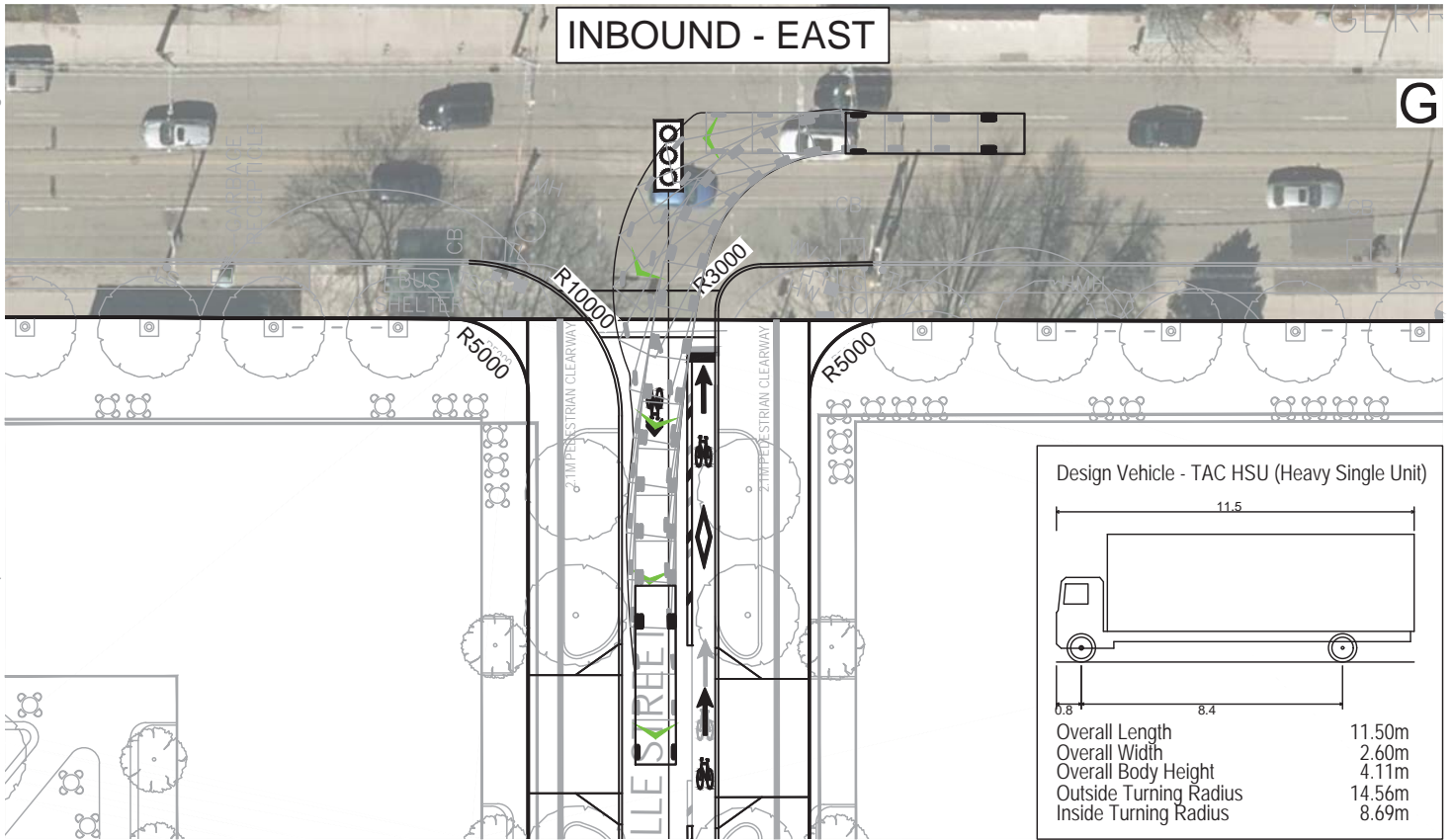
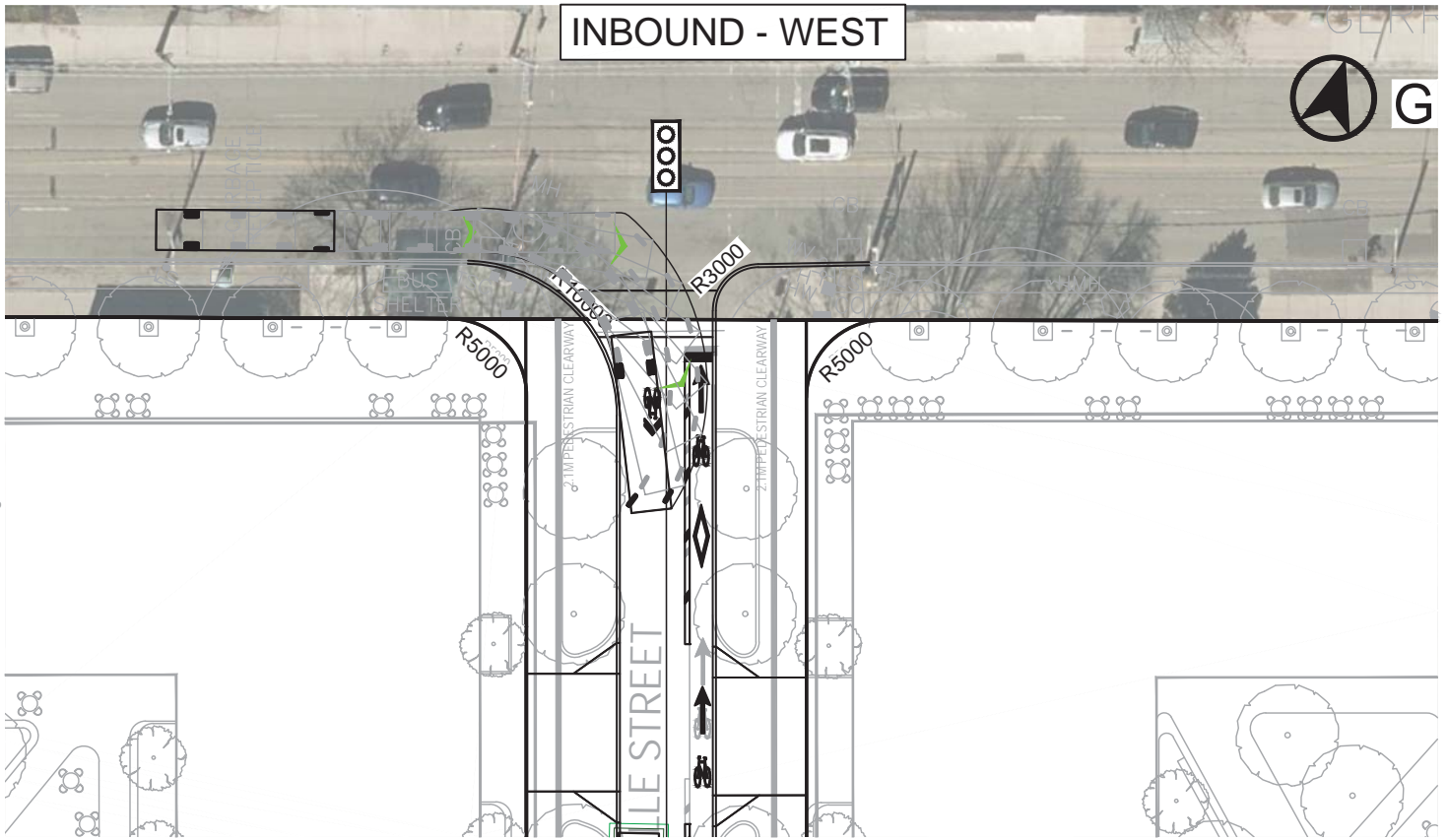
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**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Gerrard Ave to Sackville St  
 City of Toronto Front Loading Garbage Truck

Project: Regent Park Phases 4&5  
 Project No. 7575-46  
 Date: November 23, 2022  
 Revised: --  
 Drawing No. **VMD-08**

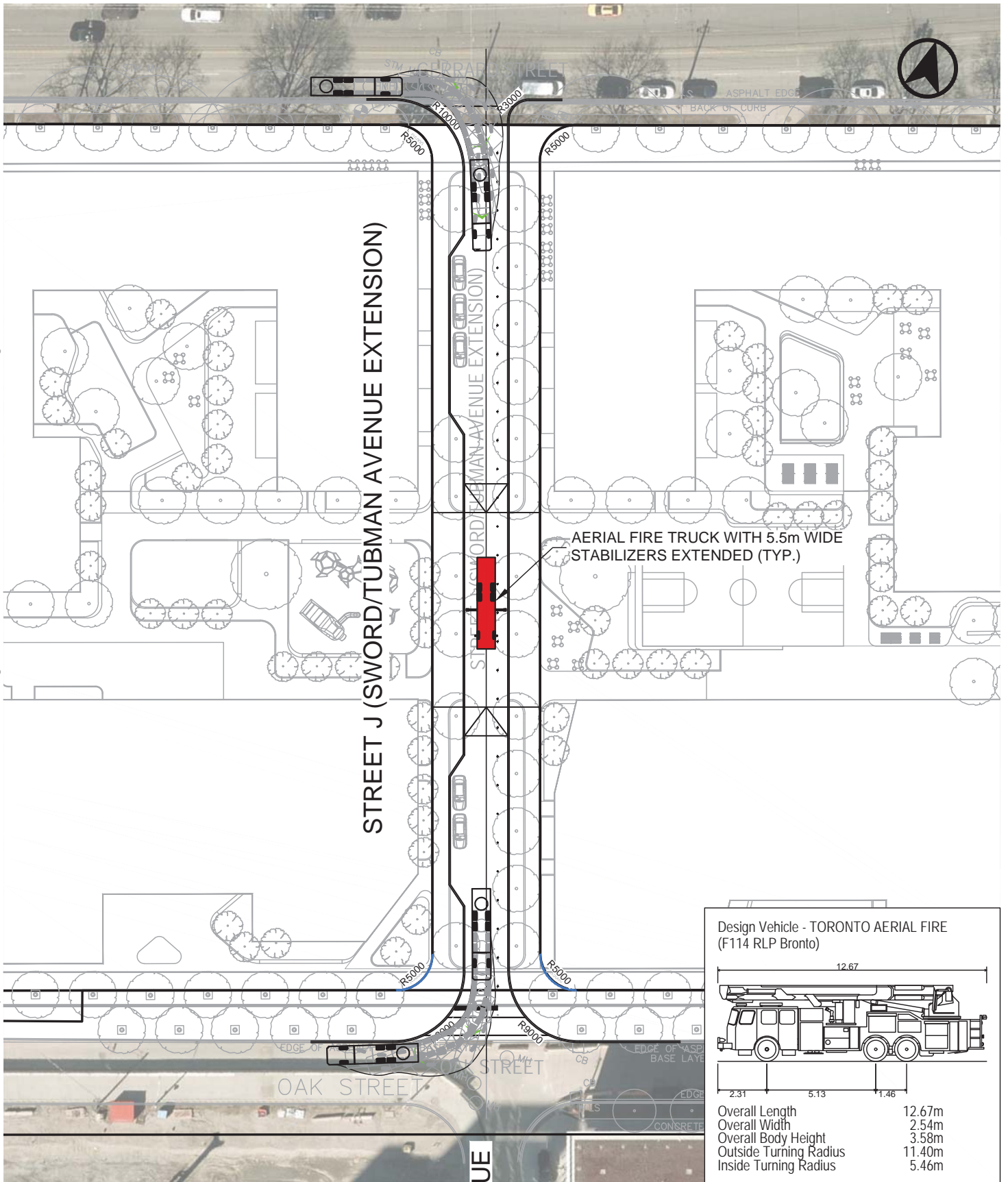
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**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Gerrard Ave to Sackville St  
 TAC Heavy Single Unit (HSU) Truck

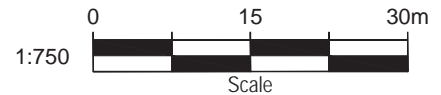
Project:	Regent Park Phases 4&5
Project No.	7575-46
Date:	November 23, 2022
Revised:	--
Drawing No.	<b>VMD-09</b>

Date Plotted: November 28, 2022. Filename: \\batp03\cad\7575-46\BA\SPR\2022\14. November 25-22 VMD\BA-Regent Park Phase4&5-FD-Nov23-22-7575-46 - Gerrard VMDs.dwg



Design Vehicle - TORONTO AERIAL FIRE (F114 RLP Bronto)

Overall Length	12.67m
Overall Width	2.54m
Overall Body Height	3.58m
Outside Turning Radius	11.40m
Inside Turning Radius	5.46m



**REGENT PARK PHASES 4 & 5**  
**VEHICULAR MANOEUVRING DIAGRAM**  
 Gerrard Ave to Street J (Sword/Tubman Ave Extension)  
 City of Toronto Aerial Fire Truck

Project:	Regent Park Phases 4&5
Project No.	7575-46
Date:	November 28, 2022
Revised:	--
Drawing No.	<b>VMD-10</b>

## Appendix F: Updated Lane Configuration and Traffic Volume Figures





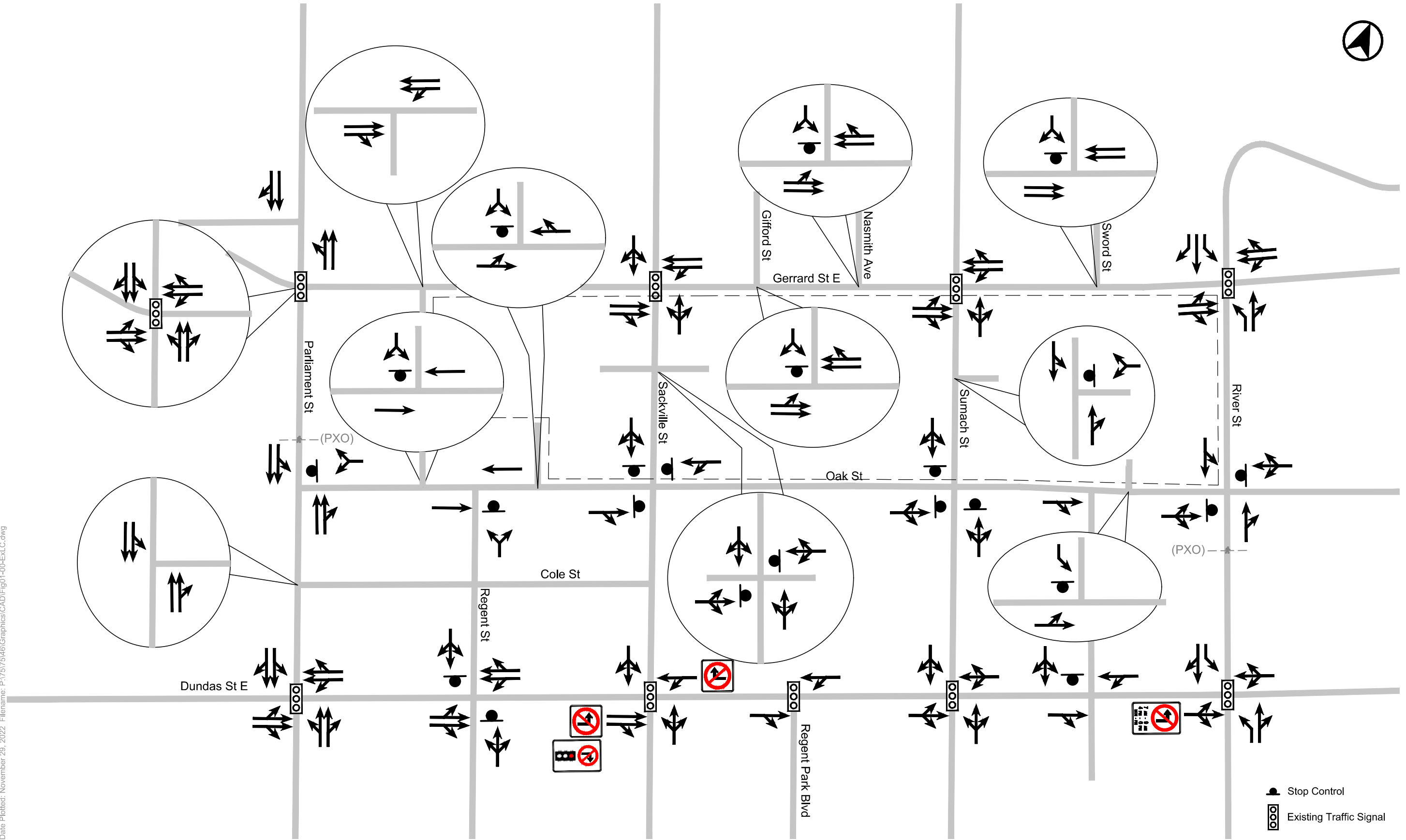


FIGURE 1 EXISTING LANE CONFIGURATION AND TRAFFIC CONTROL

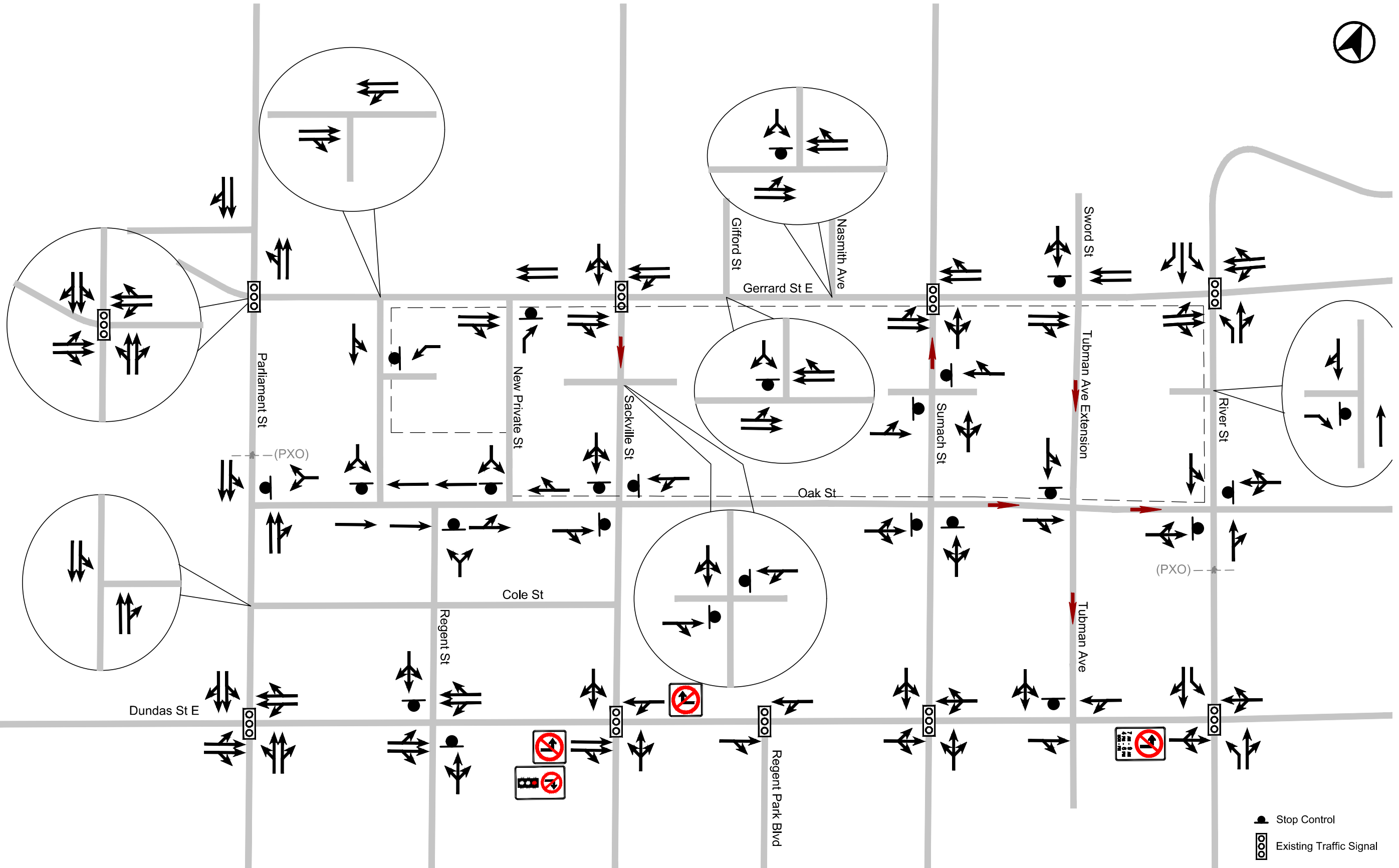


FIGURE 2 PROPOSED LANE CONFIGURATION AND TRAFFIC CONTROL

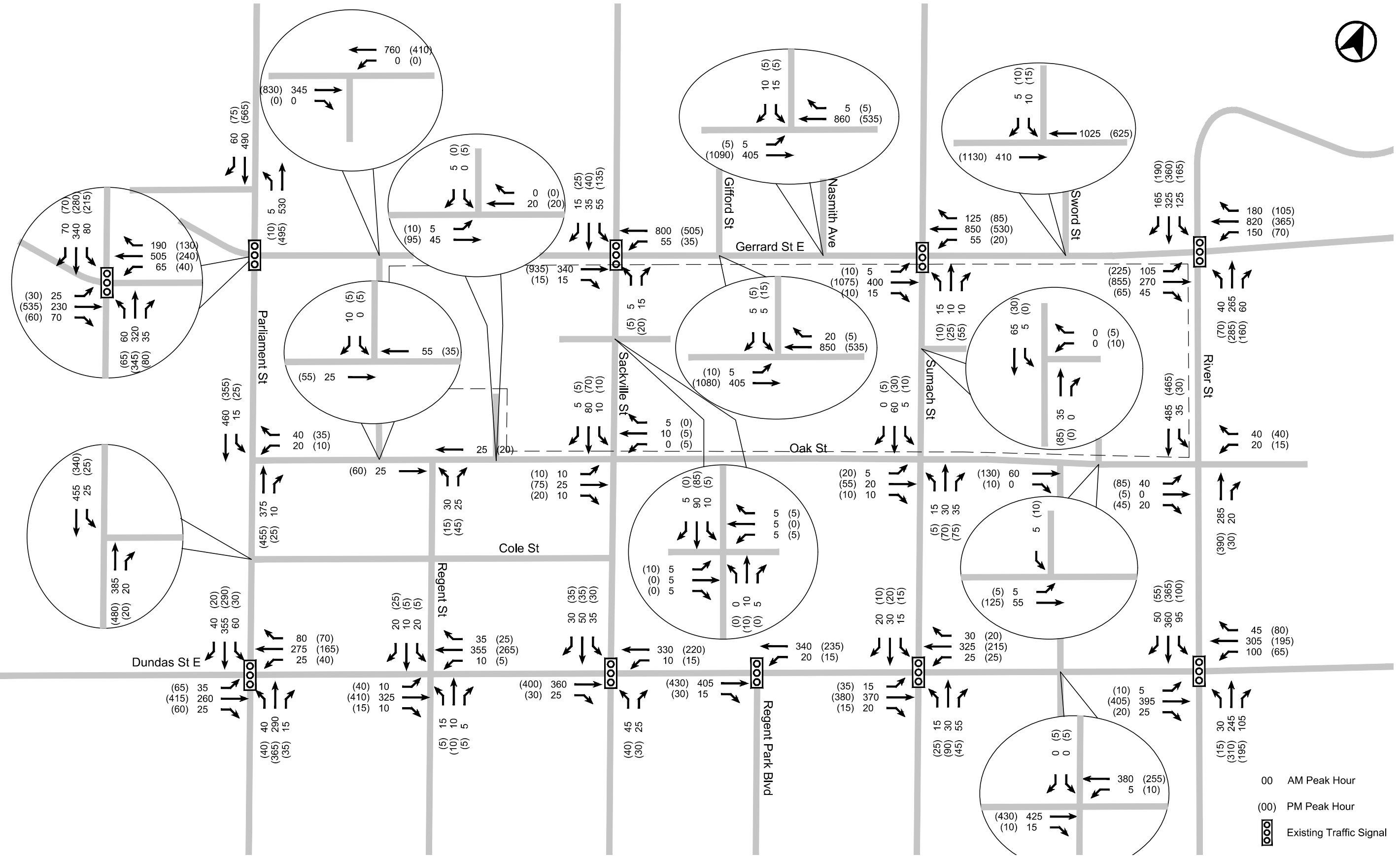


FIGURE 3 BASELINE EXISTING TRAFFIC VOLUMES

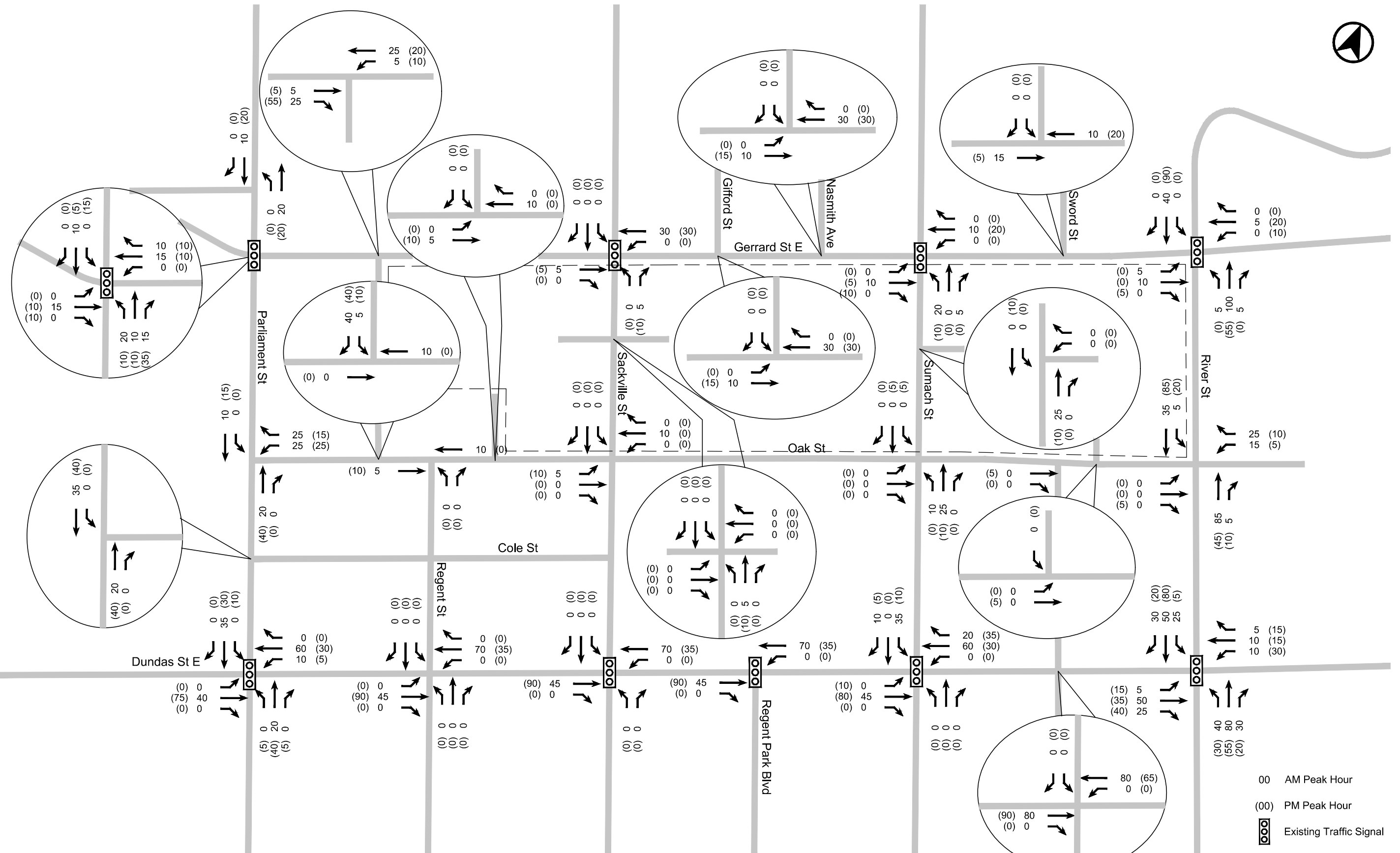
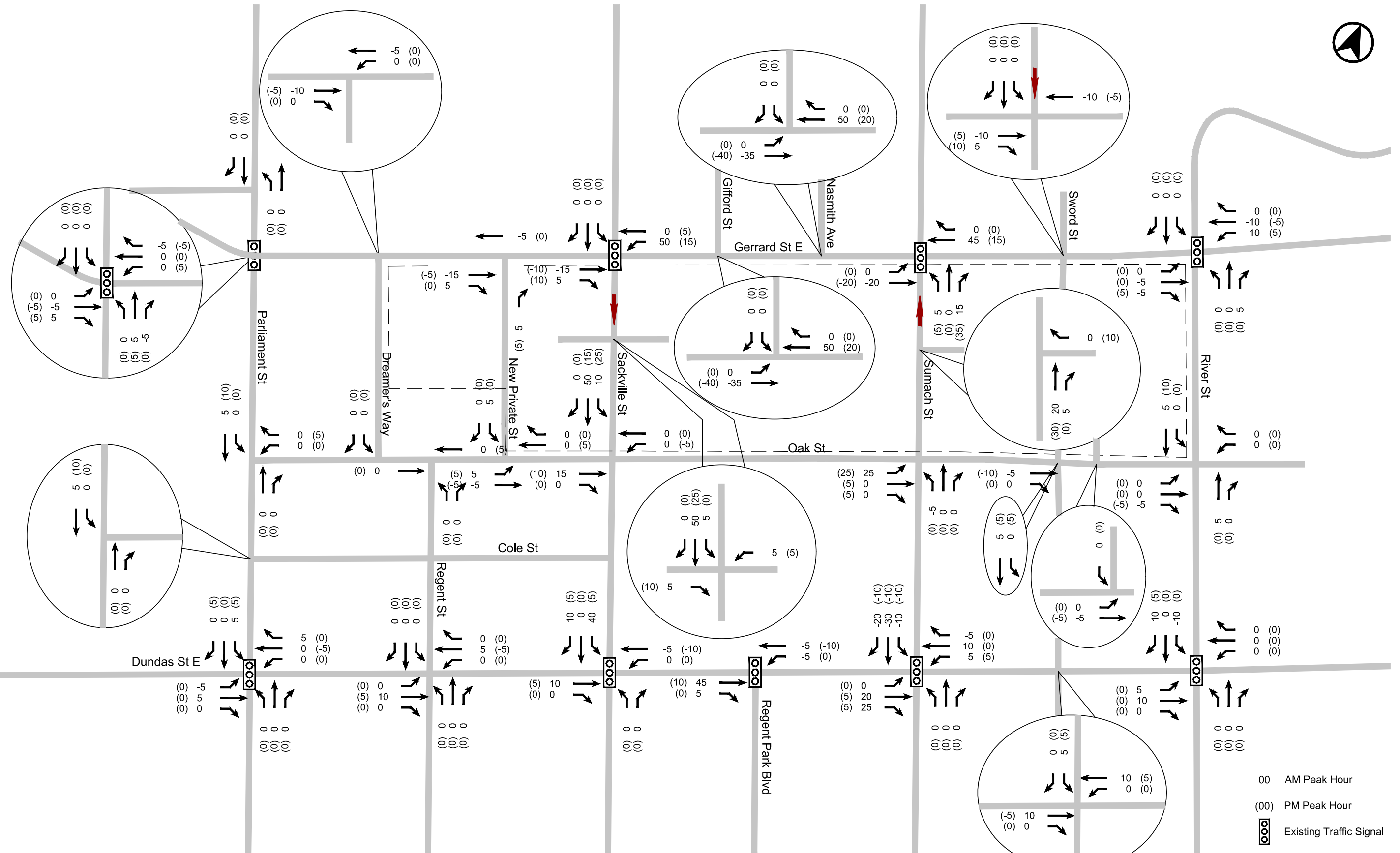
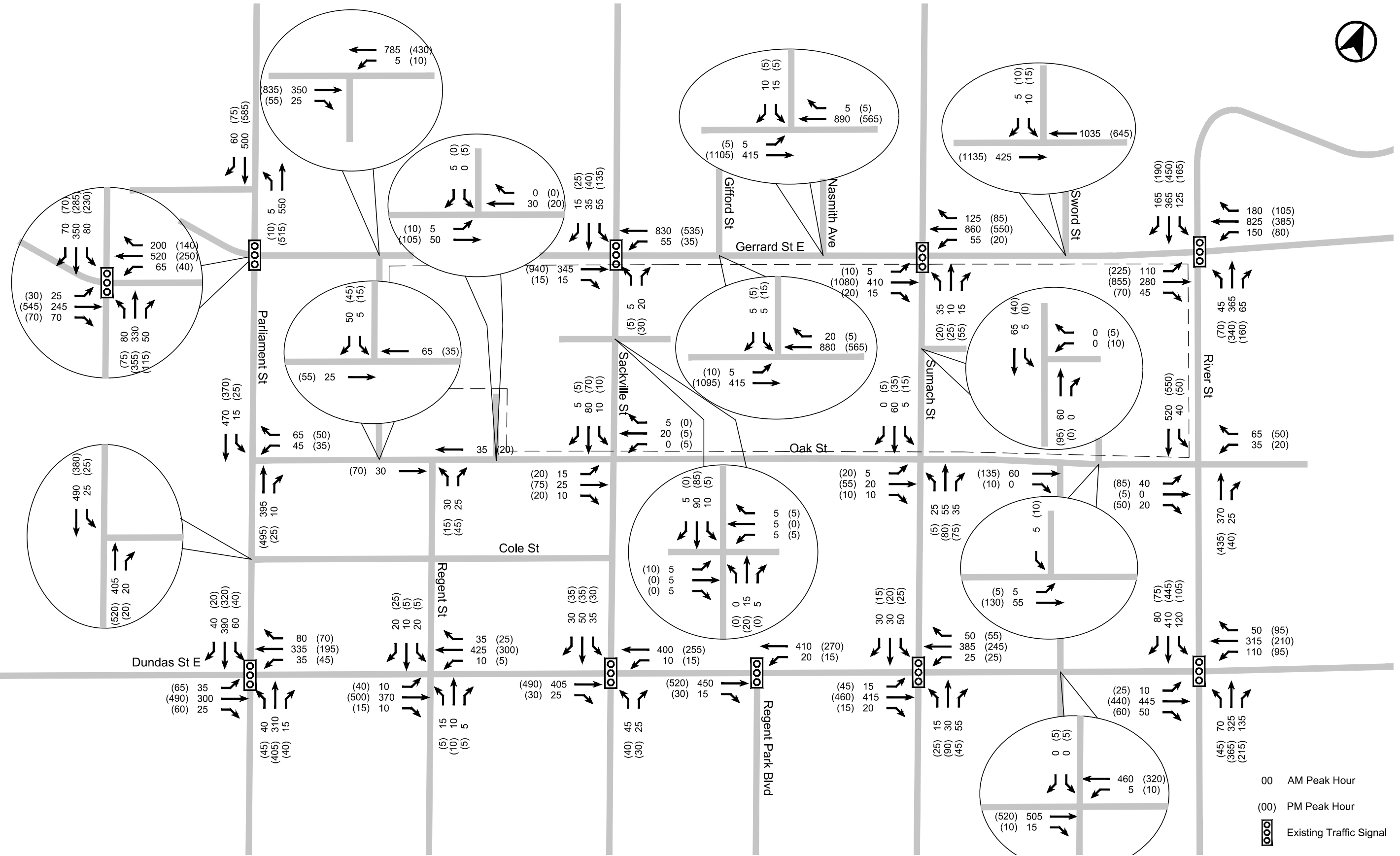


FIGURE 4 BACKGROUND DEVELOPMENT TRAFFIC ALLOWANCES

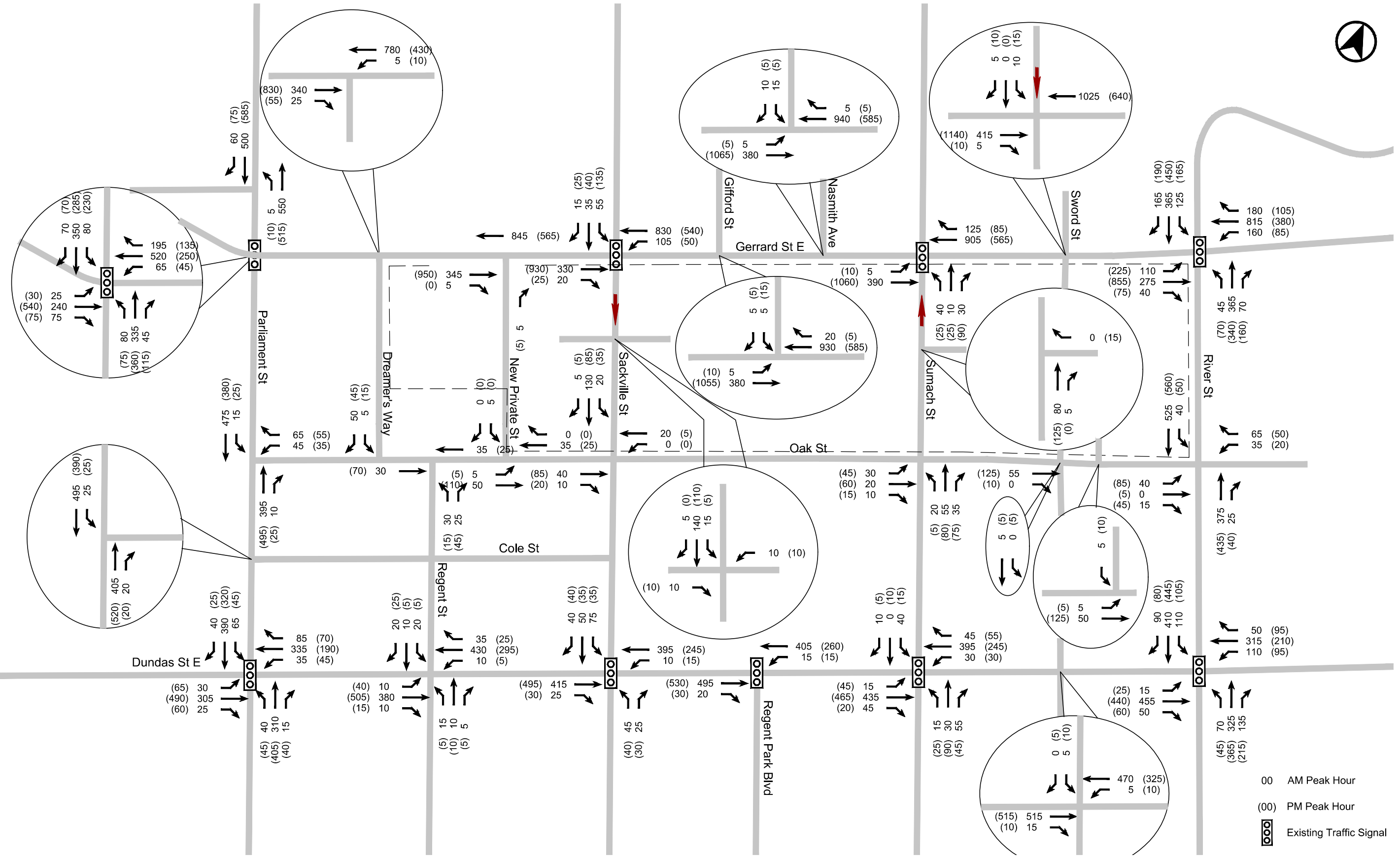


**FIGURE 5 PROPOSED ROAD NETWORK TRAFFIC VOLUME ADJUSTMENTS**  
(PROPOSED ROAD NETWORK)



**FIGURE 6 2032 FUTURE BACKGROUND TRAFFIC VOLUMES**  
 (EXISTING ROAD NETWORK)

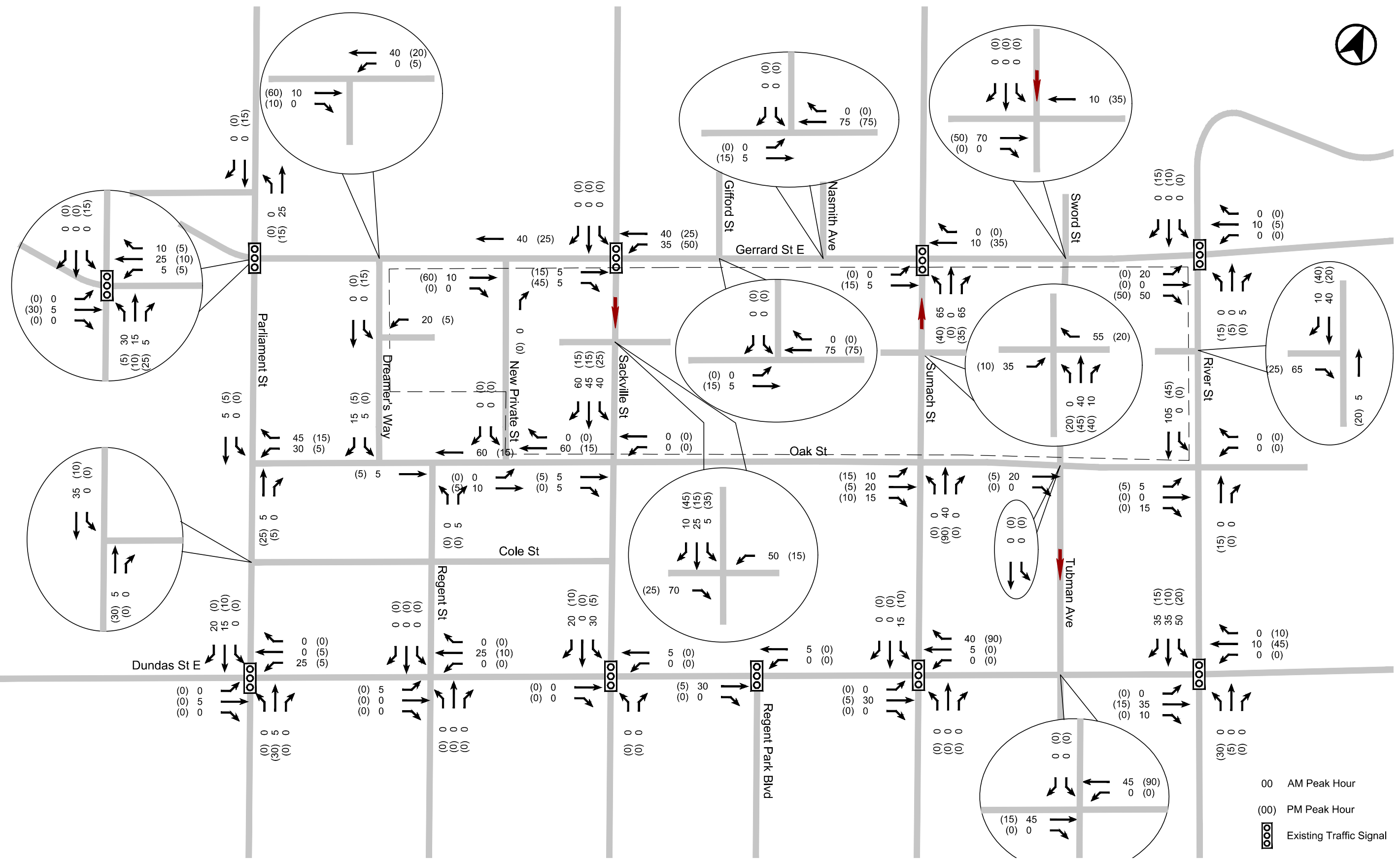
REGENT PARK PHASES 4 & 5



**FIGURE 7 2032 FUTURE BACKGROUND TRAFFIC VOLUMES**  
(PROPOSED ROAD NETWORK)

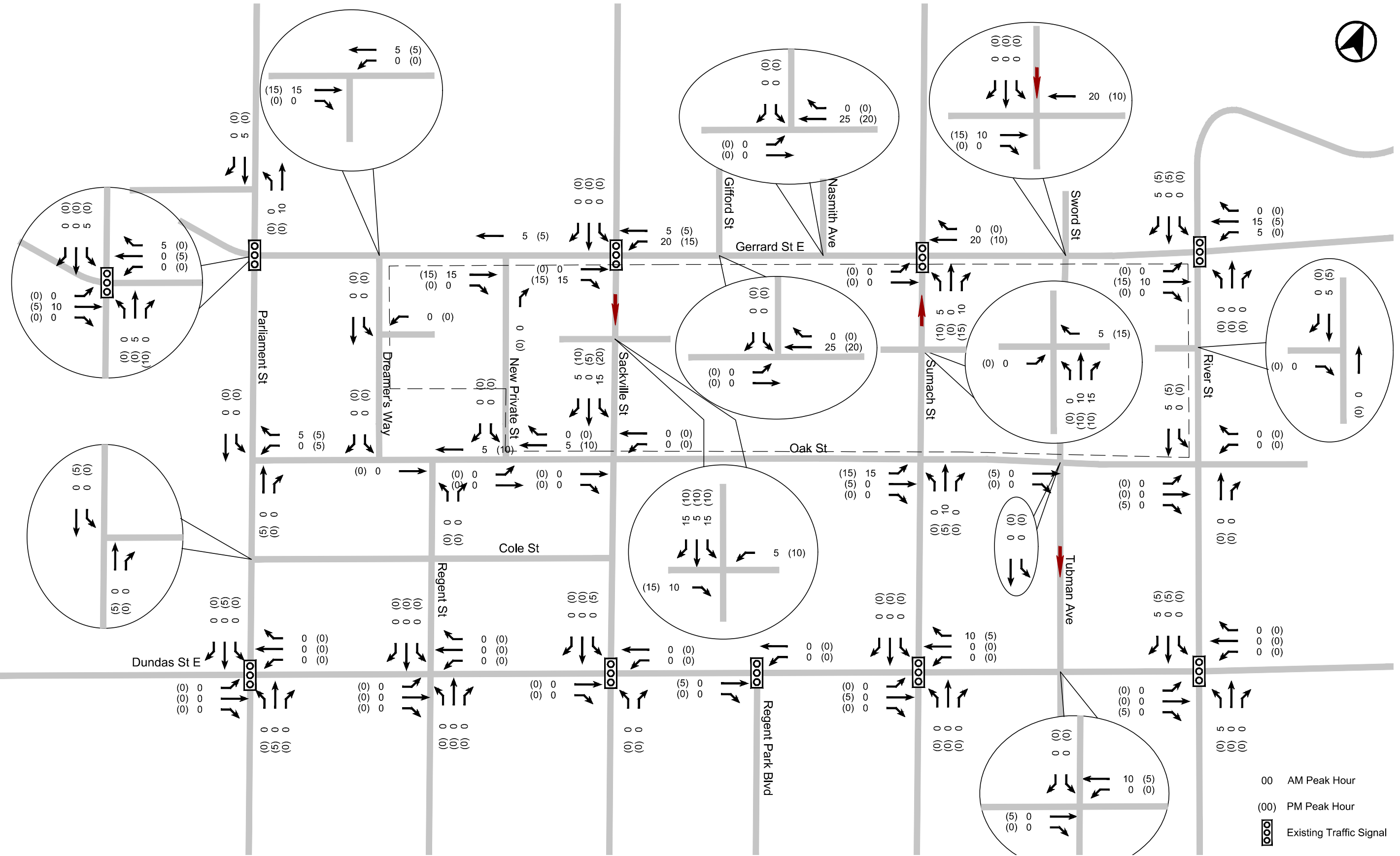






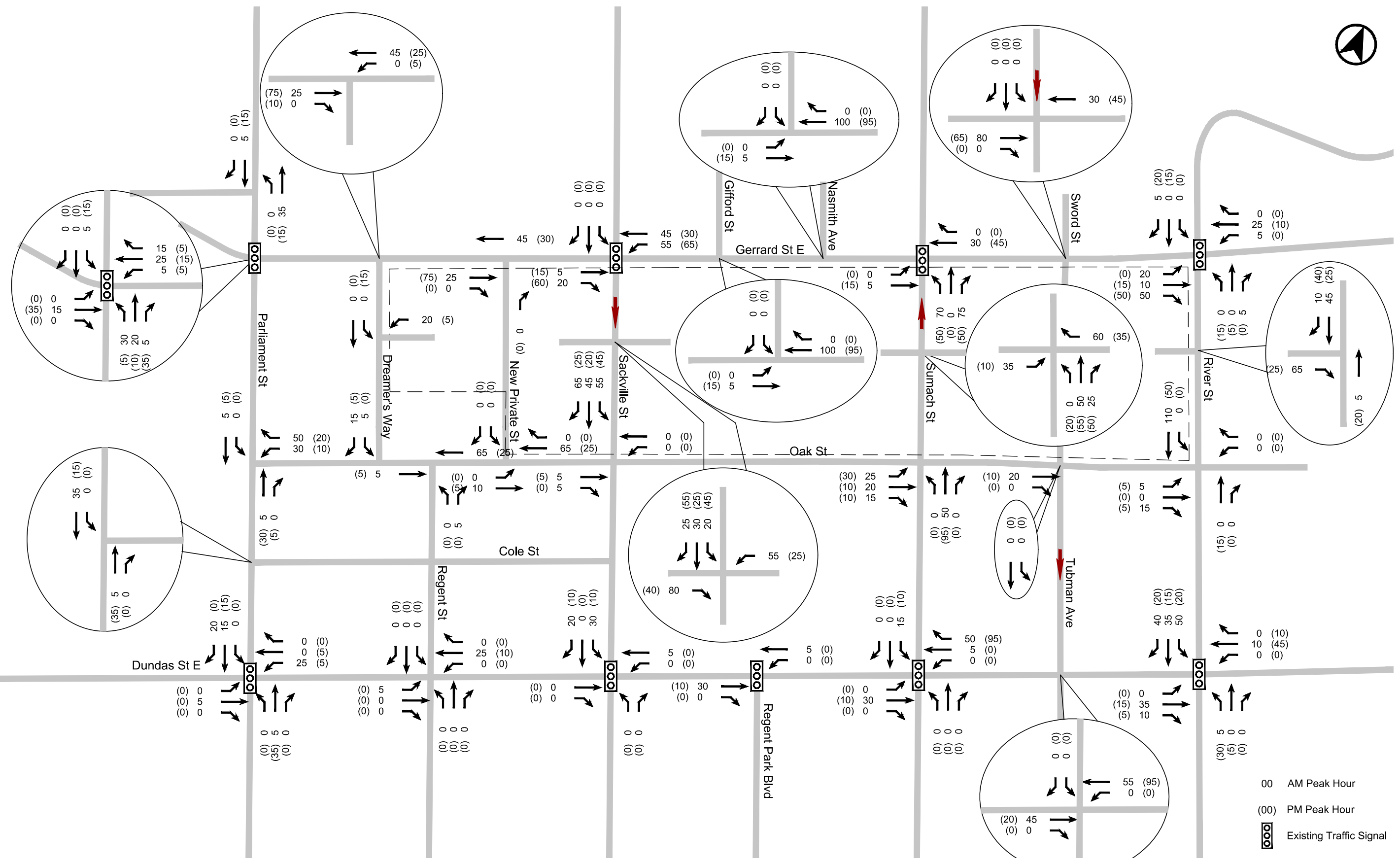
**FIGURE 9 RESIDENTIAL SITE TRAFFIC VOLUMES**  
(PROPOSED ROAD NETWORK)

REGENT PARK PHASES 4 & 5



**FIGURE 10 NON-RESIDENTIAL SITE TRAFFIC VOLUMES**  
(PROPOSED ROAD NETWORK)

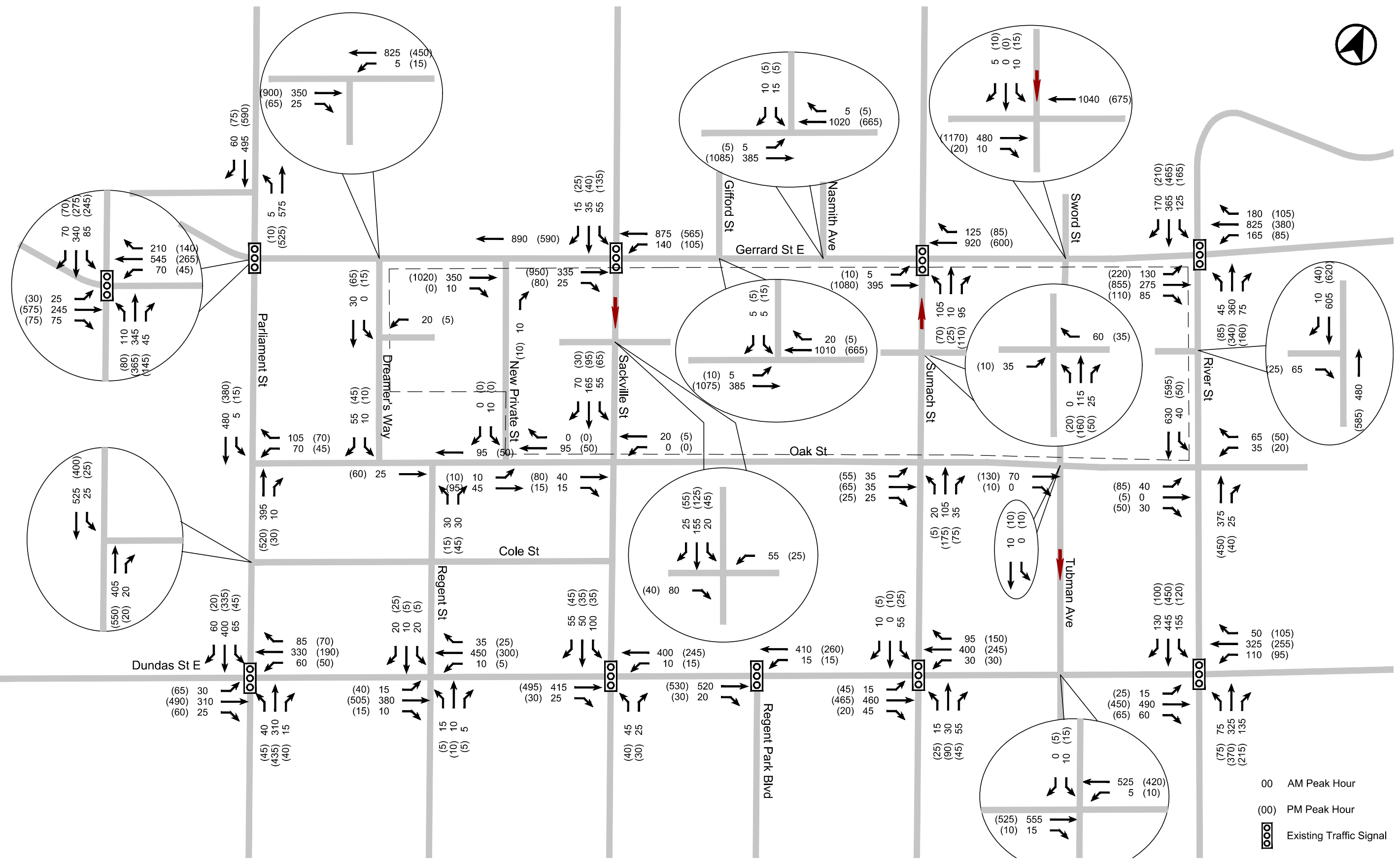
REGENT PARK PHASES 4 & 5



00 AM Peak Hour  
 (00) PM Peak Hour  
 Existing Traffic Signal

**FIGURE 11 TOTAL SITE TRAFFIC VOLUMES**  
 (PROPOSED ROAD NETWORK)  
 REGENT PARK PHASES 4 & 5

Date Plotted: November 11, 2022 Filename: P:\1757546\Graphics\CAD\Fig12-00-2032FTT\_FRN.dwg



**FIGURE 12 2032 FUTURE TOTAL TRAFFIC VOLUMES**  
(PROPOSED ROAD NETWORK)

REGENT PARK PHASES 4 & 5

**Appendix G:  
Updated Synchro Worksheets**



Existing AM Model  
10-25-2022

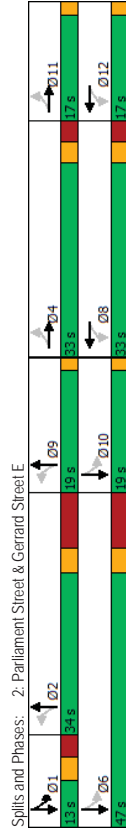
1: Parliament Street & Gerrard Street E (North Section)

Existing AM Model  
10-25-2022

2: Parliament Street & Gerrard Street E

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				4TB	4TB	
Traffic Volume (veh/h)	0	0	5	530	490	60
Future Volume (Veh/h)	0	0	5	530	490	60
Sign Control	Slop					
Grade	0%					
Peak Hour Factor	0.92					
Hourly flow rate (vph)	0	0	5	576	533	65
Pedestrians	80					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.1					
Percent Blockage	0					
Right turn flare (veh)	None					
Median type	None					
Median storage (veh)	None					
Upstream signal (m)	39					
pX platoon unblocked	0.94					
VC, conflicting volume	948					
VC1, stage 1 conf vol	379					
VC2, stage 2 conf vol	678					
VCU, unblocked vol	808					
IC, single (s)	6.8					
IC, 2 stage (s)	6.9					
p0 queue free %	3.5					
IF (s)	3.3					
CM capacity (veh/h)	100					
Direction, Lane #	NB 1		NB 2		SB 2	
Volumes Total	197		384		355	
Volume Left	5		0		0	
Volume Right	0		0		65	
cSH	799		1700		1700	
Volumes to Capacity	0.01		0.23		0.21	
Queue Length 95th (m)	0.1		0.0		0.0	
Control Delay (s)	0.3		0.0		0.0	
Lane LOS	A		A		A	
Approach Delay (s)	0.1		0.0		0.0	
Approach LOS	A		A		A	
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	21.5%					
Analysis Period (min)	15					
ICU Level of Service	A					

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB
Traffic Volume (vph)	25	230	65	505	60	320	80	340
Future Volume (vph)	25	230	65	505	60	320	80	340
Turn Type	Perm	NA	Perm	NA	Perm	NA	custom	NA
Protected Phases	4 11							
Permitted Phases	4	4	8	8	2	2	1	1
Switch Phase	4 11							
Minimum Initial (s)	6.0							
Minimum Split (s)	12.5							
Total Split (s)	13.0							
Total Split (%)	11.2%							
Yellow Time (s)	3.3							
All-Red Time (s)	3.2							
Lost Time Adjust (s)								
Total Lost Time (s)								
Lead/Lag	Lead Lag							
Recall Mode	Yes Yes							
Lead-Lag Optimize?	None							
Act Effct Green (s)	32.8		32.8		30.3		48.7	
Actuated g/C Ratio	0.36		0.36		0.33		0.53	
w/C Ratio	0.33		0.78		0.53		0.36	
Control Delay	20.6		31.2		27.7		13.3	
Queue Delay	0.0		0.0		0.0		0.0	
Total Delay	20.6		31.2		27.7		13.3	
LOS	C		C		C		B	
Approach Delay	20.6		31.2		27.7		13.3	
Approach LOS	C		C		C		B	
Intersection Summary								
Cycle Length	116							
Actuated Cycle Length	92.2							
Natural Cycle	115							
Control Type	Actuated-Uncoordinated							
Maximum w/C Ratio	0.78							
Intersection Signal Delay	24.3							
Intersection Capacity Utilization	98.8%							
Analysis Period (min)	15							



Timings  
2: Parliament Street & Gerrard Street E

Queues  
2: Parliament Street & Gerrard Street E

Existing AM Model  
10-25-2022

Existing AM Model  
10-25-2022

Lane Group	Ø9	Ø10	Ø11	Ø12
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Turn Type	9	10	11	12
Protected Phases				
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0	5.0	5.0	5.0
Minimum Split (s)	19.0	19.0	17.0	17.0
Total Split (s)	19.0	19.0	17.0	17.0
Total Split (%)	16%	16%	15%	15%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	None	None
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Intersection Summary				

	EBT	WBT	NBT	SBT
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	342	800	437	516
v/c Ratio	0.33	0.78	0.53	0.36
Control Delay	20.6	31.2	27.7	13.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	20.6	31.2	27.7	13.3
Queue Length 50th (m)	17.2	52.2	28.8	20.9
Queue Length 95th (m)	36.2	98.3	53.7	43.1
Internal Link Dist (m)	33.0	63.9	119.2	15.0
Turn Bay Length (m)				
Base Capacity (vph)	1058	1082	839	1459
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.32	0.74	0.52	0.35
Intersection Summary				

2. Parliament Street & Gerrard Street E

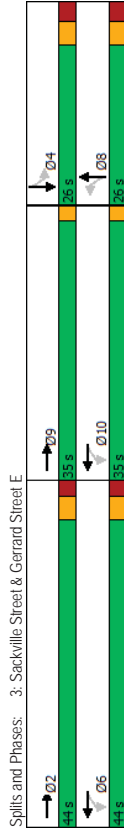
Existing AM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB		4TB		4TB		4TB		4TB
Traffic Volume (vph)	25	230	70	65	505	190	60	320	35	80	340	70
Future Volume (vph)	25	230	70	65	505	190	60	320	35	80	340	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Fpb. ped/bikes	0.98	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.97	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Flt Protected	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Sat'd Flow (prot)	3310	3202	3198	3198	3198	3198	3198	3198	3198	3198	3198	3198
Flt Permitted	0.86	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Sat'd Flow (perm)	2867	2827	2827	2827	2827	2827	2827	2827	2827	2827	2827	2827
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	26	242	74	68	532	200	63	337	37	84	358	74
RTOR Reduction (vph)	0	21	0	0	27	0	0	5	0	0	13	0
Lane Group Flow (vph)	0	321	0	0	773	0	0	432	0	0	503	0
Conf. Peds. (#/hr)	80	85	85	80	85	80	85	120	120	120	85	85
Conf. Bikes (#/hr)	5	5	5	5	5	5	5	5	5	5	5	5
Heavy Vehicles (%)	13%	0%	2%	1%	1%	11%	6%	8%	0%	11%	6%	2%
Turn Type	Perim	MA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	MA
Protected Phases	4 11		8 12		2 9		2 9		6 10		1 16 10	
Permitted Phases	4 11		8 12		2 9		2 9		6 10		1 16 10	
Actuated Green, G (s)	37.2	37.2	37.2	37.2	37.2	37.2	37.2	37.2	37.2	37.2	37.2	37.2
Effective Green, g (s)	38.2	38.2	38.2	38.2	38.2	38.2	38.2	38.2	38.2	38.2	38.2	38.2
Actuated g/C Ratio	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40	0.40
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1155	1139	1090									1277
v/s Ratio Prot												c0.03
v/s Ratio Perm	0.11	c0.27	60.17									0.15
v/c Ratio	0.28	0.68	0.40									0.39
Uniform Delay, d1	19.0	23.3	18.2									16.4
Progression Factor	1.00	1.00	1.00									1.00
Incremental Delay, d2	0.1	1.6	0.2									0.2
Delay (s)	19.2	24.9	18.5									16.6
Level of Service	B	C	B									B
Approach Delay (s)	19.2	24.9	18.5									16.6
Approach LOS	B	C	B									B
Intersection Summary												
HCM 2000 Control Delay	20.6 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.62											
Actuated Cycle Length (s)	94.8 Sum of lost time (s) 22.5											
Intersection Capacity Utilization	98.8% ICU Level of Service F											
Analysis Period (min)	15											
c Critical Lane Group												

3. Sackville Street & Gerrard Street E

Existing AM Model  
10-25-2022

Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø6	Ø2	Ø9	Ø10	
Lane Configurations	4TB		4TB		4TB		4TB		4TB		4TB	
Traffic Volume (vph)	340	55	800	5	0	55	35					
Future Volume (vph)	340	55	800	5	0	55	35					
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA					
Protected Phases	2 9		6 10		8		4					
Permitted Phases	2 9		6 10		8		4					
Detector Phase												
Switch Phase												
Minimum Initial (s)				20.0	20.0	20.0	20.0	16.0	16.0	5.0	5.0	
Minimum Split (s)				25.7	25.7	25.7	21.0	21.0	21.0	35.0	35.0	
Total Spill (s)				26.0	26.0	26.0	26.0	44.0	44.0	35.0	35.0	
Total Spill (%)				24.8%	24.8%	24.8%	24.8%	42%	42%	33%	33%	
Yellow Time (s)				3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0	
All-Red Time (s)				2.7	2.7	2.7	2.7	2.0	2.0	0.0	0.0	
Lost Time Adjust (s)				-1.0	-1.0	-1.0	-1.0					
Total Lost Time (s)				4.7	4.7	4.7	4.7					
Lead-Lag												
Lead-Lag Optimize?												
Recall Mode				Min	Min	Min	Min	Min	Min	None	None	
Act Effect Green (s)	43.6		43.6		23.0		23.0					
Actuated g/C Ratio	0.57		0.57		0.30		0.30					
v/c Ratio	0.21		0.54		0.05		0.28					
Control Delay	6.5		9.7		4.2		29.6					
Queue Delay	0.0		0.0		0.0		0.0					
Total Delay	6.5		9.7		4.2		29.6					
LOS	A		A		A		C					
Approach Delay	6.5		9.7		4.2		29.6					
Approach LOS	A		A		A		C					
Intersection Summary												
Cycle Length: 105												
Actuated Cycle Length: 76.1												
Natural Cycle: 85												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.54												
Intersection Signal Delay: 10.3	Intersection LOS: B											
Intersection Capacity Utilization 64.3%	ICU Level of Service C											
Analysis Period (min) 15												





Existing AM Model  
10-25-2022

HCM Signalized Intersection Capacity Analysis  
3. Sackville Street & Gerrard Street E

Existing AM Model  
10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	390	939	21	114
Lane Group Flow (vph)	0.21	0.54	0.05	0.28
v/c Ratio	6.5	9.7	4.2	29.6
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	6.5	9.7	4.2	29.6
Total Delay	11.3	35.7	0.0	15.3
Queue Length 50th (m)	16.2	46.6	2.7	33.6
Queue Length 95th (m)	139.8	47.8	34.3	15.3
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2331	2221	427	412
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.17	0.42	0.05	0.28
<b>Intersection Summary</b>				

	EBT	EBR	WBL	WBT	NBL	NBR	SBL	SBT	SBR
Lane Configurations	4+4	4+4	4+4	4+4	4+4	4+4	4+4	4+4	4+4
Traffic Volume (vph)	0	340	15	55	800	0	5	0	15
Future Volume (vph)	0	340	15	55	800	0	5	0	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.5	3.0	3.0	3.5	3.0	3.5
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.99	0.99	1.00	1.00	0.87	0.87	0.99	0.99	0.99
Frbp. psd/bikes	1.00	1.00	0.99	0.99	0.99	0.99	0.92	0.92	0.92
Frt	0.99	0.99	1.00	1.00	0.90	0.90	0.98	0.98	0.98
Flt Protected	1.00	1.00	1.00	1.00	0.99	0.99	0.97	0.97	0.97
Satd. Flow (prot)	3215	3215	3393	3393	1432	1432	1685	1685	1685
Flt Permitted	1.00	1.00	0.90	0.90	0.95	0.95	0.85	0.85	0.85
Satd. Flow (perm)	3215	3215	3066	3066	1376	1376	1382	1382	1382
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	0	374	16	60	879	0	5	0	16
RTOR Reduction (vph)	0	4	0	0	0	0	0	0	4
Lane Group Flow (vph)	0	386	0	0	939	0	6	0	109
Confl. Peds. (#/hr)	40	60	60	60	40	60	140	140	60
Confl. Bikes (#/hr)	15	15	15	15	75	75	140	140	15
Heavy Vehicles (%)	0%	10%	0%	9%	4%	0%	0%	0%	6%
Turn Type	NA	Perm	NA	NA	Perm	NA	Perm	NA	Perm
Protected Phases	2.9	6.10	6.10	6.10	8	8	4	4	4
Permitted Phases	46.8	46.8	46.8	46.8	21.9	21.9	21.9	21.9	21.9
Effective Green, G (s)	47.8	47.8	47.8	47.8	22.9	22.9	22.9	22.9	22.9
Actuated q/C Ratio	0.63	0.63	0.63	0.63	0.30	0.30	0.30	0.30	0.30
Clearance Time (s)	2011	2011	1918	1918	412	412	414	414	414
Vehicle Extension (s)	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12
Lane Grp Cap (vph)	0.19	0.19	0.49	0.49	0.02	0.02	0.26	0.26	0.26
v/s Ratio Perm	6.1	6.1	7.7	7.7	18.8	18.8	20.3	20.3	20.3
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	0.0	0.0	0.2	0.2	0.0	0.0	0.3	0.3	0.3
Incremental Delay, d2	6.1	6.1	7.9	7.9	18.8	18.8	20.7	20.7	20.7
Delay (s)	A	A	A	A	B	B	C	C	C
Level of Service	6.1	6.1	7.9	7.9	18.8	18.8	20.7	20.7	20.7
Approach Delay (s)	A	A	A	A	B	B	C	C	C
Approach LOS	A	A	A	A	B	B	C	C	C
<b>Intersection Summary</b>									
HCM 2000 Control Delay	8.6		HCM 2000 Level of Service		A				
HCM 2000 Volume to Capacity ratio	0.43								
Actuated Cycle Length (s)	76.4								
Sum of lost time (s)	9.7								
Intersection Capacity Utilization	64.3%								
ICU Level of Service	C								
Analysis Period (min)	15								
c Critical Lane Group									

HCM Unsignalized Intersection Capacity Analysis  
 4: Gerrard Street E & Gifford Street

HCM Unsignalized Intersection Capacity Analysis  
 5: Gerrard Street E & Nasmith Avenue

Existing AM Model  
 10-25-2022

Existing AM Model  
 10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		←	←	←	←	←
Traffic Volume (veh/h)	5	405	850	20	5	5
Future Volume (Veh/h)	5	405	850	20	5	5
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	5	422	885	21	5	5
Pedestrians		5	5		50	
Lane Width (m)		3.5	3.5		3.0	
Walking Speed (m/s)		1.1	1.1		1.1	
Percent Blockage		0	0		4	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)		72	141			
Upstream signal (m)		0.93		0.94	0.93	
PX platoon unblocked		956		1172	508	
VC conflicting volume						
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol		801		935	319	
IC single (s)		4.1		7.5	7.3	
IC 2 stage (s)		2.2		3.8	3.5	
p0 queue free %		99		97	99	
CM capacity (veh/h)		743		192	558	
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	146	281	590	316	10	
Volume Left	5	0	0	0	5	
Volume Right	0	0	0	21	5	
cSH	743	1700	1700	1700	285	
Volumes to Capacity	0.01	0.17	0.35	0.19	0.04	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.8	
Control Delay (s)	0.4	0.0	0.0	0.0	18.1	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		18.1	
Approach LOS					C	
Intersection Summary						
Average Delay	0.2					
Intersection Capacity Utilization	35.8%					
ICU Level of Service	A					
Analysis Period (min)	15					

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		←	←	←	←	←
Traffic Volume (veh/h)	5	405	860	5	15	10
Future Volume (Veh/h)	5	405	860	5	15	10
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	5	418	887	5	15	10
Pedestrians					75	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					6	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)		145	68			
Upstream signal (m)		0.90		0.90	0.90	0.90
PX platoon unblocked		967		1184	521	
VC conflicting volume						
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol		742		983	247	
IC single (s)		4.1		6.9	7.1	
IC 2 stage (s)		2.2		3.6	3.4	
p0 queue free %		99		93	98	
CM capacity (veh/h)		742		200	619	
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	144	279	591	301	25	
Volume Left	5	0	0	0	15	
Volume Right	0	0	0	5	10	
cSH	742	1700	1700	1700	275	
Volumes to Capacity	0.01	0.16	0.35	0.18	0.09	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	2.3	
Control Delay (s)	0.4	0.0	0.0	0.0	19.4	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		19.4	
Approach LOS					C	
Intersection Summary						
Average Delay	0.4					
Intersection Capacity Utilization	34.0%					
ICU Level of Service	A					
Analysis Period (min)	15					

Timings Existing AM Model  
10-25-2022

Queues Existing AM Model  
10-25-2022

6: Summach Street & Gerrard Street E

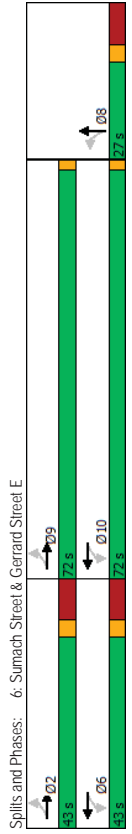
6: Summach Street & Gerrard Street E

Lane Group	EBL	EBT	WBL	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	EBT	EBT	WBT	WBT	NBT				
Traffic Volume (vph)	5	400	55	850	10				
Future Volume (vph)	5	400	55	850	10				
Turn Type	Perm	NA	Perm	NA	NA				
Protected Phases	2,9		6,10	8	2	6	9	10	
Permitted Phases	2,9		6,10	8	2	6	9	10	
Detector Phase	2	2	6	6	8				
Switch Phase									
Minimum Initial (s)					15.0	11.0	11.0	5.0	5.0
Minimum Split (s)					25.4	26.2	26.2	72.0	72.0
Total Split (s)					27.0	43.0	43.0	72.0	72.0
Total Split (%)					19.0%	30%	30%	51%	51%
Yellow Time (s)					3.0	3.0	3.0	2.0	2.0
All-Red Time (s)					7.4	7.2	7.2	0.0	0.0
Lost Time Adjust (s)					-1.0				
Total Lost Time (s)					9.4				
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode					None	Min	Min	None	None
Act Effct Green (s)		72.1		72.1	19.4				
Actuated g/C Ratio		0.74		0.74	0.20				
v/c Ratio		0.21		0.56	0.13				
Control Delay		6.1		9.3	39.9				
Queue Delay		0.0		0.2	0.0				
Total Delay		6.1		9.4	39.9				
LOS		A		A	D				
Approach Delay		6.1		9.4	39.9				
Approach LOS		A		A	D				

Lane Group	EBT	WBT	NBT
Lane Group Flow (vph)	483	1184	39
v/c Ratio	0.21	0.56	0.13
Control Delay	6.1	9.3	39.9
Queue Delay	0.0	0.2	0.0
Total Delay	6.1	9.4	39.9
Queue Length 50th (m)	17.8	62.4	3.7
Queue Length 95th (m)	22.2	71.8	17.8
Internal Link Dist (m)	44.0	75.7	33.2
Turn Bay Length (m)			
Base Capacity (vph)	2181	1990	326
Starvation Cap Reductn	0	181	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.22	0.65	0.12

Intersection Summary	
Cycle Length:	142
Actuated Cycle Length:	98
Natural Cycle:	125
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.56
Intersection Signal Delay:	9.2
Intersection Capacity Utilization:	78.6%
Analysis Period (min):	15
ICU Level of Service:	D

Intersection Summary	
Cycle Length:	142
Actuated Cycle Length:	98
Natural Cycle:	125
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.56
Intersection Signal Delay:	9.2
Intersection Capacity Utilization:	78.6%
Analysis Period (min):	15
ICU Level of Service:	D



6: Sumach Street & Gerrard Street E

7: Gerrard Street E & Sword Street

HCM Signalized Intersection Capacity Analysis  
Existing AM Model  
10-25-2022

HCM Unsignalized Intersection Capacity Analysis  
Existing AM Model  
10-25-2022

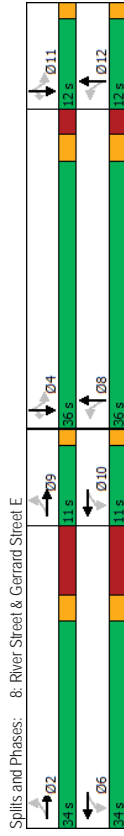
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB			4B					
Traffic Volume (vph)	5	400	15	55	850	125	15	10	10	0	0	0
Future Volume (vph)	5	400	15	55	850	125	15	10	10	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	9.2			9.2			9.4					
Lane Util. Factor	0.95			0.95			1.00					
Fpb. ped/bikes	0.99			0.97			0.96					
Fpb. ped/bikes	1.00			0.99			0.91					
Frt	0.99			0.98			0.96					
Flt Protected	1.00			1.00			0.98					
Sat'd. Flow (prot)	3852			3251			1548					
Flt Permitted	0.94			0.89			0.98					
Sat'd. Flow (perm)	3148			2887			1548					
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	6	460	17	63	977	144	17	11	11	0	0	0
RTOR Reduction (vph)	0	2	0	0	7	0	0	10	0	0	0	0
Lane Group Flow (vph)	0	481	0	0	1177	0	0	29	0	0	0	0
Conf. Peds. (#/hr)	65	85	85	65	100	65	100	65	65	65	100	100
Conf. Bikes (#/hr)												
Heavy Vehicles (%)	0%	5%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perim	MA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2.9			6.10			8					
Permitted Phases	2.9			6.10			8					
Effective Green, G (s)	76.5			76.5			11.2					
Actuated Green, g (s)	77.5			77.5			12.2					
Actuated g/C Ratio	0.77			0.77			0.12					
Clearance Time (s)							10.4					
Vehicle Extension (s)							3.0					
Lane Grp Cap (vph)	2437			2235			188					
v/s Ratio Prot												
v/s Ratio Perm	0.15			0.41			0.02					
v/c Ratio	0.20			0.53			0.16					
Uniform Delay, d1	3.0			4.3			39.3					
Progression Factor	1.00			1.00			1.00					
Incremental Delay, d2	0.0			0.2			0.4					
Delay (s)	3.1			4.5			39.7					
Level of Service	A			A			D					
Approach Delay (s)	3.1			4.5			39.7					
Approach LOS	A			A			D					
Intersection Summary												
HCM 2000 Control Delay	4.9 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	0.52											
Actuated Cycle Length (s)	100.1 Sum of lost time (s)											
Intersection Capacity Utilization	78.6% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group												

Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR
Lane Configurations		4TB		4TB			W	
Traffic Volume (veh/h)	0	410	1025	0	1025	0	10	5
Future Volume (Veh/h)	0	410	1025	0	1025	0	10	5
Sign Control	Free	Free	Free	Stop	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	432	1079	0	11	5		
Pedestrians							75	
Lane Width (m)							3.0	
Walking Speed (m/s)							1.1	
Percent Blockage							6	
Right turn flare (veh)								
Median type	None	None	None	None	None	None		
Median storage (veh)								
Upstream signal (m)	100	91						
pX platoon unblocked	0.78						0.79	0.78
vC, conflicting volume	1154						1370	614
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	643						880	0
IC, single (s)	4.1						6.8	6.9
IC, 2 stage (s)								
IF (s)	2.2						3.5	3.3
p0 queue free %	100						95	99
dM capacity (veh/h)	703						216	806
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 1		
Volumes Total	216	216	540	540	16	16		
Volume Left	0	0	0	0	11	11		
Volume Right	0	0	0	0	5	5		
CSH	1700	1700	1700	1700	279	279		
Volumes to Capacity	0.13	0.13	0.32	0.32	0.06	0.06		
Queue Length 95th (m)	0.0	0.0	0.0	0.0	1.4	1.4		
Control Delay (s)	0.0	0.0	0.0	0.0	18.7	18.7		
Lane LOS					C	C		
Approach Delay (s)	0.0	0.0	0.0	0.0	18.7	18.7		
Approach LOS					C	C		
Intersection Summary								
Average Delay	0.2							
Intersection Capacity Utilization	38.3% ICU Level of Service							
Analysis Period (min)	15							

Timings  
8: River Street & Gerrard Street E

Existing AM Model  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Lane Configurations	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB			
Traffic Volume (vph)	105	270	150	820	40	265	125	325	165			
Future Volume (vph)	105	270	150	820	40	265	125	325	165			
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm			
Protected Phases	2.9	6.10	6.10	8.12	4.11	4.11	4.11	4.11	4.11	2	4	6
Permitted Phases	2	2	6	6	8	8	4	4	4			
Detector Phase												
Switch Phase												
Minimum Initial (s)										23.0	19.0	23.0
Minimum Spilt (s)										33.8	30.0	33.8
Total Spilt (s)										34.0	36.0	34.0
Total Spilt (%)										37%	39%	37%
Yellow Time (s)										3.0	3.0	3.0
All-Red Time (s)										7.8	3.0	7.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode										Min	Min	Min
Act Effct Green (s)	30.8	30.8	29.4	29.4	29.4	29.4	29.4	29.4	29.4			
Actuated g/C Ratio	0.41	0.41	0.39	0.39	0.39	0.39	0.39	0.39	0.39			
v/C Ratio	0.63	1.10	0.14	0.50	0.43	0.47	0.28					
Control Delay	23.3	84.0	16.6	19.5	22.4	19.8	7.3					
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0					
Total Delay	23.3	84.0	16.6	19.5	22.4	19.8	7.3					
LOS	C	F	B	B	C	B	A					
Approach Delay	23.3	84.0	19.2	17.0								
Approach LOS	C	F	B	B								
Intersection Summary												
Cycle Length: 93												
Actuated Cycle Length: 75.5												
Natural Cycle: 90												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 1.10												
Intersection Signal Delay: 48.6												
Intersection Capacity Utilization: 113.0%												
Analysis Period (min): 15												



Timings  
8: River Street & Gerrard Street E

Existing AM Model  
10-25-2022

Lane Group	Ø8	Ø9	Ø10	Ø11	Ø12
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Turn Type					
Protected Phases	8	9	10	11	12
Permitted Phases					
Detector Phase					
Switch Phase					
Minimum Initial (s)	19.0	5.0	5.0	5.0	5.0
Minimum Spilt (s)	30.0	11.0	11.0	12.0	12.0
Total Spilt (s)	36.0	11.0	11.0	12.0	12.0
Total Spilt (%)	39%	12%	12%	13%	13%
Yellow Time (s)	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)	3.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	Min	None	None	None	None
Actuated g/C Ratio					
v/C Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Intersection Summary					

Existing AM Model  
10-25-2022

HCM Signalized Intersection Capacity Analysis  
8: River Street & Gerrard Street E

Existing AM Model  
10-25-2022

	EBT	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group	437	1198	42	339	130	339	172
Lane Group Flow (vph)	0.63	1.10	0.14	0.50	0.43	0.47	0.28
v/c Ratio	23.3	84.0	16.6	19.5	22.4	19.8	7.3
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	23.3	84.0	16.6	19.5	22.4	19.8	7.3
Total Delay	21.2	80.7	4.1	37.1	14.5	38.6	5.5
Queue Length 50th (m)	49.8	#175.6	10.3	58.5	28.8	59.3	16.9
Queue Length 95th (m)	67.0	81.6	126.5			61.7	
Internal Link Dist (m)			30.0			30.0	
Turn Bay Length (m)	695	1086	353	803	352	840	701
Base Capacity (vph)	0	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.63	1.10	0.12	0.42	0.37	0.40	0.25
<b>Intersection Summary</b>							
#	95th percentile volume exceeds capacity, queue may be longer.						
	Queue shown is maximum after two cycles.						

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB			4TB			4TB		
Traffic Volume (vph)	105	270	45	150	820	180	40	265	60	125	325	165
Future Volume (vph)	105	270	45	150	820	180	40	265	60	125	325	165
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Frbp. psd/bikes	1.00	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Frt	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Flt Protected	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Flt Permitted	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52
Satd. Flow (perm)	1684	1684	1684	2638	2638	2638	769	1730	787	1860	1425	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	109	281	47	156	854	188	42	276	62	130	339	172
RTOR Reduction (vph)	0	8	0	0	12	0	0	9	0	0	0	0
Lane Group Flow (vph)	0	429	0	0	1186	0	42	330	0	130	339	109
Confl. Peds. (#/hr)	50	95	95	95	95	95	50	40	85	85	40	10
Confl. Bikes (#/hr)	10	15	15	15	15	15	10	15	15	15	10	10
Heavy Vehicles (%)	4%	8%	4%	0%	0%	3%	1%	8%	4%	2%	3%	0%
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	Perm	NA
Protected Phases	2.9			6.10			8.12			4.11		4.11
Permitted Phases	2.9			6.10			8.12			4.11		4.11
Actuated Green, G (s)	39.5			39.5			33.8			33.8		33.8
Effective Green, g (s)	40.5			40.5			34.8			34.8		34.8
Actuated g/C Ratio	0.52			0.52			0.45			0.45		0.45
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	882			1382			346			778		641
v/s Ratio Prot										0.19		0.18
v/s Ratio Perm	0.25			0.45			0.05			0.17		0.08
v/c Ratio	0.49			0.86			0.12			0.37		0.41
Uniform Delay, d1	11.8			15.9			12.4			14.4		12.6
Progression Factor	1.00			1.00			1.00			1.00		1.00
Incremental Delay, d2	0.4			5.5			0.2			0.6		0.3
Delay (s)	12.2			21.4			12.5			14.8		12.8
Level of Service	B			C			B			B		B
Approach Delay (s)	12.2			21.4			14.6			14.1		14.1
Approach LOS	B			C			B			B		B
<b>Intersection Summary</b>												
HCM 2000 Control Delay	17.2 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.79											
Actuated Cycle Length (s)	77.3											
Intersection Capacity Utilization	113.0% Sum of lost time (s) 16.8 H											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 9: Sackville Street & Site Driveway

HCM Unsignalized Intersection Capacity Analysis  
 10: Parliament Street & Oak Street

Existing AM Model  
 10-25-2022

Existing AM Model  
 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Traffic Volume (veh/h)	5	5	5	5	5	5	0	10	5	10	90	5
Future Volume (Veh/h)	5	5	5	5	5	5	0	10	5	10	90	5
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	6	6	6	6	6	6	0	13	6	13	117	6
Pedestrians	70	70	70	40	40	40	15	15	15	10	10	10
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	6	6	6	4	4	4	1	1	1	1	1	1
Right turn flare (veh)												
Median type							None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)												58
PX platoon unblocked												
VC, conflicting volume	251	275	205	226	275	66	193					59
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	251	275	205	226	275	66	193					59
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1					4.1
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2					2.2
p0 queue free %	99	99	99	99	99	99	100					99
CM capacity (veh/h)	597	570	778	633	570	959	1306					1503
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volumes Total	18	18	19	136								
Volume Left	6	6	0	13								
Volume Right	6	6	6	6								
cSH	636	686	1306	1503								
Volumes to Capacity	0.03	0.03	0.00	0.01								
Queue Length 95th (m)	0.7	0.6	0.0	0.2								
Control Delay (s)	10.8	10.4	0.0	0.8								
Lane LOS	B	B	A	A								
Approach Delay (s)	10.8	10.4	0.0	0.8								
Approach LOS	B	B	A	A								
Intersection Summary												
Average Delay	2.5											
Intersection Capacity Utilization	27.2%											
ICU Level of Service	A											
Analysis Period (min)	15											

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	W	T	T	T	T
Traffic Volume (veh/h)	20	40	375	10	15	460
Future Volume (Veh/h)	20	40	375	10	15	460
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	21	42	395	11	16	484
Pedestrians	260	260	5	5	175	175
Lane Width (m)	3.0	3.0	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	20	20	0	0	15	15
Right turn flare (veh)						
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)			151	151	143	143
PX platoon unblocked						
VC, conflicting volume	940	638			666	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	914	638			666	
IC, single (s)	6.8	7.0			4.1	
IC, 2 stage (s)						
IF (s)	3.5	3.3			2.2	
p0 queue free %	90	85			98	
CM capacity (veh/h)	214	280			749	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volumes Total	63	263	143	177	323	
Volume Left	21	0	0	16	0	
Volume Right	42	0	11	0	0	
cSH	254	1700	1700	749	1700	
Volumes to Capacity	0.25	0.15	0.08	0.02	0.19	
Queue Length 95th (m)	7.2	0.0	0.0	0.5	0.0	
Control Delay (s)	23.8	0.0	0.0	1.1	0.0	
Lane LOS	C	A	A	A	A	
Approach Delay (s)	23.8	0.0	0.0	0.4	0.0	
Approach LOS	C	A	A	A	A	
Intersection Summary						
Average Delay	1.8					
Intersection Capacity Utilization	43.6%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis  
11: Oak Street & Dreamer's Way

Existing AM Model  
10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Traffic Volume (veh/h)	0	25	55	0	0	10
Future Volume (Veh/h)	0	25	55	0	0	10
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Hourly flow rate (vph)	0	30	67	0	0	12
Pedestrians	15				40	
Lane Width (m)	3.5	3.5	3.0	3.0		
Walking Speed (m/s)	1.1	1.1	1.1	1.1		
Percent Blockage	1			3		
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	107				137	122
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	107				137	122
IC, single (s)	4.1				6.4	6.3
IC, 2 stage (s)	2.2				3.5	3.4
p0 queue free %	100				100	99
CM capacity (veh/h)	1451				835	864
Direction, Lane #	EB 1	WB 1	SB 1			
Volumes Total	30	67	12			
Volume Left	0	0	0			
Volume Right	0	0	12			
cSH	1700	1700	864			
Volumes to Capacity	0.02	0.04	0.01			
Queue Length 95th (m)	0.0	0.0	0.3			
Control Delay (s)	0.0	0.0	9.2			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.2			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			26.0%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis  
12: Regent Street & Oak Street

Existing AM Model  
10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑		↑	↑	↑
Traffic Volume (veh/h)	25	0	0	25	30	25
Future Volume (Veh/h)	25	0	0	25	30	25
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78
Hourly flow rate (vph)	32	0	0	32	38	32
Pedestrians	15			10	30	
Lane Width (m)	3.5	3.5	3.0	3.0		
Walking Speed (m/s)	1.1	1.1	1.1	1.1		
Percent Blockage	1			1	2	
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume			62		109	72
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol			62		109	72
IC, single (s)			4.1		6.4	6.2
IC, 2 stage (s)			2.2		3.5	3.3
p0 queue free %	100		100		96	97
CM capacity (veh/h)	1519		1519		852	954
Direction, Lane #	EB 1	WB 1	NB 1			
Volumes Total	32	32	70			
Volume Left	0	0	38			
Volume Right	0	0	32			
cSH	1700	1700	896			
Volumes to Capacity	0.02	0.02	0.08			
Queue Length 95th (m)	0.0	0.0	1.9			
Control Delay (s)	0.0	0.0	9.4			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.4			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			4.9			
Intersection Capacity Utilization			26.0%		ICU Level of Service	A
Analysis Period (min)			15			



HCM Unsignalized Intersection Capacity Analysis  
 13: Oak Street & Site Driveway

HCM Unsignalized Intersection Capacity Analysis  
 14: Sackville Street & Oak Street

Existing AM Model  
 10-25-2022

Existing AM Model  
 10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	45	20	0	0	5
Future Volume (Veh/h)	5	45	20	0	0	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.71	0.71	0.71	0.71	0.71	0.71
Hourly flow rate (vph)	7	63	28	0	0	7
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	53				240	53
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	53				240	53
IC, single (s)	4.1				6.4	6.2
IC, 2 stage (s)	2.2				3.5	3.3
p0 queue free %	100				100	99
CM capacity (veh/h)	1536				664	1001
Direction, Lane #	EB 1	WB 1	SB 1			
Volumes Total	70	28	7			
Volume Left	7	0	0			
Volume Right	0	0	7			
cSH	1536	1700	1001			
Volumes to Capacity	0.00	0.02	0.01			
Queue Length 95th (m)	0.1	0.0	0.2			
Control Delay (s)	0.8	0.0	8.6			
Lane LOS	A	A	A			
Approach Delay (s)	0.8	0.0	8.6			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			19.0%		ICU Level of Service	A
Analysis Period (min)			15			

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Volume (vph)	10	25	10	0	10	5
Future Volume (vph)	10	25	10	0	10	5
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	12	31	12	0	12	6
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	55	18	117			
Volume Left (vph)	12	0	12			
Volume Right (vph)	12	6	6			
Head (s)	-0.01	-0.09	0.07			
Departure Headway (s)	4.2	4.1	4.1			
Degree Utilization, x	0.06	0.02	0.13			
Capacity (veh/h)	835	841	848			
Control Delay (s)	7.5	7.2	7.8			
Approach Delay (s)	7.5	7.2	7.8			
Approach LOS	A	A	A			
Intersection Summary						
Delay			7.6			
Level of Service			A			
Intersection Capacity Utilization			32.3%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis  
 15: Sumach Street & Oak Street

HCM Unsignalized Intersection Capacity Analysis  
 16: Tubman Avenue & Oak Street

Existing AM Model  
 10-25-2022

Existing AM Model  
 10-25-2022

Movement	EBL	EBT	NB1	SB1	WBR	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Stop	Stop	Stop				Stop			Stop	
Traffic Volume (vph)	5	20	10	0	0	0	15	30	35	5	60	0
Future Volume (vph)	5	20	10	0	0	0	15	30	35	5	60	0
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Hourly flow rate (vph)	7	27	13	0	0	0	20	40	47	7	80	0
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total (vph)	47	107	87									
Volume Left (vph)	7	20	7									
Volume Right (vph)	13	47	0									
Head (s)	-0.07	-0.10	0.13									
Departure Headway (s)	4.2	4.0	4.2									
Degree Utilization, x	0.06	0.12	0.10									
Capacity (veh/h)	808	877	834									
Control Delay (s)	7.5	7.5	7.7									
Approach Delay (s)	7.5	7.5	7.7									
Approach LOS	A	A	A									
Intersection Summary												
Delay												7.6
Level of Service												A
Intersection Capacity Utilization												30.2%
ICU Level of Service												A
Analysis Period (min)												15

Movement	EBT	EBR	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free		
Traffic Volume (veh/h)	60	0	0	0	0	0	0	0	0	0		
Future Volume (Veh/h)	60	0	0	0	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free		
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75		
Hourly flow rate (vph)	80	0	0	0	0	0	0	0	0	0		
Pedestrians	40	0	0	0	0	0	0	0	0	0		
Lane Width (m)	3.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1		
Percent Blockage	4	0	0	0	0	0	0	0	0	0		
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None		
Median type	None	None	None	None	None	None	None	None	None	None		
Median storage (veh)	None	None	None	None	None	None	None	None	None	None		
Upstream signal (m)	None	None	None	None	None	None	None	None	None	None		
PK, platoon unblocked	None	None	None	None	None	None	None	None	None	None		
VC, conflicting volume	125	125	125	125	125	125	125	125	125	125		
VC1, stage 1 conf vol	125	125	125	125	125	125	125	125	125	125		
VC2, stage 2 conf vol	125	125	125	125	125	125	125	125	125	125		
IC, single (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1		
IC, 2 stage (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2		
p0 queue free %	100	100	100	100	100	100	100	100	100	100		
pM capacity (veh/h)	1474	1474	1474	1474	1474	1474	1474	1474	1474	1474		
Direction, Lane #	EB 1	EB 1	EB 1	EB 1	EB 1	EB 1	EB 1	EB 1	EB 1	EB 1		
Volume Total	80	80	80	80	80	80	80	80	80	80		
Volume Left	0	0	0	0	0	0	0	0	0	0		
Volume Right	0	0	0	0	0	0	0	0	0	0		
CSH	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700		
Volume to Capacity	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05		
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Lane LOS	A	A	A	A	A	A	A	A	A	A		
Approach Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Approach LOS	A	A	A	A	A	A	A	A	A	A		
Intersection Summary												
Average Delay												0.0
Intersection Capacity Utilization												23.9%
ICU Level of Service												A
Analysis Period (min)												15

HCM Unsignalized Intersection Capacity Analysis  
17: Oak Street & Site Driveway

HCM Unsignalized Intersection Capacity Analysis  
18: River Street & Oak Street

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4			5	0
Traffic Volume (veh/h)	5	55	0	0	5	0
Future Volume (Veh/h)	5	55	0	0	5	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	61	0	0	6	0
Pedestrians			10		10	
Lane Width (m)			0.0		3.0	
Walking Speed (m/s)			1.1		1.1	
Percent Blockage			0		1	
Right turn flare (veh)			None		None	
Median type			None		None	
Median storage (veh)						
Upstream signal (m)						10
pX platoon unblocked					93	10
VC, conflicting volume	10					
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	10				93	10
IC, single (s)	4.1				6.4	6.2
IC, 2 stage (s)						
IF (s)	2.2				3.5	3.3
p0 queue free %	100				99	100
CM capacity (veh/h)	1610				902	1069
Direction, Lane #	EB 1	SB 1				
Volumes Total	67	6				
Volume Left	6	6				
Volume Right	0	0				
cSH	1610	902				
Volumes to Capacity	0.00	0.01				
Queue Length 95th (m)	0.1	0.2				
Control Delay (s)	0.7	9.0				
Lane LOS	A	A				
Approach Delay (s)	0.7	9.0				
Approach LOS	A	A				
Intersection Summary						
Average Delay		1.4				
Intersection Capacity Utilization		17.0%			ICU Level of Service	A
Analysis Period (min)		15				

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations		4	4		4	4					4
Traffic Volume (veh/h)	40	0	20	20	0	40	0	285	20	35	485
Future Volume (Veh/h)	40	0	20	20	0	40	0	285	20	35	485
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	44	0	22	22	0	44	0	313	22	38	533
Pedestrians		75		65		35					5
Lane Width (m)		3.5		3.5		3.5		3.5		3.5	3.5
Walking Speed (m/s)		1.1		1.1		1.1		1.1		1.1	1.1
Percent Blockage		7		6		3					0
Right turn flare (veh)											
Median type								None			None
Median storage (veh)											
Upstream signal (m)								143			151
pX platoon unblocked	0.93	0.93	0.88	0.93	0.93	0.90	0.88			0.90	
VC, conflicting volume	1057	1084	643	1055	1073	394	608			400	
VC1, stage 1 conf vol											
VC2, stage 2 conf vol											
VCu, unblocked vol	789	818	529	787	806	275	489			282	
IC, single (s)	7.2	6.5	6.2	7.1	7.0	6.2	4.1			4.1	
IC, 2 stage (s)											
IF (s)	3.6	4.0	3.3	3.5	4.5	3.3	2.2			2.2	
p0 queue free %	79	100	95	90	100	93	100			97	
CM capacity (veh/h)	213	247	442	218	211	647	894			1100	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1							
Volumes Total	66	66	335	571							
Volume Left	44	22	0	38							
Volume Right	22	44	22	0							
cSH	258	391	1700	1100							
Volumes to Capacity	0.26	0.17	0.20	0.03							
Queue Length 95th (m)	7.5	4.6	0.0	0.8							
Control Delay (s)	23.7	16.1	0.0	1.0							
Lane LOS	C	C	A	A							
Approach Delay (s)	23.7	16.1	0.0	1.0							
Approach LOS	C	C	A	A							
Intersection Summary											
Average Delay			3.1								
Intersection Capacity Utilization			65.5%			ICU Level of Service				C	
Analysis Period (min)			15								

19: Parliament Street & Cole Street

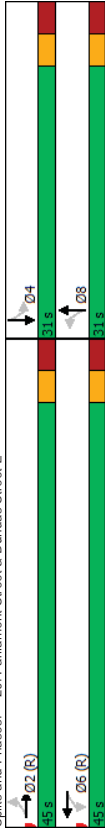
Existing AM Model  
10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	0	385	20	25	455
Future Volume (Veh/h)	0	0	385	20	25	455
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	418	22	27	495
Pedestrians	250		10			15
Lane Width (m)	0.0	3.5	3.5			3.5
Walking Speed (m/s)	1.1	1.1	1.1			1.1
Percent Blockage	0		1			1
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			80			214
pX platoon unblocked	0.95	0.95	0.95	0.95	0.95	0.95
vC1, conflicting volume	990	485				690
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vC1, unblocked vol	875	340				557
IC, single (s)	6.8	6.9				4.2
IC, 2 stage (s)						
IF (s)	3.5	3.3				2.2
p0 queue free %	100	100				97
cM capacity (veh/h)	266	617				942
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volume Total	279	161	192	330		
Volume Left	0	0	27	0		
Volume Right	0	22	0	0		
cSH	1700	1700	942	1700		
Volume to Capacity	0.16	0.09	0.03	0.19		
Queue Length 95th (m)	0.0	0.0	0.7	0.0		
Control Delay (s)	0.0	0.0	1.5	0.0		
Lane LOS			A			
Approach Delay (s)	0.0		0.6			
Approach LOS			A			
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			43.9%			
ICU Level of Service			A			
Analysis Period (min)			15			

20: Parliament Street & Dundas Street E

Existing AM Model  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	35	260	25	275	40	290	60	365
Future Volume (vph)	35	260	25	275	40	290	60	365
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2		6		8		8	4
Permitted Phases	2	2	6	6	8	8	4	4
Detector Phase								
Switch Phase								
Minimum Initial (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Minimum Split (s)	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	45.0	45.0	45.0	45.0	31.0	31.0	31.0	31.0
Total Split (%)	59.2%	59.2%	59.2%	59.2%	40.8%	40.8%	40.8%	40.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)								
Lead/Lag		5.0		5.0		5.0		5.0
Lead-Lag Optimize?								
Recall Mode								
Act Effct Green (s)	41.7	41.7	41.7	41.7	24.3	24.3	24.3	24.3
Actuated g/C Ratio	0.55	0.55	0.55	0.55	0.32	0.32	0.32	0.32
v/C Ratio	0.22	0.22	0.26	0.26	0.43	0.43	0.54	0.54
Control Delay	8.9	8.9	12.5	12.5	21.8	21.8	23.1	23.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.9	8.9	12.5	12.5	21.8	21.8	23.1	23.1
LOS	A	A	B	B	C	C	C	C
Approach Delay	8.9		12.5		21.8		23.1	
Approach LOS	A		B		C		C	
Intersection Summary								
Cycle Length: 76								
Actuated Cycle Length: 76								
Offset: 54 (7%), Referenced to phase 2EBTL and 6:WBT, Start of 1st Green								
Natural Cycle: 60								
Control Type: Actuated-Coordinated								
Maximum v/C Ratio: 0.54								
Intersection Signal Delay: 17.1								
Intersection Capacity Utilization 87.0%								
ICU Level of Service E								
Analysis Period (min) 15								



Existing AM Model  
10-25-2022

HCM Signalized Intersection Capacity Analysis  
20: Parliament Street & Dundas Street E

Existing AM Model  
10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	341	405	368	485
Lane Group Flow (vph)	0.22	0.26	0.43	0.54
v/c Ratio	8.9	12.5	21.8	23.1
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	8.9	12.5	21.8	23.1
Total Delay	11.3	20.2	21.4	28.9
Queue Length 50th (m)	18.8	29.1	32.0	41.7
Queue Length 95th (m)	85.0	103.5	45.0	56.1
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1541	1579	911	958
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.22	0.26	0.40	0.51
<b>Intersection Summary</b>				

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4TB			4TB			4TB					
Traffic Volume (vph)	35	260	25	25	275	80	40	290	15	60	355	40	
Future Volume (vph)	35	260	25	25	275	80	40	290	15	60	355	40	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Frbp. ped/bikes	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Frbp. psd/bikes	0.99	0.99	1.00	1.00	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Frt	0.99	0.99	0.97	0.97	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Flt Protected	0.99	0.99	1.00	1.00	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Satd. Flow (prot)	3155	3068	3104	3104	3068	3104	3104	3068	3104	3068	3104	3068	
Flt Permitted	0.88	0.88	0.92	0.92	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	
Satd. Flow (perm)	2798	2798	2825	2825	2798	2798	2798	2825	2798	2798	2825	2798	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	37	277	27	27	293	85	43	309	16	64	378	43	
RTOR Reduction (vph)	0	8	0	0	30	0	0	4	0	0	10	0	
Lane Group Flow (vph)	0	333	0	0	375	0	0	364	0	0	475	0	
Confl. Peds. (#/hr)	155	110	110	110	155	140	160	160	160	160	140	140	
Confl. Bikes (#/hr)	10	10	10	10	25	25	25	25	25	25	25	25	
Heavy Vehicles (%)	6%	10%	4%	22%	5%	13%	18%	11%	13%	2%	5%	5%	
Parking (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0	
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA	
Protected Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Permitted Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Actuated Green, G (s)	40.7	40.7	40.7	41.7	41.7	41.7	23.3	23.3	23.3	23.3	23.3	23.3	
Effective Green, g (s)	41.7	41.7	41.7	42.7	42.7	42.7	24.3	24.3	24.3	24.3	24.3	24.3	
Actuated g/C Ratio	0.55	0.55	0.55	0.55	0.55	0.55	0.32	0.32	0.32	0.32	0.32	0.32	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1535	1535	1550	1550	1535	1535	848	848	848	848	886	886	
v/s Ratio Prot	0.12	0.12	0.13	0.13	0.13	0.14	0.14	0.14	0.14	0.17	0.17	0.17	
v/c Ratio Perm	0.22	0.22	0.24	0.24	0.24	0.43	0.43	0.43	0.43	0.54	0.54	0.54	
Uniform Delay, d1	8.8	8.8	8.9	8.9	8.9	20.4	20.4	20.4	20.4	21.2	21.2	21.2	
Progression Factor	1.00	1.00	1.59	1.59	1.59	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.3	0.3	0.4	0.4	0.4	0.4	0.4	0.4	0.4	0.6	0.6	0.6	
Delay (s)	9.1	9.1	14.5	14.5	14.5	20.7	20.7	20.7	20.7	21.8	21.8	21.8	
Level of Service	A	A	B	B	B	C	C	C	C	C	C	C	
Approach Delay (s)	9.1	9.1	14.5	14.5	14.5	20.7	20.7	20.7	20.7	21.8	21.8	21.8	
Approach LOS	A	A	B	B	B	C	C	C	C	C	C	C	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	17.0											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.35												
Actuated Cycle Length (s)	76.0											Sum of lost time (s)	10.0
Intersection Capacity Utilization	87.0%											ICU Level of Service	E
Analysis Period (min)	15												
c Critical Lane Group													

21: Regent Street & Dundas Street E

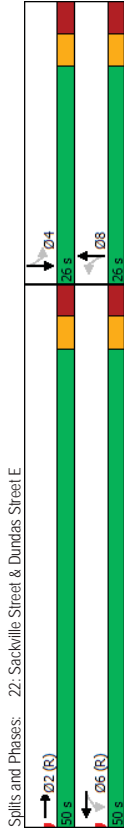
Existing AM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4T		4T				4T			4T	
Traffic Volume (veh/h)	10	325	10	10	355	35	15	10	5	20	10	20
Future Volume (Veh/h)	10	325	10	10	355	35	15	10	5	20	10	20
Sign Control	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	353	11	11	386	38	16	11	5	22	11	22
Pedestrians	20			5			70				80	
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	2			0			6				7	
Right turn flare (veh)												
Median type	None	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)	127			124								
pX platoon unblocked												
vC1 conflicting volume	504			434			713		976		257	721
vC1, stage 1 conf vol												963
vC2, stage 2 conf vol												312
vC1, unblocked vol	504			434			713		976		257	721
IC, single (s)	4.1			4.1			7.6		6.7		6.9	6.5
IC, 2 stage (s)	2.2			2.2			3.6		4.1		3.3	3.5
p0 queue free %	99			99			93		95		99	91
CM capacity (veh/h)	995			1066			235		200		699	242
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1					
Volumes Total	188	188	204	231	32	55						
Volume Left	11	0	11	0	16	22						
Volume Right	0	11	0	38	5	22						
cSH	995	1700	1066	1700	246	310						
Volumes to Capacity	0.01	0.11	0.01	0.14	0.13	0.18						
Queue Length 95th (m)	0.3	0.0	0.2	0.0	3.4	4.8						
Control Delay (s)	0.6	0.0	0.5	0.0	21.8	19.1						
Lane LOS	A	A	A	C	C	C						
Approach Delay (s)	0.3		0.3		21.8	19.1						
Approach LOS	C		C		C	C						
Intersection Summary												
Average Delay	2.2											
Intersection Capacity Utilization	33.9%											
Analysis Period (min)	15											
	ICU Level of Service A											

22: Sackville Street & Dundas Street E

Existing AM Model  
10-25-2022

Movement	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4T	4T	4T	4T	4T	4T	4T
Traffic Volume (vph)	360	10	330	45	0	35	50
Future Volume (vph)	360	10	330	45	0	35	50
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2		6		8		4
Permitted Phases	2	6	6	6	8	8	4
Detector Phase							
Switch Phase							
Minimum Initial (s)	17.0	17.0	17.0	7.0	7.0	7.0	7.0
Minimum Split (s)	24.0	24.0	24.0	26.0	26.0	26.0	26.0
Total Split (s)	50.0	50.0	50.0	26.0	26.0	26.0	26.0
Total Split (%)	65.8%	65.8%	65.8%	34.2%	34.2%	34.2%	34.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag							
Lead-Lag Optimize?							
Recall Mode	C-Min	C-Min	C-Min	None	None	None	None
Ad Effct Green (s)	47.6	47.6	47.6	18.4	18.4	18.4	18.4
Actuated g/C Ratio	0.63	0.63	0.63	0.24	0.24	0.24	0.24
v/C Ratio	0.20	0.33	0.22	0.55	12.9	20.2	20.2
Control Delay	6.8	5.5	5.5	12.9	12.9	20.2	20.2
Queue Delay	0.0	0.3	0.3	0.0	0.0	0.0	0.0
Total Delay	6.8	5.8	5.8	12.9	12.9	20.2	20.2
LOS	A	A	A	B	B	C	C
Approach Delay	6.8	5.8	5.8	12.9	12.9	20.2	20.2
Approach LOS	A	A	A	B	B	C	C
Intersection Summary							
Cycle Length: 76							
Actuated Cycle Length: 76							
Offset: T1 (14%), Referenced to phase 2EBT and 6,WBTL Start of 1st Green							
Natural Cycle: 50							
Control Type: Actuated-Coordinated							
Maximum v/C Ratio: 0.33							
Intersection Signal Delay: 8.6							
Intersection Capacity Utilization 49.5%							
Analysis Period (min) 15							
	ICU Level of Service A						



Queues  
22: Sackville Street & Dundas Street E

Existing AM Model  
10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	405	358	73	122
Lane Group Flow (vph)	0.20	0.33	0.22	0.32
v/c Ratio	6.8	5.5	12.9	20.2
Control Delay	0.0	0.3	0.0	0.0
Queue Delay	6.8	5.8	12.9	20.2
Total Delay	8.7	23.8	3.2	11.0
Queue Length 50th (m)	19.8	8.8	12.5	23.9
Queue Length 95th (m)	99.8	79.6	36.2	126.8
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2015	1073	376	426
Station Cap Reductn	0	272	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.20	0.45	0.19	0.29
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
22: Sackville Street & Dundas Street E

Existing AM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4						4				4	
Traffic Volume (vph)	0	360	25	10	330	0	45	0	25	35	50	30	
Future Volume (vph)	0	360	25	10	330	0	45	0	25	35	50	30	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Lane Util. Factor	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frbp. ped/bikes	0.99	0.99	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.98	0.98	0.98	
Frbp. psd/bikes	1.00	1.00	1.00	1.00	0.96	0.96	0.96	0.96	0.96	0.97	0.97	0.97	
Frt	0.99	0.99	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.96	0.96	0.96	
Flt Protected	1.00	1.00	1.00	1.00	0.97	0.97	0.97	0.97	0.97	0.99	0.99	0.99	
Satd. Flow (prot)	3220	3220	1735	1735	1545	1545	1545	1545	1545	1630	1630	1630	
Flt Permitted	1.00	0.99	0.99	0.99	0.78	0.78	0.78	0.78	0.78	0.89	0.89	0.89	
Satd. Flow (perm)	3220	1714	1714	1250	1250	1250	1250	1250	1481	1481	1481	1481	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Adj. Flow (vph)	0	379	26	11	347	0	47	0	26	37	53	32	
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Group Flow (vph)	0	405	0	0	358	0	40	0	40	0	105	0	
Confl. Peds. (#/hr)	65	85	85	65	65	65	65	65	60	60	60	65	
Confl. Bikes (#/hr)	10	10	10	25	25	25	25	25	25	25	25	25	
Heavy Vehicles (%)	0%	8%	13%	0%	8%	0%	0%	0%	8%	6%	2%	4%	
Turn Type	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	
Protected Phases	2	6	6	6	6	6	8	8	8	8	8	4	
Permitted Phases													
Actuated Green, G (s)	46.6	46.6	46.6	46.6	46.6	46.6	17.4	17.4	17.4	17.4	17.4	17.4	
Effective Green, g (s)	47.6	47.6	47.6	47.6	47.6	47.6	18.4	18.4	18.4	18.4	18.4	18.4	
Actuated q/C Ratio	0.63	0.63	0.63	0.63	0.63	0.63	0.24	0.24	0.24	0.24	0.24	0.24	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	2016	1073	1073	302	302	302	358	358	358	358	358	358	
v/s Ratio Prot	0.13	0.21	0.21	0.03	0.03	0.03	0.07	0.07	0.07	0.07	0.07	0.07	
v/s Ratio Perm	0.20	0.33	0.33	0.13	0.13	0.13	0.29	0.29	0.29	0.29	0.29	0.29	
Uniform Delay, d1	6.1	6.7	6.7	22.6	22.6	22.6	23.5	23.5	23.5	23.5	23.5	23.5	
Progression Factor	0.96	0.60	0.60	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.2	0.2	0.2	0.8	0.8	0.8	0.5	0.5	0.5	0.5	0.5	0.5	
Delay (s)	6.0	6.0	6.0	4.9	4.9	4.9	23.9	23.9	23.9	23.9	23.9	23.9	
Level of Service	A	A	A	C	C	C	C	C	C	C	C	C	
Approach Delay (s)	6.0	4.9	4.9	22.8	22.8	22.8	23.9	23.9	23.9	23.9	23.9	23.9	
Approach LOS	A	A	A	A	A	A	C	C	C	C	C	C	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	9.2											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.32												
Actuated Cycle Length (s)	76.0											Sum of lost time (s)	10.0
Intersection Capacity Utilization	49.5%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

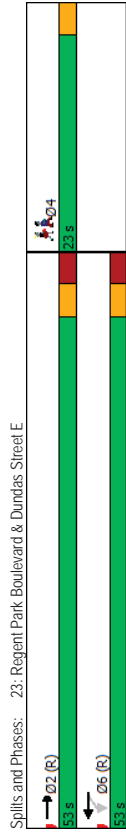
Timings  
23: Regent Park Boulevard & Dundas Street E

Queues  
23: Regent Park Boulevard & Dundas Street E

Existing AM Model  
10-25-2022

Existing AM Model  
10-25-2022

Lane Group	EBT	WBL	WBT	Ø4
Lane Configurations	4	2	3	4
Traffic Volume (vph)	405	20	340	
Future Volume (vph)	405	20	340	
Turn Type	NA	Perm	NA	
Protected Phases	2	6	6	4
Permitted Phases	2	6	6	
Detector Phase				
Switch Phase				
Minimum Initial (s)	18.0	18.0	18.0	20.0
Minimum Split (s)	23.9	23.9	23.9	23.0
Total Split (s)	53.0	53.0	53.0	23.0
Total Split (%)	69.7%	69.7%	69.7%	30%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.9		4.9	
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	C-Min	C-Min	C-Min	None
Act Effct Green (s)	64.8		64.8	
Actuated g/C Ratio	0.85		0.85	
v/c Ratio	0.30		0.26	
Control Delay	3.5		4.3	
Queue Delay	0.0		0.0	
Total Delay	3.5		4.3	
LOS	A		A	
Approach Delay	3.5		4.3	
Approach LOS	A		A	
<b>Intersection Summary</b>				
Cycle Length: 76				
Actuated Cycle Length: 76				
Offset: 8 (1%) Referenced to phase 2:EBT and 6:WBLT, Start of 1st Green				
Natural Cycle: 50				
Control Type: Actuated-Coordinated				
Maximum v/c Ratio: 0.30				
Intersection Signal Delay: 3.9				
Intersection Capacity Utilization 53.6%				
Analysis Period (min) 15				



Lane Group	EBT	WBT
Lane Group Flow (vph)	438	375
v/c Ratio	0.30	0.26
Control Delay	3.5	4.3
Queue Delay	0.0	0.0
Total Delay	3.5	4.3
Queue Length 50th (m)	0.0	0.0
Queue Length 95th (m)	22.1	36.3
Internal Link Dist (m)	79.6	86.0
Turn Bay Length (m)		
Base Capacity (vph)	1466	1429
Starvation Cap Reductn	54	121
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.31	0.29
<b>Intersection Summary</b>		



23: Regent Park Boulevard & Dundas Street E

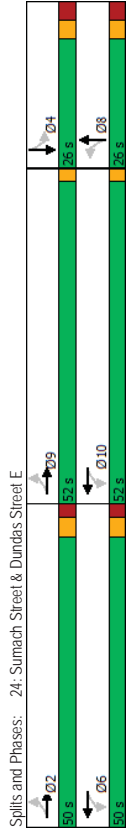
Existing AM Model  
10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4	4	4	4	4	4
Traffic Volume (vph)	405	15	20	340	0	0
Future Volume (vph)	405	15	20	340	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.0	3.0	3.5	3.0	3.0
Total Lost time (s)	4.9	4.9	4.9	4.9	4.9	4.9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1718	1718	1720	1720	1720	1720
Flt Permitted	1.00	0.97	0.97	0.97	0.97	0.97
Satd. Flow (perm)	1718	1718	1673	1673	1673	1673
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	422	16	21	354	0	0
RTOR Reduction (vph)	1	0	0	0	0	0
Lane Group Flow (vph)	437	0	0	375	0	0
Conf. Peds. (#/hr)	125	125	125	40	60	60
Heavy Vehicles (%)	8%	0%	9%	8%	0%	0%
Turn Types	NA	Perm	NA	NA	NA	NA
Protected Phases	2	6	6	6	6	6
Permitted Phases	6	6	6	6	6	6
Actuated Green, G (s)	59.1	59.1	59.1	59.1	59.1	59.1
Effective Green, g (s)	60.1	60.1	60.1	60.1	60.1	60.1
Actuated g/C Ratio	0.79	0.79	0.79	0.79	0.79	0.79
Clearance Time (s)	5.9	5.9	5.9	5.9	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap. (vph)	1358	1358	1322	1322	1322	1322
v/s Ratio Prot	0.25	0.25	0.25	0.25	0.25	0.25
v/s Ratio Perm	0.32	0.32	0.28	0.28	0.28	0.28
Uniform Delay, d1	2.2	2.2	2.1	2.1	2.1	2.1
Progression Factor	0.74	0.74	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.6	0.6	0.5	0.5	0.5	0.5
Delay (s)	2.3	2.3	2.7	2.7	2.7	2.7
Level of Service	A	A	A	A	A	A
Approach Delay (s)	2.3	2.3	2.7	2.7	2.7	2.7
Approach LOS	A	A	A	A	A	A
Intersection Summary						
HCM 2000 Control Delay	2.5 HCM 2000 Level of Service A					
HCM 2000 Volume to Capacity ratio	0.28					
Actuated Cycle Length (s)	76.0 Sum of lost time (s)					
Intersection Capacity Utilization	53.6% ICU Level of Service A					
Analysis Period (min)	15					
c. Critical Lane Group						

24: Sumach Street & Dundas Street E

Existing AM Model  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4	4	4	4	4	4	4	4
Traffic Volume (vph)	15	370	25	325	15	30	15	30
Future Volume (vph)	15	370	25	325	15	30	15	30
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2,9	6,10	8	8	8	8	4	4
Detector Phase	2	2	6	6	6	6	4	4
Switch Phase	2	2	6	6	6	6	4	4
Minimum Initial (s)	20.0	20.0	20.0	20.0	20.0	20.0	16.0	16.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	21.0	21.0
Total Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	50.0	52.0
Total Split (%)	20.3%	20.3%	20.3%	20.3%	20.3%	20.3%	39%	41%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	2.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode	67.7	67.7	67.7	67.7	67.7	67.7	25.0	25.0
Act Effct Green (s)	0.76	0.76	0.76	0.76	0.76	0.76	0.28	0.28
Actuated g/C Ratio	0.35	0.35	0.34	0.34	0.34	0.34	0.18	0.18
v/C Ratio	6.4	6.4	6.3	6.3	6.3	6.3	29.9	29.9
Control Delay	0.9	0.9	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	7.4	7.4	6.3	6.3	6.3	6.3	29.9	29.9
Total Delay	A	A	A	A	A	A	C	C
LOS	A	A	A	A	A	A	C	C
Approach Delay	7.4	7.4	6.3	6.3	6.3	6.3	29.9	29.9
Approach LOS	A	A	A	A	A	A	C	C
Intersection Summary								
Cycle Length: 128								
Actuated Cycle Length: 88.6								
Natural Cycle: 100								
Control Type: Actuated-Uncoordinated								
Maximum v/C Ratio: 0.35								
Intersection Signal Delay: 10.4								
Intersection Capacity Utilization 53.7%								
Analysis Period (min) 15								



Queues  
24: Sumach Street & Dundas Street E

Existing AM Model  
10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	440	413	109	71
Lane Group Flow (vph)	0.35	0.34	0.27	0.18
v/c Ratio	6.4	6.3	26.0	29.9
Control Delay	0.9	0.0	0.0	0.0
Queue Delay	7.4	6.3	26.0	29.9
Total Delay	30.0	27.4	11.1	9.0
Queue Length 50th (m)	44.4	41.4	27.9	22.3
Queue Length 95th (m)	86.0	75.5	76.2	121.5
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1263	1218	400	393
Station Cap Reductn	544	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.61	0.34	0.27	0.18
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
24: Sumach Street & Dundas Street E

Existing AM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4		4			4			4			
Traffic Volume (vph)	15	370	20	25	325	30	15	30	55	15	30	20	
Future Volume (vph)	15	370	20	25	325	30	15	30	55	15	30	20	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	4.0			4.0			4.0			4.0			
Lane Util. Factor	1.00	1.00		1.00			1.00			1.00			
Frbp. ped/bikes	0.98	0.98		0.98			0.93			0.91			
Frbp. psd/bikes	1.00	0.99		0.99			0.97			0.96			
Frt	1.00	0.99		0.99			0.93			0.99			
Flt Protected	1.00	1.00		1.00			0.99			0.99			
Sat'd. Flow (prot)	1691	1669		1669			1432			1523			
Flt Permitted	0.98	0.96		0.96			0.95			0.92			
Sat'd. Flow (perm)	1665	1608		1608			1370			1419			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	16	402	22	27	353	33	16	33	60	16	33	22	
RTOR Reduction (vph)	0	1	0	0	2	0	0	34	0	0	13	0	
Lane Group Flow (vph)	0	439	0	0	411	0	0	75	0	0	58	0	
Confl. Peds. (#/hr)	70	135	135	70	120	70	120	50	50	120	50	120	
Confl. Bikes (#/hr)		5			30								
Heavy Vehicles (%)	13%	7%	14%	9%	8%	3%	11%	3%	10%	0%	3%	10%	
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	2.9			6.10			8			4			
Permitted Phases		2.9		6.10			8			4			
Actuated Green, G (s)		68.2		68.2			13.9			13.9			
Effective Green, g (s)		69.2		69.2			14.9			14.9			
Actuated q/C Ratio		0.77		0.77			0.17			0.17			
Clearance Time (s)							6.0			6.0			
Vehicle Extension (s)							3.0			3.0			
Lane Grp Cap (vph)		1278		1235			226			234			
v/s Ratio Prot							60.05			0.04			
v/s Ratio Perm		0.26		0.33			0.33			0.25			
v/c Ratio		0.34		0.33			0.33			0.25			
Uniform Delay, d1		3.3		3.3			33.2			32.7			
Progression Factor		1.00		1.00			1.00			1.00			
Incremental Delay, d2		0.2		0.2			0.9			0.6			
Delay (s)		3.5		3.4			34.1			33.3			
Level of Service		A		A			C			C			
Approach Delay (s)		3.5		3.4			34.1			33.3			
Approach LOS		A		A			C			C			
<b>Intersection Summary</b>													
HCM 2000 Control Delay	8.7											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.35												
Actuated Cycle Length (s)	90.1											Sum of lost time (s)	10.0
Intersection Capacity Utilization	53.7%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

25: Tubman Avenue & Dundas Street E

Existing AM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	425	15	5	380	0	0	0	0	0	0	0
Traffic Volume (veh/h)	0	425	15	5	380	0	0	0	0	0	0	0
Future Volume (Veh/h)	0	425	15	5	380	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	472	17	6	422	0	0	0	0	0	0	0
Pedestrians	5			5			115			45		
Lane Width (m)	3.5			3.5			0.0			3.5		
Walking Speed (m/s)	1.1			1.1			1.1			1.1		
Percent Blockage	0			0			0			4		
Right turn flare (veh)	None			None								
Median type	None			None								
Median storage (veh)												
Upstream signal (m)	99			98								
pX platoon unblocked	0.89			0.94			0.93			0.93		0.93
VC, conflicting volume	467			604			1034			600		1083
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	344			543			842			885		539
IC, single (s)	4.1			4.1			7.1			6.2		7.1
IC, 2 stage (s)	2.2			2.2			3.5			4.0		3.3
p0 queue free %	100			99			100			100		100
CM capacity (veh/h)	1052			970			255			509		275
Direction, Lane #	EB 1	WB 1	SB 1									
Volumes Total	489	428	0									
Volume Left	0	6	0									
Volume Right	17	0	0									
cSH	1052	970	1700									
Volumes to Capacity	0.00	0.01	0.00									
Queue Length 95th (m)	0.0	0.1	0.0									
Control Delay (s)	0.0	0.2	0.0									
Lane LOS	A	A	A									
Approach Delay (s)	0.0	0.2	0.0									
Approach LOS	A	A	A									
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	35.5%											
ICU Level of Service	A											
Analysis Period (min)	15											

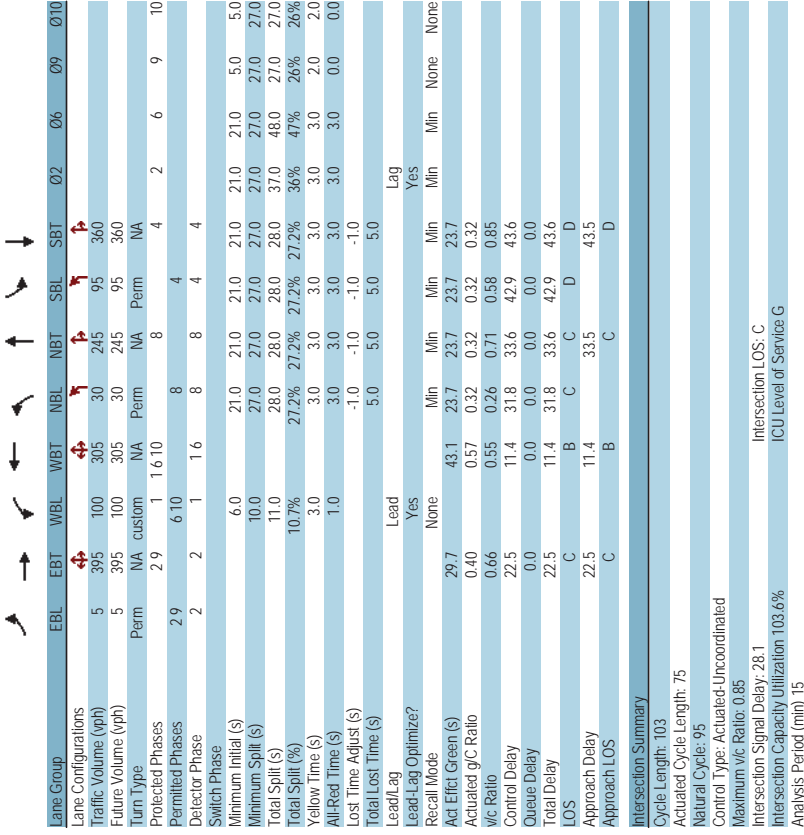
Regent Park Phases 4 & 5  
BA Group - NHY

26: River Street & Dundas Street E

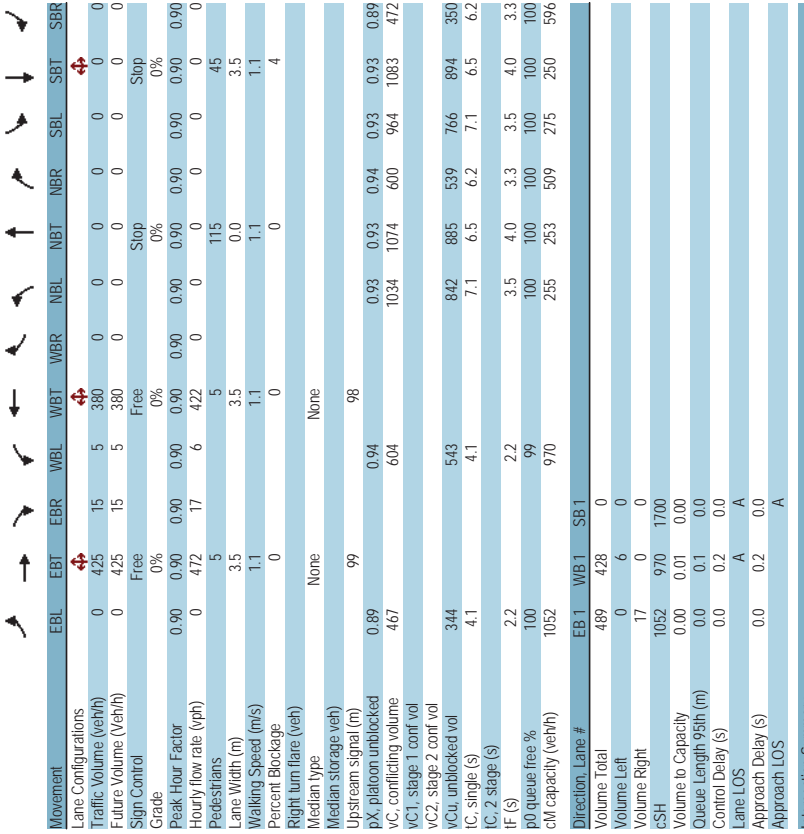
Existing AM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	5	395	100	305	30	245	95	360				
Traffic Volume (vph)	5	395	100	305	30	245	95	360				
Future Volume (vph)	5	395	100	305	30	245	95	360				
Sign Control	Perm	NA	custom	NA	Perm	NA	Perm	NA				
Grade	2.9	1	1.6	10	8	4	4	4				
Peak Hour Factor	2	2	2	2	2	2	2	2				
Hourly flow rate (vph)	2	2	2	2	2	2	2	2				
Pedestrians	2			2			8			4		
Lane Width (m)	6.0			6.0			21.0			21.0		5.0
Walking Speed (m/s)	11.0			11.0			27.0			27.0		27.0
Percent Blockage	10.7%			10.7%			27.2%			27.2%		26%
Right turn flare (veh)	3.0			3.0			3.0			3.0		2.0
Median type	1.0			1.0			3.0			3.0		0.0
Median storage (veh)	-1.0			-1.0			-1.0			-1.0		0.0
Upstream signal (m)	5.0			5.0			5.0			5.0		5.0
pX platoon unblocked	29.7			43.1			23.7			23.7		23.7
VC, conflicting volume	0.40			0.57			0.32			0.32		0.32
VC1, stage 1 conf vol	0.66			0.55			0.26			0.71		0.58
VC2, stage 2 conf vol	22.5			11.4			31.8			33.6		43.6
IC, single (s)	0.0			0.0			0.0			0.0		0.0
IC, 2 stage (s)	22.5			11.4			31.8			33.6		43.6
p0 queue free %	C			B			C			C		D
CM capacity (veh/h)	22.5			11.4			33.5			43.5		43.5
Direction, Lane #	EB 1	WB 1	SB 1									
Volumes Total	489	428	0									
Volume Left	0	6	0									
Volume Right	17	0	0									
cSH	1052	970	1700									
Volumes to Capacity	0.00	0.01	0.00									
Queue Length 95th (m)	0.0	0.1	0.0									
Control Delay (s)	0.0	0.2	0.0									
Lane LOS	A	A	A									
Approach Delay (s)	0.0	0.2	0.0									
Approach LOS	A	A	A									
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	35.5%											
ICU Level of Service	A											
Analysis Period (min)	15											

Regent Park Phases 4 & 5  
BA Group - NHY



Intersection Summary  
Cycle Length: 103  
Actuated Cycle Length: 75  
Natural Cycle: 95  
Control Type: Actuated-Uncoordinated  
Maximum v/c Ratio: 0.85  
Intersection Signal Delay: 28.1  
Intersection Capacity Utilization 103.6%  
Analysis Period (min) 15



Intersection Summary  
Average Delay  
Intersection Capacity Utilization  
ICU Level of Service  
Analysis Period (min)

Queues  
26: River Street & Dundas Street E

Existing AM Model  
10-25-2022

	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group	452	478	32	373	101	436
Lane Group Flow (vph)	0.66	0.55	0.26	0.71	0.58	0.85
v/c Ratio	22.5	11.4	31.8	33.6	42.9	43.6
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	22.5	11.4	31.8	33.6	42.9	43.6
Total Delay	48.7	33.8	2.9	37.8	10.3	48.7
Queue Length 50th (m)	72.1	50.0	15.2	#130.4	#49.2	#164.7
Queue Length 95th (m)	73.4	80.5		131.0		118.9
Internal Link Dist (m)			20.0		25.0	
Turn Bay Length (m)						
Base Capacity (vph)	843	997	124	523	173	514
Stavation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.54	0.48	0.26	0.71	0.58	0.85
<b>Intersection Summary</b>						
#	95th percentile volume exceeds capacity, queue may be longer.					
	Queue shown is maximum after two cycles.					

HCM Signalized Intersection Capacity Analysis  
26: River Street & Dundas Street E

Existing AM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		+		+								
Traffic Volume (vph)	5	395	25	100	305	45	30	245	105	95	360	50
Future Volume (vph)	5	395	25	100	305	45	30	245	105	95	360	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0			3.0			5.0		5.0		5.0	
Lane Util. Factor	1.00			1.00			1.00		1.00		1.00	
Frbp. ped/bikes	0.99			0.99			0.99		0.97		0.98	
Frbp. psd/bikes	1.00			0.99			0.95		1.00		0.96	
Frt	1.00			0.99			1.00		0.95		1.00	
Frt Protected	1.00			0.99			0.95		1.00		0.95	
Satd. Flow (prot)	1725			1702			1474		1633		1543	
Frt Permitted	1.00			0.84			0.25		1.00		0.34	
Satd. Flow (perm)	1717			1448			394		1633		551	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	5	420	27	106	324	48	32	261	112	101	383	53
RTOR Reduction (vph)	0	3	0	0	6	0	0	13	0	0	4	0
Lane Group Flow (vph)	0	449	0	0	472	0	32	360	0	101	432	0
Conf. Peds. (#/hr)	65		90	90		65	65		45		45	
Conf. Bikes (#/hr)			5			30						15
Heavy Vehicles (%)	0%	7%	4%	3%	7%	0%	9%	9%	1%	5%	12%	7%
Turn Type	Permitted	NA	NA	custom	NA	NA	Permitted	NA	Permitted	NA	Permitted	NA
Protected Phases	2.9			1	1.6	1.0			8			4
Permitted Phases	2.9			6	10				8			4
Actuated Green, G (s)	34.5			45.7			22.6		22.6		22.6	22.6
Effective Green, g (s)	35.5			41.7			23.6		23.6		23.6	23.6
Actuated g/C Ratio	0.47			0.55			0.31		0.31		0.31	0.31
Clearance Time (s)							6.0		6.0		6.0	6.0
Vehicle Extension (s)							3.0		3.0		3.0	3.0
Lane Grp Cap (vph)	798			818			505		170		502	
v/s Ratio Prot				c0.06			0.22				c0.27	
v/s Ratio Perm	c0.26			c0.25			0.08		0.18		0.18	
v/c Ratio	0.56			0.58			0.26		0.71		0.59	0.86
Uniform Delay, d1	14.8			11.5			19.8		22.3		22.3	24.8
Progression Factor	1.00			1.00			1.00		1.00		1.00	1.00
Incremental Delay, d2	0.9			1.0			1.2		4.7		5.5	14.0
Delay (s)	15.7			12.5			21.0		28.1		27.8	38.8
Level of Service	B			B			C		C		C	D
Approach Delay (s)	15.7			12.5			27.5		36.7		36.7	
Approach LOS	B			B			C		C		D	D
<b>Intersection Summary</b>												
HCM 2000 Control Delay	23.5 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.72											
Actuated Cycle Length (s)	76.3 Sum of lost time (s) 14.0											
Intersection Capacity Utilization	103.6% ICU Level of Service G											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 27: Dreamer's Way & Gerrard Street E

HCM Unsignalized Intersection Capacity Analysis  
 29: Sumach Street & Site Driveway

Existing AM Model  
 10-25-2022

Existing AM Model  
 10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		
Traffic Volume (veh/h)	345	0	0	760	0	0
Future Volume (Veh/h)	345	0	0	760	0	0
Sign Control	Free	Free	Stop	Free	Stop	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	383	0	0	844	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None			None		None
Median type						
Median storage (veh)						
Upstream signal (m)	88			164		
pX platoon unblocked	0.98			0.90		0.98
VC, conflicting volume	383			805		192
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	330			467		134
IC, single (s)	4.1			6.8		6.9
IC, 2 stage (s)						
p0 queue free %	2.2			3.5		3.3
IF (s)	100			100		100
CM capacity (veh/h)	1216			478		878
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volumes Total	255	128	281	563		
Volume Left	0	0	0	0		
Volume Right	0	0	0	0		
cSH	1700	1700	1216	1700		
Volumes to Capacity	0.15	0.08	0.00	0.33		
Queue Length 95th (m)	0.0	0.0	0.0	0.0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0		0.0			
Approach LOS			A			
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	24.3%					
ICU Level of Service	A					
Analysis Period (min)	15					

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑				↑	↑
Traffic Volume (veh/h)	0	0	35	0	5	65
Future Volume (Veh/h)	0	0	35	0	5	65
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	39	0	6	72
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None		None		None	None
Median type						
Median storage (veh)						
Upstream signal (m)						57
pX platoon unblocked						
VC, conflicting volume	123		39			39
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	123		39			39
IC, single (s)	6.4		6.2			4.1
IC, 2 stage (s)						
p0 queue free %	3.5		3.3			2.2
IF (s)	100		100			100
CM capacity (veh/h)	874		1038			1584
Direction, Lane #	WB 1	NB 1	SB 1			
Volumes Total	0	39	78			
Volume Left	0	0	6			
Volume Right	0	0	0			
cSH	1700	1700	1584			
Volumes to Capacity	0.00	0.02	0.00			
Queue Length 95th (m)	0.0	0.0	0.1			
Control Delay (s)	0.0	0.0	0.6			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	0.6			
Approach LOS			A			
Intersection Summary						
Average Delay	0.4					
Intersection Capacity Utilization	10.9%					
ICU Level of Service	A					
Analysis Period (min)	15					

1: Parliament Street & Gerrard Street E (North Section)

Existing PM Model  
10-25-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				4A	4A	
Traffic Volume (veh/h)	0	0	10	495	565	75
Future Volume (Veh/h)	0	0	10	495	565	75
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	0	0	11	556	635	84
Pedestrians	150			5	10	
Lane Width (m)	0.0			3.5	3.5	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	0			0	1	
Right turn flare (veh)				None	None	
Median type				None	None	
Median storage (veh)				39		
Upstream signal (m)						
pX platoon unblocked	0.93					
VC conflicting volume	1137	514	869			
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU unblocked vol	1005	514	869			
IC, single (s)	6.8	6.9	4.3			
IC, 2 stage (s)						
p0 queue free %	3.5	3.3	2.3			
IF (s)	100	100	98			
CM capacity (veh/h)	220	508	717			
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	1%	371	423	296		
Volume Left	11	0	0	0		
Volume Right	0	0	0	84		
cSH	717	1700	1700	1700		
Volumes to Capacity	0.02	0.22	0.25	0.17		
Queue Length 95th (m)	0.4	0.0	0.0	0.0		
Control Delay (s)	0.7	0.0	0.0	0.0		
Lane LOS	A					
Approach Delay (s)	0.3		0.0			
Approach LOS						
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			32.3%		ICU Level of Service	A
Analysis Period (min)			15			

2: Parliament Street & Gerrard Street E

Existing PM Model  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		4TB		4TB		4TB		4TB
Traffic Volume (vph)	30	535	40	240	65	345	215	280
Future Volume (vph)	30	535	40	240	65	345	215	280
Turn Type	Perm	NA	Perm	NA	Perm	NA	custom	NA
Protected Phases	4 11		8 12		2 9		1 16	2 4
Permitted Phases	4	4	8	8	2	2	1	1 6
Switch Phase								
Minimum Initial (s)							6.0	22.0
Minimum Split (s)							12.5	33.2
Total Split (s)							13.0	43.0
Total Split (%)							12.3%	41%
Yellow Time (s)							3.3	3.0
All-Red Time (s)							3.2	7.9
Lost Time Adjust (s)								2.8
Total Lost Time (s)								7.9
Lead/Lag							Lead	Lag
Recall Mode							Yes	Yes
Ad Effct Green (s)	31.4		31.4		27.0		51.1	
Actuated g/C Ratio	0.34		0.34		0.29		0.55	
v/C Ratio	0.61		0.45		0.70		0.49	
Control Delay	28.3		21.3		35.0		13.3	
Queue Delay	0.0		0.0		0.0		0.0	
Total Delay	28.3		21.3		35.0		13.3	
LOS	C		C		C		B	
Approach Delay	28.3		21.3		35.0		13.3	
Approach LOS	C		C		C		B	
Intersection Summary								
Cycle Length: 106								
Actuated Cycle Length: 93								
Natural Cycle: 95								
Control Type: Actuated-Uncoordinated								
Maximum v/C Ratio: 0.70								
Intersection Signal Delay: 24.4								
Intersection Capacity Utilization 98.0%								
Analysis Period (min) 15								



Timings  
2: Parliament Street & Gerrard Street E

Queues  
2: Parliament Street & Gerrard Street E

Existing PM Model  
10-25-2022

Existing PM Model  
10-25-2022

Lane Group	Ø9	Ø10	Ø11	Ø12
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Turn Type	9	10	11	12
Protected Phases				
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0	5.0	5.0	5.0
Minimum Split (s)	8.0	8.0	8.0	8.0
Total Split (s)	8.0	8.0	8.0	8.0
Total Split (%)	8%	8%	8%	8%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	None	None
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Intersection Summary				

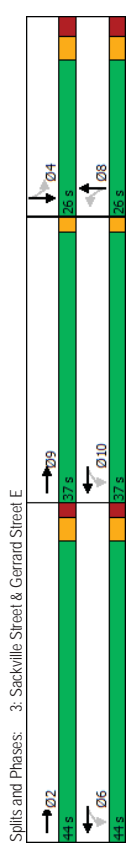
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	645	422	505	583
v/c Ratio	0.61	0.45	0.70	0.49
Control Delay	28.3	21.3	35.0	13.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	28.3	21.3	35.0	13.3
Queue Length 50th (m)	49.4	24.7	43.7	29.6
Queue Length 95th (m)	73.6	41.6	63.4	42.1
Internal Link Dist (m)	33.0	65.6	119.2	15.0
Turn Bay Length (m)				
Base Capacity (vph)	1096	957	895	1236
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.59	0.44	0.56	0.47
Intersection Summary				

2. Parliament Street & Gerrard Street E  
 HCM Signalized Intersection Capacity Analysis  
 Existing PM Model  
 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB			4TB		4TB		4TB	
Traffic Volume (vph)	30	535	60	40	240	130	65	345	80	215	280	70
Future Volume (vph)	30	535	60	40	240	130	65	345	80	215	280	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Sat'd Flow (prot)	3414	3414	3414	3414	3414	3414	3414	3414	3414	3414	3414	3414
Flt Permitted	0.92	0.84	0.84	0.77	0.77	0.77	0.62	0.62	0.62	0.62	0.62	0.62
Sat'd Flow (perm)	3133	2627	2627	2459	2459	2459	1992	1992	1992	1992	1992	1992
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	31	552	62	41	247	134	67	356	82	222	289	72
RTOR Reduction (vph)	0	7	0	0	49	0	0	12	0	0	12	0
Lane Group Flow (vph)	0	638	0	0	373	0	0	493	0	0	571	0
Conf. Peds. (#/hr)	130	120	120	130	125	130	125	235	235	235	125	125
Conf. Bikes (#/hr)	25	25	25	25	25	25	25	15	15	15	15	15
Heavy Vehicles (%)	10%	0%	5%	0%	0%	10%	4%	2%	2%	6%	4%	0%
Turn Type	Perim	MA	Perm	MA	Perm	NA	Perm	NA	Perm	NA	Perm	MA
Protected Phases	4 11	8 12	8 12	2 9	2 9	2 9	1 6 10	1 6 10	1 6 10	1 6 10	1 6 10	1 6 10
Permitted Phases	4 11	8 12	8 12	2 9	2 9	2 9	6 10	6 10	6 10	6 10	6 10	6 10
Actuated Green, G (s)	35.1	35.1	35.1	41.9	41.9	41.9	55.0	55.0	55.0	55.0	55.0	55.0
Effective Green, g (s)	36.1	36.1	36.1	42.9	42.9	42.9	45.8	45.8	45.8	45.8	45.8	45.8
Actuated g/C Ratio	0.38	0.38	0.38	0.46	0.46	0.46	0.49	0.49	0.49	0.49	0.49	0.49
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1201	1007	1007	1121	1121	1121	1064	1064	1064	1064	1064	1064
v/s Ratio Prot	c0.20	0.14	0.14	60.20	60.20	60.20	c0.04	c0.04	c0.04	c0.04	c0.04	c0.04
v/s Ratio Perm	0.53	0.37	0.37	0.44	0.44	0.44	0.54	0.54	0.54	0.54	0.54	0.54
v/c Ratio	22.4	20.8	20.8	17.4	17.4	17.4	16.8	16.8	16.8	16.8	16.8	16.8
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	0.5	0.2	0.2	0.3	0.3	0.3	0.5	0.5	0.5	0.5	0.5	0.5
Incremental Delay, d2	22.9	21.1	21.1	17.7	17.7	17.7	17.3	17.3	17.3	17.3	17.3	17.3
Delay (s)	C	C	C	B	B	B	B	B	B	B	B	B
Level of Service	C	C	C	B	B	B	B	B	B	B	B	B
Approach Delay (s)	22.9	21.1	21.1	17.7	17.7	17.7	17.3	17.3	17.3	17.3	17.3	17.3
Approach LOS	C	C	C	B	B	B	B	B	B	B	B	B
Intersection Summary												
HCM 2000 Control Delay	19.8 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.59											
Actuated Cycle Length (s)	94.1 Sum of lost time (s)											
Intersection Capacity Utilization	98.0% ICU Level of Service F											
Analysis Period (min)	15											
c Critical Lane Group												

3. Sackville Street & Gerrard Street E  
 Timings  
 Existing PM Model  
 10-25-2022

Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø6	Ø9	Ø10
Lane Configurations	4TB	4TB	4TB	4TB	4TB	4TB	4TB			
Traffic Volume (vph)	935	35	505	5	0	135	40			
Future Volume (vph)	935	35	505	5	0	135	40			
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA			
Protected Phases	2 9	6 10	6 10	8	4	2	6			
Permitted Phases	2 9	6 10	6 10	8	4	2	6			
Detector Phase	2	6	6	8	8	4	4			
Switch Phase										
Minimum Initial (s)	20.0	20.0	20.0	20.0	20.0	16.0	16.0			
Minimum Split (s)	25.7	25.7	25.7	21.0	21.0	21.0	21.0			
Total Split (%)	24.3%	24.3%	24.3%	24.3%	24.3%	41%	41%			
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	2.0			
All-Red Time (s)	2.7	2.7	2.7	2.7	2.7	2.0	2.0			
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0			
Total Lost Time (s)	4.7	4.7	4.7	4.7	4.7	4.7	4.7			
Lead-Lag										
Lead-Lag Optimize?										
Recall Mode										
Act Effect Green (s)	37.2	37.2	37.2	21.7	21.7	21.7	21.7			
Actuated g/C Ratio	0.55	0.55	0.55	0.32	0.32	0.32	0.32			
v/c Ratio	0.52	0.35	0.35	0.06	0.06	0.49	0.49			
Control Delay	10.1	8.6	8.6	5.0	5.0	26.2	26.2			
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Total Delay	10.1	8.6	8.6	5.0	5.0	26.2	26.2			
LOS	B	A	A	A	A	C	C			
Approach Delay	10.1	8.6	8.6	5.0	5.0	26.2	26.2			
Approach LOS	B	A	A	A	A	C	C			
Intersection Summary										
Cycle Length	107									
Actuated Cycle Length	67.8									
Natural Cycle	85									
Control Type	Actuated-Uncoordinated									
Maximum v/c Ratio	0.52									
Intersection Signal Delay	11.5									
Intersection Capacity Utilization	66.1%									
Analysis Period (min)	15									





Existing PM Model  
10-25-2022

HCM Signalized Intersection Capacity Analysis  
3. Sackville Street & Gerrard Street E

Existing PM Model  
10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	990	562	26	209
Lane Group Flow (vph)	0.52	0.35	0.06	0.49
v/c Ratio	10.1	8.6	5.0	26.2
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	10.1	8.6	5.0	26.2
Total Delay	36.4	18.3	0.0	22.0
Queue Length 50th (m)	49.3	26.7	3.8	47.6
Queue Length 95th (m)	138.2	47.8	34.3	15.3
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2215	1862	470	430
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.45	0.30	0.06	0.49
<b>Intersection Summary</b>				

	EBT	EBR	WBL	WBT	NBL	NBR	SBL	SBR
Lane Configurations	4+4	4+4	4+4	4+4	4+4	4+4	4+4	4+4
Traffic Volume (vph)	0	935	15	35	505	0	5	0
Future Volume (vph)	0	935	15	35	505	0	5	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. psd/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	3448	3448	3363	3363	1480	1480	1682	1682
Satd. Flow (prot)	1.00	1.00	0.86	0.86	0.95	0.95	0.78	0.78
Flt Permitted	3448	3448	2901	2901	1416	1416	1359	1359
Satd. Flow (perm)	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Peak-hour factor, PHF	0	974	16	36	526	0	5	0
Adj. Flow (vph)	0	2	0	0	0	0	18	0
RTOR Reduction (vph)	0	988	0	0	562	0	8	0
Lane Group Flow (vph)	85	70	70	85	90	60	60	90
Confl. Bikes (#/hr)	0	60	15	15	5	5	10	10
Heavy Vehicles (%)	0%	3%	0%	0%	6%	0%	5%	0%
Turn Type	NA	Perm	NA	NA	Perm	NA	Perm	NA
Protected Phases	2.9		6.10		8		4	
Permitted Phases			6.10		8		4	
Actuated Green, G (s)	39.7		39.7		20.6		20.6	
Effective Green, g (s)	40.7		40.7		21.6		21.6	
Actuated g/C Ratio	0.60		0.60		0.32		0.32	
Clearance Time (s)					5.7		5.7	
Vehicle Extension (s)					3.0		3.0	
Lane Grp Cap (vph)	2063		1736		449		431	
v/s Ratio Prot	c0.29							
v/s Ratio Perm	0.48		0.19		0.01		0.15	
v/c Ratio	7.7		0.32		0.02		0.48	
Uniform Delay, d1	1.00		6.8		15.9		18.6	
Progression Factor	0.2		1.00		1.00		1.00	
Incremental Delay, d2	7.9		0.1		0.0		0.8	
Delay (s)	A		6.9		15.9		19.5	
Level of Service	A		A		B		B	
Approach Delay (s)	7.9		6.9		15.9		19.5	
Approach LOS	A		A		B		B	
<b>Intersection Summary</b>								
HCM 2000 Control Delay	9.0			HCM 2000 Level of Service			A	
HCM 2000 Volume to Capacity ratio	0.50							
Actuated Cycle Length (s)	68.0			Sum of lost time (s)			9.7	
Intersection Capacity Utilization	66.1%			ICU Level of Service			C	
Analysis Period (min)	15							
c Critical Lane Group								

HCM Unsignalized Intersection Capacity Analysis  
 4: Gerrard Street E & Gifford Street

HCM Unsignalized Intersection Capacity Analysis  
 5: Gerrard Street E & Nasmith Avenue

Existing PM Model  
 10-25-2022

Existing PM Model  
 10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4A	4B		W	
Traffic Volume (veh/h)	10	1080	535	5	15	5
Future Volume (Veh/h)	10	1080	535	5	15	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	12	1271	629	6	18	6
Pedestrians		5			65	
Lane Width (m)		3.5			3.0	
Walking Speed (m/s)		1.1			1.1	
Percent Blockage		0			5	
Right turn flare (veh)		None			None	
Median type		None			None	
Median storage (veh)						
Upstream signal (m)		72			141	
pX platoon unblocked		0.99			0.84	0.99
VC conflicting volume		700			1356	388
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol		685			1014	370
IC single (s)		4.1			6.8	6.9
IC 2 stage (s)		2.2			3.5	3.3
p0 queue free %		99			90	99
CM capacity (veh/h)		867			188	595
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	436	847	419	216	24	
Volume Left	12	0	0	0	18	
Volume Right	0	0	0	0	6	
cSH	867	1700	1700	1700	227	
Volumes to Capacity	0.01	0.50	0.25	0.13	0.11	
Queue Length 95th (m)	0.3	0.0	0.0	0.0	2.7	
Control Delay (s)	0.4	0.0	0.0	0.0	22.7	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		22.7	
Approach LOS					C	
Intersection Summary						
Average Delay	0.4					
Intersection Capacity Utilization	48.4%					
ICU Level of Service	A					
Analysis Period (min)	15					

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4A	4B		W	
Traffic Volume (veh/h)	5	1090	535	5	5	5
Future Volume (Veh/h)	5	1090	535	5	5	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	6	1253	615	6	6	6
Pedestrians					60	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					5	
Right turn flare (veh)					None	
Median type					None	
Median storage (veh)						
Upstream signal (m)					145	68
pX platoon unblocked					0.96	0.87
VC conflicting volume					681	1316
VC1 stage 1 conf vol						370
VC2 stage 2 conf vol						
VCu unblocked vol					588	868
IC single (s)					4.1	6.8
IC 2 stage (s)					2.2	3.5
p0 queue free %					99	98
CM capacity (veh/h)					915	244
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	424	835	410	211	12	
Volume Left	6	0	0	0	6	
Volume Right	0	0	0	0	6	
cSH	915	1700	1700	1700	359	
Volumes to Capacity	0.01	0.49	0.24	0.12	0.03	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.8	
Control Delay (s)	0.2	0.0	0.0	0.0	15.4	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		15.4	
Approach LOS					C	
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	43.6%					
ICU Level of Service	A					
Analysis Period (min)	15					

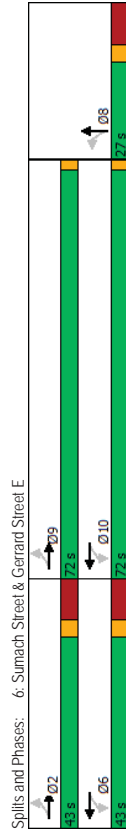
Timings Existing PM Model  
10-25-2022

Queues Existing PM Model  
10-25-2022

6: Summach Street & Gerrard Street E

6: Summach Street & Gerrard Street E

	EBL	EBT	WBL	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Group	EBL	EBT	WBL	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	10	1075	20	530	25				
Traffic Volume (vph)	10	1075	20	530	25				
Future Volume (vph)	Perm	NA	Perm	NA	NA				
Turn Type	2.9	6.10	8	2	6	9	10		
Protected Phases	2.9	6.10							
Detector Phase	2.9	2.9	6.10	6.10	8				
Switch Phase									
Minimum Initial (s)	15.0	11.0	11.0	11.0	5.0				
Minimum Split (s)	25.4	26.2	26.2	72.0	72.0				
Total Split (s)	27.0	43.0	43.0	72.0	72.0				
Total Split (%)	19.0%	30%	30%	51%	51%				
Yellow Time (s)	3.0	3.0	3.0	2.0	2.0				
All-Red Time (s)	7.4	7.2	7.2	0.0	0.0				
Lost Time Adjust (s)	-1.0								
Total Lost Time (s)	9.4								
Lead/Lag									
Lead-Lag Optimize?									
Recall Mode	None	Min	Min	None	None				
Act Effct Green (s)	65.7	65.7	16.4						
Actuated g/C Ratio	0.72	0.72	0.18						
v/c Ratio	0.48	0.32	0.29						
Control Delay	8.9	7.1	24.2						
Queue Delay	0.0	0.0	0.0						
Total Delay	8.9	7.1	24.2						
LOS	A	A	C						
Approach Delay	8.9	7.1	24.2						
Approach LOS	A	A	C						
Intersection Summary									
Cycle Length: 142									
Actuated Cycle Length: 91.6									
Natural Cycle: 125									
Control Type: Actuated-Uncoordinated									
Maximum v/c Ratio: 0.48									
Intersection Signal Delay: 9.0									
Intersection Capacity Utilization 66.4%									
Analysis Period (min) 15									



HCM Signalized Intersection Capacity Analysis  
6: Sumach Street & Gerrard Street E

Existing PM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB			4B					
Traffic Volume (vph)	10	1075	10	20	530	85	10	25	55	0	0	0
Future Volume (vph)	10	1075	10	20	530	85	10	25	55	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.0	3.0	3.0	3.5	3.0
Total Lost time (s)		9.2		9.2			9.4					
Lane Util. Factor	0.95	0.95		0.95			1.00					
Frbp. ped/bikes	1.00	0.96		0.96			0.95					
Frbp. ped/bikes	1.00	1.00		1.00			0.98					
Frt	1.00	0.98		0.98			0.99					
Flt Protected	1.00	1.00		1.00			0.99					
Sat'd. Flow (prot)	3482	3235		3235			1600					
Flt Permitted	0.95	0.89		0.89			0.99					
Sat'd. Flow (perm)	3301	2885		2885			1600					
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	10	1108	10	21	546	88	10	26	57	0	0	0
RTOR Reduction (vph)	0	0	0	8	0	0	40	0	0	0	0	0
Lane Group Flow (vph)	0	1128	0	0	647	0	0	53	0	0	0	0
Conf. Bikes (#/hr)	95	95	95	95	95	80	35	35	35	35	80	80
Heavy Vehicles (%)	0%	2%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perim	MA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2.9		6.10		6.10		8				
Permitted Phases		2.9		6.10		6.10		8				
Actuated Green, G (s)		70.2		70.2		70.2		11.2				
Effective Green, g (s)		71.2		71.2		71.2		12.2				
Actuated g/C Ratio		0.76		0.76		0.76		0.13				
Clearance Time (s)								10.4				
Vehicle Extension (s)								3.0				
Lane Grp Cap (vph)		2505		2189		2189		208				
v/s Ratio Prot												
v/s Ratio Perm		60.34		0.22		0.22		0.03				
v/c Ratio		0.45		0.30		0.30		0.25				
Uniform Delay, d1		4.1		3.5		36.7		36.7				
Progression Factor		1.00		1.00		1.00		1.00				
Incremental Delay, d2		0.1		0.1		0.6		0.6				
Delay (s)		4.3		3.6		37.4		37.4				
Level of Service		A		A		D		D				
Approach Delay (s)		4.3		3.6		37.4		37.4				
Approach LOS		A		A		D		D				
Intersection Summary												
HCM 2000 Control Delay		5.7		HCM 2000 Level of Service								A
HCM 2000 Volume to Capacity ratio		0.46										
Actuated Cycle Length (s)		93.8		Sum of lost time (s)								19.6
Intersection Capacity Utilization		66.4%		ICU Level of Service								C
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
7: Gerrard Street E & Sword Street

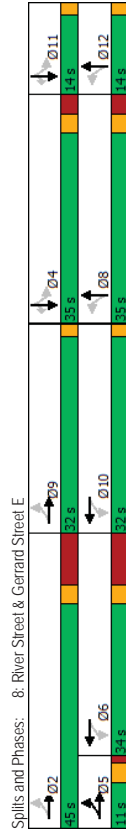
Existing PM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR
Lane Configurations		4TB		4TB			4TB	
Traffic Volume (veh/h)	0	1130	625	0	15	10	15	10
Future Volume (Veh/h)	0	1130	625	0	15	10	15	10
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	1256	694	0	17	11	17	11
Pedestrians							65	
Lane Width (m)							3.0	
Walking Speed (m/s)							1.1	
Percent Blockage							5	
Right turn flare (veh)								
Median type		None		None				
Median storage (veh)								
Upstream signal (m)		100		91				
pX platoon unblocked	0.95						0.91	0.95
vC, conflicting volume	759						1387	412
VC1, stage 1 conf vol								
VC2, stage 2 conf vol								
VCU, unblocked vol	638						972	273
IC, single (s)	4.1						6.8	7.1
IC, 2 stage (s)								
IF (s)	2.2						3.5	3.4
p0 queue free %	100						92	98
dM capacity (veh/h)	862						219	638
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2		
Volumes Total	628	628	347	347	28	28		
Volume Left	0	0	0	0	0	17		
Volume Right	0	0	0	0	0	11		
CSH	1700	1700	1700	1700	295	295		
Volumes to Capacity	0.37	0.37	0.20	0.20	0.20	0.09		
Queue Length 95th (m)	0.0	0.0	0.0	0.0	2.4	2.4		
Control Delay (s)	0.0	0.0	0.0	0.0	18.5	18.5		
Lane LOS					C	C		
Approach Delay (s)	0.0	0.0	0.0	0.0	18.5	18.5		
Approach LOS					C	C		
Intersection Summary								
Average Delay					0.3			
Intersection Capacity Utilization					41.2%			
ICU Level of Service					A			
Analysis Period (min)					15			

Timings  
8: River Street & Gerrard Street E

Existing PM Model  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Lane Configurations	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB			
Traffic Volume (vph)	225	855	70	365	70	285	165	360	190			
Future Volume (vph)	225	855	70	365	70	285	165	360	190			
Turn Type	custom	NA	Perm	NA	Perm	NA	Perm	NA	Perm			
Protected Phases	5	5.2.9	6.10	6.10	8.12	8.12	4.11	4.11	4.11	2	4	6
Permitted Phases	5	5.2.9	6.10	6.10	8.12	8.12	4.11	4.11	4.11			
Detector Phase												
Switch Phase												
Minimum Initial (s)	6.0									23.0	19.0	23.0
Minimum Spilt (s)	10.0									33.8	30.0	33.8
Total Spilt (s)	11.0									45.0	35.0	34.0
Total Spilt (%)	8.7%									36%	28%	27%
Yellow Time (s)	3.0									3.0	3.0	3.0
All-Red Time (s)	1.0									7.8	3.0	7.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag	Lead											
Lead-Lag Optimize?	Yes											
Recall Mode	None									Min	Min	Min
Act Effct Green (s)	51.9		33.5	32.9	32.9	32.9	32.9	32.9	32.9			
Actuated g/C Ratio	0.56		0.36	0.35	0.35	0.35	0.35	0.35	0.35			
v/C Ratio	0.85		0.82	0.30	0.74	0.92	0.57	0.31	0.31			
Control Delay	22.7		35.6	29.5	35.4	82.5	30.4	5.3	5.3			
Queue Delay	0.8		0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Total Delay	23.6		35.6	29.5	35.4	82.5	30.4	5.3	5.3			
LOS	C		D	C	D	F	C	C	A			
Approach Delay	23.6		35.6	34.6	34.6	35.8						
Approach LOS	C		D	C	C	D						
Intersection Summary												
Cycle Length: 126												
Actuated Cycle Length: 93												
Natural Cycle: 120												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.92												
Intersection Signal Delay: 30.7												
Intersection Capacity Utilization 113.3%												
Analysis Period (min) 15												



Timings  
8: River Street & Gerrard Street E

Existing PM Model  
10-25-2022

Lane Group	Ø8	Ø9	Ø10	Ø11	Ø12
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Turn Type					
Protected Phases	8	9	10	11	12
Permitted Phases					
Detector Phase					
Switch Phase					
Minimum Initial (s)	19.0	5.0	5.0	5.0	5.0
Minimum Spilt (s)	30.0	32.0	32.0	14.0	14.0
Total Spilt (s)	35.0	32.0	32.0	14.0	14.0
Total Spilt (%)	28%	25%	25%	11%	11%
Yellow Time (s)	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)	3.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	Min	None	None	None	None
Act Effct Green (s)					
Actuated g/C Ratio					
v/C Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Intersection Summary					

Queues  
8: River Street & Gerrard Street E

10-25-2022

	EBT	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group	1193	562	73	464	172	375	198
Lane Group Flow (vph)	0.85	0.82	0.30	0.74	0.92	0.57	0.31
v/c Ratio	22.7	35.6	29.5	35.4	82.5	30.4	5.3
Control Delay	0.8	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	23.6	35.6	29.5	35.4	82.5	30.4	5.3
Total Delay	64.8	41.9	7.6	55.4	23.7	43.7	0.0
Queue Length 50th (m)	111.9	74.3	24.9	127.8	#80.9	99.8	15.1
Queue Length 95th (m)	67.0	81.6		126.5		61.7	
Internal Link Dist (m)			30.0				
Turn Bay Length (m)	1409	688	240	624	187	658	629
Base Capacity (vph)	60	0	0	0	0	0	0
Stavation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.88	0.82	0.30	0.74	0.92	0.57	0.31

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
8: River Street & Gerrard Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB								
Traffic Volume (vph)	225	855	65	70	365	105	70	285	160	165	360	190
Future Volume (vph)	225	855	65	70	365	105	70	285	160	165	360	190
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	3.0	9.8		9.8			5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	0.95		0.95			1.00	1.00	1.00	1.00	1.00	1.00
Frpb, ped/bikes	0.99	0.99		0.99			1.00	0.98	1.00	1.00	0.97	0.97
Flpb, psd/bikes	1.00	1.00		1.00			0.99	1.00	0.98	1.00	1.00	1.00
Flt	0.99	0.99		0.99			1.00	0.95	1.00	1.00	0.85	0.85
Flt Protected	0.99	0.99		0.99			0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	3387	3270		3270			1622	1729	1639	1860	1426	1426
Flt Permitted	0.69	0.57		0.57			0.40	1.00	0.31	1.00	1.00	1.00
Satd. Flow (perm)	2365	1865		1865			681	1729	527	1860	1426	1426
Peak-hour factor, PHF	0.96	0.96		0.96			0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	234	891	68	73	380	109	73	297	167	172	375	198
RTOR Reduction (vph)	0	4	0	0	16	0	0	15	0	0	0	118
Lane Group Flow (vph)	0	1189	0	0	546	0	73	449	0	172	375	80
Confl. Peds. (#/hr)	40	80	80	40	15	40	15	40	40	40	15	10
Confl. Bikes (#/hr)			35		15		15		15		15	
Heavy Vehicles (%)	3%	2%	3%	0%	4%	0%	3%	0%	1%	1%	1%	2%
Turn Type	custom	NA		NA			NA		NA		NA	Perm
Protected Phases	5	5 2 9		6 10			8 12		8 12		4 11	4 11
Permitted Phases	2 9			6 10			8 12		8 12		4 11	4 11
Actuated Green, G (s)	54.0			42.8			37.5		37.5		37.5	37.5
Effective Green, g (s)	45.2			43.8			38.5		38.5		38.5	38.5
Actuated g/C Ratio	0.47			0.46			0.40		0.40		0.40	0.40
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1207			855			274		697		212	749
v/s Ratio Prot	c0.08						0.26				0.20	
v/s Ratio Perm	c0.38			c0.29			0.11				c0.33	0.06
v/c Ratio	0.98			0.64			0.27		0.64		0.81	0.50
Uniform Delay, d1	24.8			19.8			19.1		23.0		21.3	18.0
Progression Factor	1.00			1.00			1.00		1.00		1.00	1.00
Incremental Delay, d2	22.1			1.6			0.5		2.1		20.5	0.5
Delay (s)	46.9			21.4			19.6		25.0		45.8	21.8
Level of Service	D			C			B		C		D	C
Approach Delay (s)	46.9			21.4			24.3		26.4		26.4	
Approach LOS	D			C			C		C		C	C
Intersection Summary												
HCM 2000 Control Delay	33.2 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.96											
Actuated Cycle Length (s)	95.5 Sum of lost time (s) 19.8											
Intersection Capacity Utilization	113.3% ICU Level of Service H											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 9: Sackville Street & Site Driveway  
 10-25-2022

HCM Unsignalized Intersection Capacity Analysis  
 10: Parliament Street & Oak Street  
 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	10	0	0	5	0	5	0	10	0	5	85	0
Future Volume (Veh/h)	10	0	0	5	0	5	0	10	0	5	85	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	12	0	0	6	0	6	0	12	0	6	100	0
Pedestrians	60			35			5			15		
Lane Width (m)	3.5			3.5			3.5			3.5		
Walking Speed (m/s)	1.1			1.1			1.1			1.1		
Percent Blockage	5			3			0			1		
Right turn flare (veh)												
Median type							None					
Median storage (veh)												
Upstream signal (m)												58
pX platoon unblocked												
VC, conflicting volume	205	219	165	164	219	62	160					47
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	205	219	165	164	219	62	160					47
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1					4.1
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2					2.2
p0 queue free %	98	100	100	99	100	99	100					100
CM capacity (veh/h)	657	624	834	726	624	964	1356					1525
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volumes Total	12	12	12	106								
Volume Left	12	6	0	6								
Volume Right	0	6	0	0								
cSH	657	828	1356	1525								
Volumes to Capacity	0.02	0.01	0.00	0.00								
Queue Length 95th (m)	0.4	0.3	0.0	0.1								
Control Delay (s)	10.6	9.4	0.0	0.4								
Lane LOS	B	A	A	A								
Approach Delay (s)	10.6	9.4	0.0	0.4								
Approach LOS	B	A	A	A								
Intersection Summary												
Average Delay	2.0											
Intersection Capacity Utilization	26.6%											
ICU Level of Service	A											
Analysis Period (min)	15											

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	10	35	455	25	25	355
Future Volume (Veh/h)	10	35	455	25	25	355
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	12	41	535	29	29	418
Pedestrians	220		5			55
Lane Width (m)	3.0		3.5			3.5
Walking Speed (m/s)	1.1		1.1			1.1
Percent Blockage	17		0			5
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			151			143
pX platoon unblocked	0.95	0.95		0.95		0.95
VC, conflicting volume	1042	557		784		
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	934	422		662		
IC, single (s)	6.8	6.9		4.3		
IC, 2 stage (s)						
IF (s)	3.5	3.3		2.3		
p0 queue free %	94	91		96		
CM capacity (veh/h)	202	440		678		
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volumes Total	53	357	207	168	279	
Volume Left	12	0	0	29	0	
Volume Right	41	0	29	0	0	
cSH	347	1700	1700	678	1700	
Volumes to Capacity	0.15	0.21	0.12	0.04	0.16	
Queue Length 95th (m)	4.0	0.0	0.0	1.0	0.0	
Control Delay (s)	17.2	0.0	0.0	2.2	0.0	
Lane LOS	C	A	A	A	A	
Approach Delay (s)	17.2	0.0	0.0	0.8		
Approach LOS	C	A	A	A		
Intersection Summary						
Average Delay	1.2					
Intersection Capacity Utilization	46.5%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis  
11: Oak Street & Dreamer's Way

HCM Unsignalized Intersection Capacity Analysis  
12: Regent Street & Oak Street

Existing PM Model  
10-25-2022

Existing PM Model  
10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Traffic Volume (veh/h)	0	55	35	0	5	5
Future Volume (Veh/h)	0	55	35	0	5	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	59	38	0	5	5
Pedestrians	25	10			40	
Lane Width (m)	3.5	3.5	3.0		3.0	
Walking Speed (m/s)	1.1	1.1	1.1		1.1	
Percent Blockage	2	1			3	
Right turn flare (veh)						
Median type	None	None				
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	78				147	103
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	78				147	103
IC, single (s)	4.1				6.4	6.2
IC, 2 stage (s)						
IF (s)	2.2				3.5	3.3
p0 queue free %	100				99	99
CM capacity (veh/h)	1487				817	908
Direction, Lane #	EB 1	WB 1	SB 1			
Volumes Total	59	38	10			
Volume Left	0	0	5			
Volume Right	0	0	5			
GSH	1700	1700	860			
Volumes to Capacity	0.03	0.02	0.01			
Queue Length 95th (m)	0.0	0.0	0.3			
Control Delay (s)	0.0	0.0	9.2			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.2			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			26.4%		ICU Level of Service	A
Analysis Period (min)			15			

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑		↑	↑	↑
Traffic Volume (veh/h)	60	0	0	20	15	45
Future Volume (Veh/h)	60	0	0	20	15	45
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	72	0	0	24	18	54
Pedestrians	15			15	40	
Lane Width (m)	3.5			3.5	3.0	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	1			1	3	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume			112		151	127
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol			112		151	127
IC, single (s)			4.1		6.4	6.2
IC, 2 stage (s)						
IF (s)			2.2		3.5	3.3
p0 queue free %			100		98	94
CM capacity (veh/h)			1445		809	889
Direction, Lane #	EB 1	WB 1	NB 1			
Volumes Total	72	24	72			
Volume Left	0	0	18			
Volume Right	0	0	54			
GSH	1700	1700	867			
Volumes to Capacity	0.04	0.01	0.08			
Queue Length 95th (m)	0.0	0.0	2.1			
Control Delay (s)	0.0	0.0	9.5			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.5			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			4.1			
Intersection Capacity Utilization			26.4%		ICU Level of Service	A
Analysis Period (min)			15			



HCM Unsignalized Intersection Capacity Analysis  
 13: Oak Street & Site Driveway

HCM Unsignalized Intersection Capacity Analysis  
 14: Sackville Street & Oak Street

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	10	95	20	0	5	0
Future Volume (Veh/h)	10	95	20	0	5	0
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	12	114	24	0	6	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	59				262	59
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	59				262	59
IC, single (s)	4.1				6.4	6.2
IC, 2 stage (s)	2.2				3.5	3.3
p0 queue free %	99				99	100
CM capacity (veh/h)	1516				666	986
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	126	24	6			
Volume Left	12	0	6			
Volume Right	0	0	0			
cSH	1516	1700	666			
Volume to Capacity	0.01	0.01	0.01			
Queue Length 95th (m)	0.2	0.0	0.2			
Control Delay (s)	0.8	0.0	10.5			
Lane LOS	A	A	B			
Approach Delay (s)	0.8	0.0	10.5			
Approach LOS			B			
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			22.2%		ICU Level of Service	A
Analysis Period (min)			15			

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Volume (vph)	10	75	20	5	0	0
Future Volume (vph)	10	75	20	5	0	0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	11	83	22	6	0	0
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	116	12	95			
Volume Left (vph)	11	6	11			
Volume Right (vph)	22	0	6			
Head (s)	-0.07	0.27	0.03			
Departure Headway (s)	4.1	4.5	4.2			
Degree Utilization, x	0.13	0.01	0.11			
Capacity (veh/h)	864	775	824			
Control Delay (s)	7.7	7.6	7.7			
Approach Delay (s)	7.7	7.6	7.7			
Approach LOS	A	A	A			
Intersection Summary						
Delay			7.7			
Level of Service			A			
Intersection Capacity Utilization			32.3%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis  
 15: Sumach Street & Oak Street

HCM Unsignalized Intersection Capacity Analysis  
 16: Tubman Avenue & Oak Street

Existing PM Model  
 10-25-2022

Existing PM Model  
 10-25-2022

Movement	EBL	EBT	NB1	NB2	WBR	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Stop	Stop	Stop				Stop			Stop	
Traffic Volume (vph)	20	55	10	0	0	0	5	70	75	10	30	5
Future Volume (vph)	20	55	10	0	0	0	5	70	75	10	30	5
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	22	60	11	0	0	0	5	77	82	11	33	5
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total (vph)	93	164	49									
Volume Left (vph)	22	5	11									
Volume Right (vph)	11	82	5									
Head (s)	0.05	-0.22	0.11									
Departure Headway (s)	4.4	4.0	4.4									
Degree Utilization, x	0.11	0.18	0.06									
Capacity (veh/h)	779	882	795									
Control Delay (s)	8.0	7.8	7.7									
Approach Delay (s)	8.0	7.8	7.7									
Approach LOS	A	A	A									
Intersection Summary												
Delay			7.8									
Level of Service			A									
Intersection Capacity Utilization			32.2%									A
Analysis Period (min)			15									

Movement	EBT	EBR	WBL	WBT	NBL	NBT	NBR
Lane Configurations	1						
Traffic Volume (veh/h)	130	10	0	0	0	0	0
Future Volume (Veh/h)	130	10	0	0	0	0	0
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	140	11	0	0	0	0	0
Pedestrians	20			5	30		
Lane Width (m)	3.5			0.0	0.0		
Walking Speed (m/s)	1.1			1.1	1.1		
Percent Blockage	2			0	0		
Right turn flare (veh)							
Median type	None			None			
Median storage (veh)							
Upstream signal (m)							
PX, platoon unblocked							
VC, conflicting volume		181			196		180
VC1, stage 1 conf vol							
VC2, stage 2 conf vol							
VCu, unblocked vol		181			196		180
IC, single (s)		4.1			6.4		6.2
IC, 2 stage (s)							
p0 queue free %		100			3.5		3.3
IF (s)					100		100
pM capacity (veh/h)		1407			784		867
Direction, Lane #	EB 1						
Volume Total	151						
Volume Left	0						
Volume Right	11						
rSH	1700						
Volume to Capacity	0.09						
Queue Length 95th (m)	0.0						
Control Delay (s)	0.0						
Lane LOS							
Approach Delay (s)	0.0						
Approach LOS							
Intersection Summary							
Average Delay					0.0		
Intersection Capacity Utilization					22.8%		ICU Level of Service
Analysis Period (min)					15		A

HCM Unsignalized Intersection Capacity Analysis  
17: Oak Street & Site Driveway

HCM Unsignalized Intersection Capacity Analysis  
18: River Street & Oak Street

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	125	0	0	10	0
Traffic Volume (veh/h)	5	125	0	0	10	0
Future Volume (Veh/h)	5	125	0	0	10	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	6	142	0	0	11	0
Pedestrians	5	5	5	5	25	5
Lane Width (m)	3.5	0.0	3.0	3.0	1.1	1.1
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	0	0	0	0	2	2
Right turn flare (veh)	None	None	None	None	None	None
Median type	None	None	None	None	None	None
Median storage (veh)	None	None	None	None	None	None
Upstream signal (m)	None	None	None	None	None	None
pX platoon unblocked	None	None	None	None	None	None
VC, conflicting volume	25	184	30	30	30	30
VC1, stage 1 conf vol	25	184	30	30	30	30
VC2, stage 2 conf vol	4.1	6.4	6.2	6.2	6.2	6.2
IC, single (s)	2.2	3.5	3.3	3.3	3.3	3.3
IF (s)	100	99	100	100	100	100
p0 queue free %	1572	792	792	1026	1026	1026
CM capacity (veh/h)	1572	792	792	1026	1026	1026
Direction, Lane #	EB 1	SB 1				
Volumes Total	148	11				
Volume Left	6	11				
Volume Right	0	0				
cSH	1572	792				
Volumes to Capacity	0.00	0.01				
Queue Length 95th (m)	0.1	0.3				
Control Delay (s)	0.3	9.6				
Lane LOS	A	A				
Approach Delay (s)	0.3	9.6				
Approach LOS	A	A				
Intersection Summary						
Average Delay	22.2%	1.0				
Intersection Capacity Utilization	22.2%	22.2%				
Analysis Period (min)	15	15				
ICU Level of Service		A				

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	85	5	45	15	0	40	0	390	30	30	465
Traffic Volume (veh/h)	85	5	45	15	0	40	0	390	30	30	465
Future Volume (Veh/h)	85	5	45	15	0	40	0	390	30	30	465
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	92	5	49	16	0	43	0	424	33	33	505
Pedestrians	75	75	45	45	50	50	5	5	5	5	5
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	7	7	4	4	4	4	4	4	4	4	4
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None	None
Median type	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)	None	None	None	None	None	None	None	None	None	None	None
Upstream signal (m)	None	None	None	None	None	None	None	None	None	None	None
pX platoon unblocked	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
VC, conflicting volume	1134	1148	630	1158	1132	490	580	580	502	502	502
VC1, stage 1 conf vol	1069	1084	473	1096	1065	490	414	414	502	502	502
VC2, stage 2 conf vol	7.1	6.5	6.2	7.1	6.5	6.2	4.1	4.1	4.1	4.1	4.1
IC, single (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2	2.2	2.2	2.2	2.2
IF (s)	30	97	89	86	100	92	100	100	97	97	97
p0 queue free %	131	161	442	117	165	556	914	914	1030	1030	1030
CM capacity (veh/h)	131	161	442	117	165	556	914	914	1030	1030	1030
Direction, Lane #	EB 1	WB 1	NB 1	SB 1							
Volumes Total	146	59	457	538							
Volume Left	92	16	0	33							
Volume Right	49	43	33	0							
cSH	173	275	1700	1030							
Volumes to Capacity	0.84	0.21	0.27	0.03							
Queue Length 95th (m)	44.8	6.0	0.0	0.8							
Control Delay (s)	86.0	21.6	0.0	0.9							
Lane LOS	F	C	A	A							
Approach Delay (s)	86.0	21.6	0.0	0.9							
Approach LOS	F	C	C	C							
Intersection Summary											
Average Delay	71.4%	11.9									
Intersection Capacity Utilization	71.4%	71.4%									
Analysis Period (min)	15	15									
ICU Level of Service		C									

19: Parliament Street & Cole Street

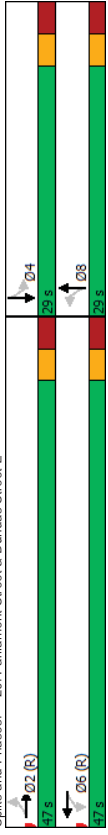
Existing PM Model  
10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			←	←	←	←
Traffic Volume (veh/h)	0	0	480	20	25	340
Future Volume (Veh/h)	0	0	480	20	25	340
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	0	0	571	24	30	405
Pedestrians	255	0	25	0	0	15
Lane Width (m)	0.0	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	0	2	2	2	2	2
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			80			214
pX platoon unblocked						
VC, conflicting volume	1126	568				850
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	938	325				635
IC, single (s)	6.8	6.9				4.1
IC, 2 stage (s)						
p0 queue free %	3.5	3.3				2.2
IF (s)	100	100				97
CM capacity (veh/h)	229	607				871
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	381	214	165	270		
Volume Left	0	0	30	0		
Volume Right	0	24	0	0		
cSH	1700	1700	871	1700		
Volumes to Capacity	0.22	0.13	0.03	0.16		
Queue Length 95th (m)	0.0	0.0	0.8	0.0		
Control Delay (s)	0.0	0.0	2.0	0.0		
Lane LOS			A			
Approach Delay (s)	0.0		0.7			
Approach LOS						
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			41.6%			ICU Level of Service
Analysis Period (min)			15			A

20: Parliament Street & Dundas Street E

Existing PM Model  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		←	←	←	←	←	←	←
Traffic Volume (vph)	65	415	40	165	40	365	30	290
Future Volume (vph)	65	415	40	165	40	365	30	290
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2		6		8		4
Permitted Phases	2	2	6	6	8	8	4	4
Detector Phase								
Switch Phase								
Minimum Initial (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Minimum Split (s)	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	47.0	47.0	47.0	47.0	47.0	47.0	47.0	47.0
Total Split (%)	61.8%	61.8%	61.8%	61.8%	61.8%	61.8%	61.8%	61.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)			5.0			5.0		5.0
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode								
Act Effct Green (s)	41.8	41.8	41.8	41.8	41.8	41.8	41.8	41.8
Actuated g/C Ratio	0.55	0.55	0.55	0.55	0.55	0.55	0.55	0.55
v/C Ratio	0.39	0.39	0.23	0.23	0.53	0.53	0.42	0.42
Control Delay	10.3	10.8	10.8	10.8	23.1	23.1	21.6	21.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	10.3	10.8	10.8	10.8	23.1	23.1	21.6	21.6
LOS	B	B	B	B	C	C	C	C
Approach Delay			10.3		10.8		23.1	
Approach LOS			B		C		C	
Intersection Summary								
Cycle Length: 76								
Actuated Cycle Length: 76								
Offset: 9 (12%) Referenced to phase 2:EBTL and 6:WBT, Start of 1st Green								
Natural Cycle: 60								
Control Type: Actuated-Coordinated								
Maximum v/C Ratio: 0.53								
Intersection Signal Delay: 16.3								Intersection LOS: B
Intersection Capacity Utilization 93.3%								ICU Level of Service F
Analysis Period (min) 15								



Existing PM Model  
10-25-2022

20: Parliament Street & Dundas Street E

	→	←	↑	↓
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	606	309	494	382
v/c Ratio	0.39	0.23	0.63	0.42
Control Delay	10.3	10.8	23.1	21.6
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	10.3	10.8	23.1	21.6
Queue Length 50th (m)	22.8	8.5	29.6	22.0
Queue Length 95th (m)	34.0	22.1	42.1	32.5
Internal Link Dist (m)	85.0	103.5	45.0	56.1
Turn Bay Length (m)				
Base Capacity (vph)	1544	1374	930	904
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.39	0.22	0.63	0.42
Intersection Summary				

HCM Signalized Intersection Capacity Analysis  
20: Parliament Street & Dundas Street E

Existing PM Model  
10-25-2022

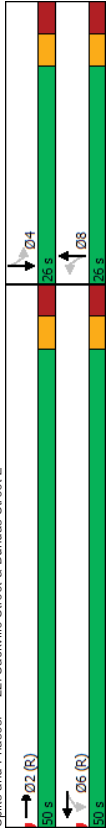
	→	←	↑	↓
Movement	EBL	EBT	EBR	WBL
Lane Configurations	4TB	4TB	4TB	4TB
Traffic Volume (vph)	65	415	60	40
Future Volume (vph)	65	415	60	40
Ideal Flow (vphpl)	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0
Total Lost time (s)	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.98	0.98	0.98	0.98
Frbp. psd/bikes	0.98	0.98	0.98	0.98
Frt	0.98	0.98	0.98	0.98
Flt Protected	0.99	0.99	0.99	0.99
Satd. Flow (prot)	3179	2971	3273	3209
Flt Permitted	0.87	0.82	0.88	0.88
Satd. Flow (perm)	2772	2466	2896	2823
Peak-hour factor, PHF	0.89	0.89	0.89	0.89
Adj. Flow (vph)	73	466	67	45
RTOR Reduction (vph)	0	13	0	0
Lane Group Flow (vph)	0	593	0	0
Confl. Peds. (#/hr)	260	155	155	260
Confl. Bikes (#/hr)	25	25	25	25
Heavy Vehicles (%)	3%	6%	5%	6%
Parking (#/hr)	0	0	0	0
Turn Type	Perm	NA	NA	Perm
Protected Phases	2	2	6	6
Permitted Phases	2	2	6	6
Actuated Green, G (s)	40.8	40.8	40.8	40.8
Effective Green, g (s)	41.8	41.8	41.8	41.8
Actuated g/C Ratio	0.55	0.55	0.55	0.55
Clearance Time (s)	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1524	1356	922	898
v/s Ratio Prot				
v/s Ratio Perm	0.21	0.12	0.17	0.13
v/c Ratio	0.39	0.22	0.53	0.42
Uniform Delay, d1	9.8	8.8	21.2	20.4
Progression Factor	1.00	1.27	1.00	1.00
Incremental Delay, d2	0.8	0.4	0.5	0.3
Delay (s)	10.5	11.5	21.8	20.7
Level of Service	B	B	C	C
Approach Delay (s)	10.5	11.5	21.8	20.7
Approach LOS	B	B	C	C
Intersection Summary				
HCM 2000 Control Delay	16.0	HCM 2000 Level of Service	B	
HCM 2000 Volume to Capacity ratio	0.44			
Actuated Cycle Length (s)	76.0	Sum of lost time (s)	10.0	
Intersection Capacity Utilization	93.3%	ICU Level of Service	F	
Analysis Period (min)	15			
c Critical Lane Group				

10-25-2022  
Existing PM Model  
HCM Unsignalized Intersection Capacity Analysis  
21: Regent Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	40	410	15	5	265	25	5	10	5	5	5	25
Traffic Volume (veh/h)	40	410	15	5	265	25	5	10	5	5	5	25
Future Volume (Veh/h)	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Sign Control	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Grade	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Peak Hour Factor	43	441	16	5	285	27	5	11	5	5	5	27
Hourly flow rate (vph)	30	35	3.5	3.5	3.5	3.5	125	125	125	135	135	135
Pedestrians	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Lane Width (m)	3	3	3	3	3	3	11	11	11	12	12	12
Walking Speed (m/s)	None	None	None	None	None	None	None	None	None	None	None	None
Percent Blockage	127	127	124	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Right turn flare (veh)	447	447	582	872	1117	364	770	1112	321	321	321	321
Median type	VC1, stage 1 conf vol	VC2, stage 2 conf vol	VC, conflicting volume	VC1, stage 1 conf vol	VC2, stage 2 conf vol	VC, conflicting volume	VC1, stage 1 conf vol	VC2, stage 2 conf vol	VC, conflicting volume	VC1, stage 1 conf vol	VC2, stage 2 conf vol	VC, conflicting volume
Median storage (veh)	447	447	546	840	1089	324	737	1083	321	321	321	321
Upstream signal (m)	4.1	4.1	4.4	7.5	6.5	7.9	7.5	6.5	6.9	6.9	6.9	6.9
pX platoon unblocked	2.2	2.2	2.4	3.5	4.0	3.8	3.5	4.0	3.3	3.3	3.3	3.3
IC, single (s)	96	96	99	97	93	99	98	97	95	95	95	95
IC, 2 stage (s)	990	990	814	167	159	477	201	161	584	584	584	584
CM capacity (veh/h)	EB1	EB2	WB1	WB2	NB1	NB1	SB1	SB1				
Direction Lane #	264	236	148	170	21	37						
Volumes Total	43	0	5	0	5	5						
Volume Left	0	16	0	27	5	27						
Volume Right	990	1700	814	1700	192	362						
cSH	0.04	0.14	0.01	0.10	0.11	0.10						
Volumes to Capacity	1.0	0.0	0.1	0.0	2.8	2.6						
Queue Length 95th (m)	1.8	0.0	0.4	0.0	26.1	16.1						
Control Delay (s)	A	A	A	D	C	C						
Lane LOS	1.0	0.2	26.1	16.1								
Approach Delay (s)	D	D	C									
Approach LOS												
Intersection Summary	Average Delay											
Average Delay	1.9											
Intersection Capacity Utilization	46.6%											
ICU Level of Service	A											
Analysis Period (min)	15											

10-25-2022  
Existing PM Model  
Timings  
22: Sackville Street & Dundas Street E

Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	40	15	220	40	0	30	35
Traffic Volume (vph)	400	15	220	40	0	30	35
Future Volume (vph)	400	15	220	40	0	30	35
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	6	6	8	8	4	4
Permitted Phases	2	6	6	8	8	4	4
Detector Phase							
Switch Phase							
Minimum Initial (s)	17.0	17.0	17.0	7.0	7.0	7.0	7.0
Minimum Split (s)	24.0	24.0	24.0	26.0	26.0	26.0	26.0
Total Split (s)	50.0	50.0	50.0	26.0	26.0	26.0	26.0
Total Split (%)	65.8%	65.8%	65.8%	34.2%	34.2%	34.2%	34.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag							
Lead-Lag Optimize?							
Recall Mode	C-Min	C-Min	C-Min	None	None	None	None
Ad Effct Green (s)	47.6	47.6	47.6	18.4	18.4	18.4	18.4
Actuated g/C Ratio	0.63	0.63	0.63	0.24	0.24	0.24	0.24
w/C Ratio	0.23	0.23	0.23	0.23	0.23	0.23	0.23
Control Delay	5.0	6.1	6.1	13.2	17.1	17.1	17.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	5.0	6.1	6.1	13.2	17.1	17.1	17.1
LOS	A	A	A	B	B	B	B
Approach Delay	5.0	6.1	6.1	13.2	17.1	17.1	17.1
Approach LOS	A	A	A	B	B	B	B
Intersection Summary	Cycle Length: 76						
Cycle Length: 76	Actuated Cycle Length: 76						
Offset: 25 (33%), Referenced to phase 2EBT and 6WBTL Start of 1st Green	Natural Cycle: 50						
Control Type: Actuated-Coordinated	Maximum v/c Ratio: 0.28						
Intersection Signal Delay: 7.4	Intersection LOS: A						
Intersection Capacity Utilization 48.0%	ICU Level of Service A						
Analysis Period (min) 15							



Queues  
22: Sackville Street & Dundas Street E  
Existing PM Model  
10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	462	253	75	108
Lane Group Flow (vph)	0.23	0.24	0.23	0.28
v/c Ratio	5.0	6.1	13.2	17.1
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	5.0	6.1	13.2	17.1
Total Delay	10.3	15.7	3.4	7.8
Queue Length 50th (m)	13.5	15.3	12.9	19.6
Queue Length 95th (m)	99.8	79.6	36.2	126.8
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2046	1061	369	434
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.23	0.24	0.20	0.25
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
22: Sackville Street & Dundas Street E  
Existing PM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4	4					4					
Traffic Volume (vph)	0	400	30	15	220	0	40	0	30	30	35	35	
Future Volume (vph)	0	400	30	15	220	0	40	0	30	30	35	35	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Lane Util. Factor	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frbp. ped/bikes	0.97	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.97	0.97	0.97	
Frbp. psd/bikes	1.00	1.00	0.99	1.00	0.99	0.96	0.96	0.96	0.96	0.97	0.97	0.97	
Frt	0.99	1.00	1.00	1.00	0.94	0.94	0.94	0.94	0.94	0.95	0.95	0.95	
Flt Protected	0.99	1.00	1.00	1.00	0.94	0.94	0.94	0.94	0.94	0.95	0.95	0.95	
Satd. Flow (prot)	3265	1750	1750	1750	1494	1494	1494	1494	1494	1618	1618	1618	
Flt Permitted	1.00	0.97	0.97	1.00	0.80	0.80	0.80	0.80	0.80	0.90	0.90	0.90	
Satd. Flow (perm)	3265	1694	1694	1226	1226	1226	1226	1226	1226	1478	1478	1478	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Adj. Flow (vph)	0	430	32	16	237	0	43	0	32	32	38	38	
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Group Flow (vph)	0	462	0	0	253	0	42	0	42	0	81	0	
Conf. Peds. (#/hr)	105	175	175	105	70	105	70	105	55	55	70	70	
Conf. Bikes (#/hr)	35	35	35	20	20	20	20	20	20	20	20	20	
Heavy Vehicles (%)	0%	5%	6%	0%	6%	0%	0%	0%	12%	0%	2%	5%	
Turn Type	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	
Protected Phases	2	6	6	6	6	8	8	8	8	4	4	4	
Permitted Phases													
Actuated Green, G (s)	46.6	46.6	46.6	46.6	46.6	17.4	17.4	17.4	17.4	17.4	17.4	17.4	
Effective Green, g (s)	47.6	47.6	47.6	47.6	47.6	18.4	18.4	18.4	18.4	18.4	18.4	18.4	
Actuated g/C Ratio	0.63	0.63	0.63	0.63	0.63	0.24	0.24	0.24	0.24	0.24	0.24	0.24	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	2044	1060	1060	296	296	357	357	357	357	357	357	357	
v/s Ratio Prot	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14	
v/s Ratio Perm	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	0.23	
Uniform Delay, d1	6.2	6.2	6.2	6.2	6.2	22.6	22.6	22.6	22.6	23.1	23.1	23.1	
Progression Factor	0.68	0.68	0.68	0.68	0.68	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.3	0.3	0.3	
Delay (s)	4.5	4.5	4.5	4.5	4.5	22.8	22.8	22.8	22.8	23.4	23.4	23.4	
Level of Service	A	A	A	A	A	C	C	C	C	C	C	C	
Approach Delay (s)	4.5	4.5	4.5	4.5	4.5	22.8	22.8	22.8	22.8	23.4	23.4	23.4	
Approach LOS	A	A	A	A	A	C	C	C	C	C	C	C	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	8.5											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.23												
Actuated Cycle Length (s)	76.0											Sum of lost time (s)	10.0
Intersection Capacity Utilization	48.0%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

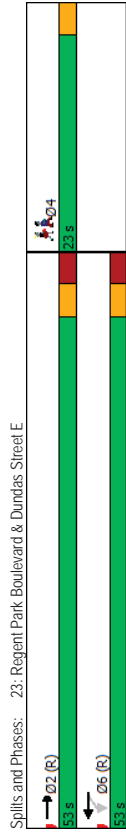
Timings  
23: Regent Park Boulevard & Dundas Street E

Queues  
23: Regent Park Boulevard & Dundas Street E

Existing PM Model  
10-25-2022

Existing PM Model  
10-25-2022

Lane Group	EBT	WBL	WBT	Ø4
Lane Configurations	4	15	235	4
Traffic Volume (vph)	430	15	235	
Future Volume (vph)	430	15	235	
Turn Type	NA	Perm	NA	
Protected Phases	2	6	6	4
Permitted Phases		6	6	
Detector Phase	2	6	6	
Switch Phase				
Minimum Initial (s)	18.0	18.0	18.0	20.0
Minimum Split (s)	23.9	23.9	23.9	23.0
Total Split (s)	53.0	53.0	53.0	23.0
Total Split (%)	69.7%	69.7%	69.7%	30%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.9	4.9	4.9	
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	C-Min	C-Min	C-Min	None
Act Effct Green (s)	64.8	64.8	64.8	
Actuated g/C Ratio	0.85	0.85	0.85	
v/c Ratio	0.36	0.19	0.19	
Control Delay	2.5	3.9	3.9	
Queue Delay	0.0	0.0	0.0	
Total Delay	2.5	3.9	3.9	
LOS	A	A	A	
Approach Delay	2.5	3.9	3.9	
Approach LOS	A	A	A	
<b>Intersection Summary</b>				
Cycle Length: 76				
Actuated Cycle Length: 76				
Offset: 22 (29%), Referenced to phase 2EBT and 6WBT, Start of 1st Green				
Natural Cycle: 50				
Control Type: Actuated-Coordinated				
Maximum v/c Ratio: 0.36				
Intersection Signal Delay: 3.0				Intersection LOS: A
Intersection Capacity Utilization 44.6%				ICU Level of Service A
Analysis Period (min) 15				



Lane Group	EBT	WBT
Lane Group Flow (vph)	517	281
v/c Ratio	0.36	0.19
Control Delay	2.5	3.9
Queue Delay	0.0	0.0
Total Delay	2.5	3.9
Queue Length 50th (m)	0.1	0.0
Queue Length 95th (m)	12.9	25.8
Internal Link Dist (m)	79.6	86.0
Turn Bay Length (m)		
Base Capacity (vph)	1450	1448
Sanctuary Cap Reductn	27	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.36	0.19
<b>Intersection Summary</b>		



23: Regent Park Boulevard & Dundas Street E

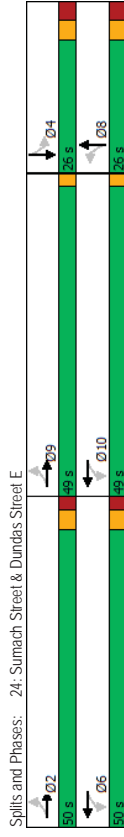
Existing PM Model  
10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4	4	4	4	4	4
Traffic Volume (vph)	430	30	15	235	0	0
Future Volume (vph)	430	30	15	235	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.0	3.0	3.5	3.0	3.0
Total Lost time (s)	4.9		4.9			
Lane Util. Factor	1.00		1.00			
Frbp. ped/bikes	0.97		1.00			
Frbp. ped/bikes	1.00		0.99			
Frt	0.99		1.00			
Flt Protected	1.00		1.00			
Sat'd Flow (prot)	1698		1748			
Flt Permitted	1.00		0.97			
Sat'd Flow (perm)	1698		1694			
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	483	34	17	264	0	0
RTOR Reduction (vph)	2	0	0	0	0	0
Lane Group Flow (vph)	515	0	0	281	0	0
Conf. Peds. (#/hr)	235	235	60	65		
Heavy Vehicles (%)	6%	6%	0%	6%	0%	0%
Turn Types	NA	Perm	NA			
Protected Phases	2		6			
Permitted Phases	6		6			
Actuated Green, G (s)	59.1		59.1			
Effective Green, g (s)	60.1		60.1			
Actuated g/C Ratio	0.79		0.79			
Clearance Time (s)	5.9		5.9			
Vehicle Extension (s)	3.0		3.0			
Lane Grp Cap. (vph)	1342		1339			
v/s Ratio Prot	0.30		0.17			
v/s Ratio Perm	0.38		0.21			
Uniform Delay, d1	2.4		2.0			
Progression Factor	0.43		1.00			
Incremental Delay, d2	0.8		0.4			
Delay (s)	1.9		2.4			
Level of Service	A		A			
Approach Delay (s)	1.9		2.4	0.0		
Approach LOS	A		A	A		
Intersection Summary						
HCM 2000 Control Delay		2.0		HCM 2000 Level of Service		A
HCM 2000 Volume to Capacity ratio		0.34				
Actuated Cycle Length (s)		76.0		Sum of lost time (s)		7.9
Intersection Capacity Utilization		44.6%		ICU Level of Service		A
Analysis Period (min)		15				
c. Critical Lane Group						

24: Sumach Street & Dundas Street E

Existing PM Model  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4	4	4	4	4	4	4	4
Traffic Volume (vph)	35	380	25	215	25	90	15	20
Future Volume (vph)	35	380	25	215	25	90	15	20
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2, 9		6, 10		8		4	
Permitted Phases	2		6		8		4	
Switch Phase								
Minimum Initial (s)		20.0		20.0		20.0		20.0
Minimum Split (s)		26.0		26.0		26.0		21.0
Total Split (s)		26.0		26.0		26.0		50.0
Total Split (%)		20.8%		20.8%		20.8%		40%
Yellow Time (s)		3.0		3.0		3.0		3.0
All-Red Time (s)		3.0		3.0		3.0		2.0
Lost Time Adjust (s)		-1.0		-1.0		-1.0		-1.0
Total Lost Time (s)		5.0		5.0		5.0		5.0
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode		None		None		None		None
Act Effct Green (s)	58.1		58.1		58.1		22.6	
Actuated g/C Ratio	0.64		0.64		0.64		0.25	
v/c Ratio	0.45		0.28		0.45		0.14	
Control Delay	8.3		6.4		37.1		29.2	
Queue Delay	1.3		0.0		0.0		0.0	
Total Delay	9.5		6.4		37.1		29.2	
LOS	A		A		D		C	
Approach Delay	9.5		6.4		37.1		29.2	
Approach LOS	A		A		D		C	
Intersection Summary								
Cycle Length		125						
Actuated Cycle Length		90.4						
Natural Cycle		100						
Control Type		Actuated-Uncoordinated						
Maximum v/c Ratio		0.45						
Intersection Signal Delay		14.5						
Intersection Capacity Utilization		53.3%						
Analysis Period (min)		15						



Queues  
24: Sumach Street & Dundas Street E

Existing PM Model  
10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	463	280	172	49
Lane Group Flow (vph)	0.45	0.28	0.45	0.14
v/c Ratio	8.3	6.4	37.1	29.2
Control Delay	1.3	0.0	0.0	0.0
Queue Delay	9.5	6.4	37.1	29.2
Total Delay	32.6	16.6	26.9	6.0
Queue Length 50th (m)	48.5	26.3	51.5	16.8
Queue Length 95th (m)	86.0	75.5	76.2	121.5
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1196	1136	380	356
Station Cap Reductn	503	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.67	0.25	0.45	0.14
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
24: Sumach Street & Dundas Street E

Existing PM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		+			+			+			+		
Traffic Volume (vph)	35	380	15	25	215	20	25	90	45	15	20	10	
Future Volume (vph)	35	380	15	25	215	20	25	90	45	15	20	10	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	4.0			4.0							5.0		
Lane Util. Factor	1.00			1.00				1.00			1.00		
Frpb, ped/bikes	0.98			0.98				0.96			0.94		
Flpb, ped/bikes	0.99			0.98				0.97			0.98		
Flt	1.00			0.99				0.96			0.97		
Flt Protected	1.00			1.00				0.99			0.98		
Satd. Flow (prot)	1691			1639				1587			1586		
Flt Permitted	0.96			0.94				0.95			0.89		
Satd. Flow (perm)	1628			1550				1522			1433		
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Adj. Flow (vph)	38	409	16	27	231	22	27	97	48	16	22	11	
RTOR Reduction (vph)	0	1	0	0	3	0	0	10	0	0	8	0	
Lane Group Flow (vph)	0	462	0	0	277	0	0	162	0	0	41	0	
Confl. Peds. (#/hr)	100		185	185		100	105		50	50		105	
Confl. Bikes (#/hr)			25			20							
Heavy Vehicles (%)	12%	6%	8%	0%	9%	5%	4%	5%	6%	0%	9%	0%	
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	2.9			6.10			8				4		
Permitted Phases	2.9			6.10			8				4		
Actuated Green, G (s)	60.7			60.7			21.5				21.5		
Effective Green, g (s)	61.7			61.7			22.5				22.5		
Actuated g/C Ratio	0.68			0.68			0.25				0.25		
Clearance Time (s)							6.0				6.0		
Vehicle Extension (s)							3.0				3.0		
Lane Grp Cap (vph)	1113			1060			379				357		
v/s Ratio Prot													
v/s Ratio Perm	60.28			0.18			60.11				0.03		
v/c Ratio	0.41			0.26			0.43				0.12		
Uniform Delay, d1	6.3			5.5			28.4				26.2		
Progression Factor	1.00			1.00			1.00				1.00		
Incremental Delay, d2	0.3			0.1			0.8				0.1		
Delay (s)	6.5			5.6			29.2				26.3		
Level of Service	A			A			C				C		
Approach Delay (s)	6.5			5.6			29.2				26.3		
Approach LOS	A			A			C				C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay	11.3											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.43												
Actuated Cycle Length (s)	90.2											Sum of lost time (s)	10.0
Intersection Capacity Utilization	53.3%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

25: Tubman Avenue & Dundas Street E

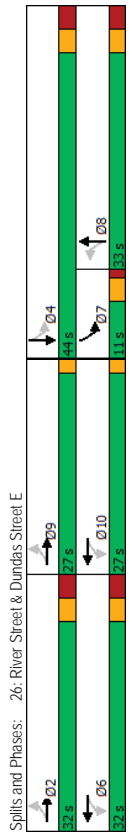
Existing PM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	430	10	10	255	0	0	0	0	5	0	5
Future Volume (Veh/h)	0	430	10	10	255	0	0	0	0	5	0	5
Sign Control	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	453	11	11	268	0	0	0	0	5	0	5
Pedestrians	10			10			130			50		
Lane Width (m)	3.5			3.5			0.0			3.5		
Walking Speed (m/s)	1.1			1.1			1.1			1.1		
Percent Blockage	1			1			0			4		
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)	99			98								
pX platoon unblocked												
VC, conflicting volume	318			594			894			928		934
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	318			496			828			867		873
IC, single (s)	4.1			4.1			7.1			6.2		6.5
IC, 2 stage (s)												
IF (s)	2.2			2.2			3.5			3.3		4.0
p0 queue free %	100			99			100			100		99
CM capacity (veh/h)	1198			973			249			514		277
Direction, Lane #	EB 1	WB 1	SB 1									
Volumes Total	464	279	10									
Volume Left	0	11	5									
Volume Right	11	0	5									
cSH	1198	973	394									
Volumes to Capacity	0.00	0.01	0.03									
Queue Length 95th (m)	0.0	0.3	0.6									
Control Delay (s)	0.0	0.5	14.4									
Lane LOS	A	A	B									
Approach Delay (s)	0.0	0.5	14.4									
Approach LOS	B	B	B									
Intersection Summary												
Average Delay				0.4								
Intersection Capacity Utilization				37.9%			ICU Level of Service			A		
Analysis Period (min)				15								

26: River Street & Dundas Street E

Existing PM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	10	405	65	195	15	310	100	100	365			
Future Volume (vph)	10	405	65	195	15	310	100	100	365			
Turn Type	Perm	NA	Perm	NA	Perm	NA	pm+pt	NA				
Protected Phases	2 9			6 10			8	7	4	2	6	9
Permitted Phases	2			6			8	8	7	4		
Detector Phase												
Switch Phase												
Minimum Initial (s)	21.0			21.0			6.0			21.0		5.0
Minimum Split (s)	27.0			27.0			10.0			27.0		27.0
Total Split (s)	33.0			33.0			11.0			44.0		32.0
Total Split (%)	32.0%			32.0%			10.7%			42.7%		31%
Yellow Time (s)	3.0			3.0			3.0			3.0		2.0
All-Red Time (s)	3.0			3.0			3.0			3.0		0.0
Lost Time Adjust (s)	-1.0			-1.0			-1.0			-1.0		-1.0
Total Lost Time (s)	5.0			5.0			5.0			5.0		5.0
Lead-Lag	Lag			Lag			Lag			Lag		Lag
Lead-Lag Optimize?	Yes			Yes			Yes			Yes		Yes
Recall Mode	Min			Min			Min			Min		Min
Act Effct Green (s)	29.3			29.3			28.9			29.4		37.4
Actuated g/C Ratio	0.38			0.38			0.38			0.51		0.49
v/c Ratio	0.69			0.66			0.05			0.34		0.56
Control Delay	25.6			24.6			22.5			40.2		15.9
Queue Delay	0.0			0.0			0.0			0.0		0.0
Total Delay	25.6			24.6			22.5			40.2		15.9
LOS	C			C			C			D		B
Approach Delay	25.6			24.6			39.7			18.8		18.8
Approach LOS	C			C			D			B		B
Intersection Summary												
Cycle Length	103											
Actuated Cycle Length	77											
Natural Cycle	95											
Control Type	Actuated-Uncoordinated											
Maximum v/c Ratio	0.86											
Intersection Signal Delay	27.5											
Intersection Capacity Utilization	100.4%											
Analysis Period (min)	15											



Queues  
26: River Street & Dundas Street E

Existing PM Model  
10-25-2022

	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group	458	357	16	531	105	442
Lane Group Flow (vph)	0.69	0.66	0.05	0.86	0.34	0.56
v/c Ratio	25.6	24.6	22.5	40.2	15.9	19.5
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	25.6	24.6	22.5	40.2	15.9	19.5
Total Delay	55.9	40.5	1.4	64.6	6.7	37.7
Queue Length 50th (m)	82.4	64.5	7.8	#186.2	24.7	109.1
Queue Length 95th (m)	73.4	80.5		131.0		118.9
Internal Link Dist (m)			20.0		25.0	
Turn Bay Length (m)						
Base Capacity (vph)	723	587	292	616	308	851
Stavation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.63	0.61	0.05	0.86	0.34	0.52

**Intersection Summary**  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
26: River Street & Dundas Street E

Existing PM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	10	405	20	65	195	80	15	310	195	100	365	55
Future Volume (vph)	10	405	20	65	195	80	15	310	195	100	365	55
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.99	0.99	0.99	0.97	0.97	0.97	1.00	0.94	1.00	0.99	1.00	0.98
Frbp. psd/bikes	1.00	1.00	1.00	0.99	0.99	0.99	1.00	0.94	1.00	0.99	1.00	0.98
Frt	1.00	1.00	1.00	0.97	0.97	0.97	1.00	0.94	1.00	0.95	1.00	0.98
Flt Protected	1.00	1.00	1.00	0.99	0.99	0.99	1.00	0.94	1.00	0.95	1.00	0.98
Satd. Flow (prot)	1762	1638	1638	1482	1482	1482	1626	1626	1643	1643	1633	1633
Flt Permitted	0.99	0.99	0.99	0.85	0.85	0.85	0.51	1.00	0.19	0.19	1.00	1.00
Satd. Flow (perm)	1745	1745	1745	1402	1402	1402	790	1626	324	324	1633	1633
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	11	426	21	68	205	84	16	326	205	105	384	58
RTOR Reduction (vph)	0	2	0	0	13	0	0	19	0	0	4	0
Lane Group Flow (vph)	0	456	0	0	344	0	16	512	0	105	438	0
Confl. Peds. (#/hr)	55	75	75	75	75	55	75	75	75	75	75	5
Confl. Bikes (#/hr)			25			20			10			5
Heavy Vehicles (%)	9%	5%	0%	1%	9%	0%	5%	3%	0%	2%	12%	0%
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA	NA	pm+pl	NA	NA	NA
Protected Phases	2.9			6.10					7			4
Permitted Phases	2.9			6.10			8		8			4
Actuated Green, G (s)	33.9			33.9			27.8		27.8			37.2
Effective Green, g (s)	34.9			34.9			28.8		28.8			38.2
Actuated g/C Ratio	0.44			0.44			0.36		0.36			0.48
Clearance Time (s)							6.0		6.0			4.0
Vehicle Extension (s)							3.0		3.0			3.0
Lane Grp Cap (vph)	769			618			287		592			788
v/s Ratio Prot							c0.31		c0.27			c0.27
v/s Ratio Perm	c0.26			0.25			0.02		0.16			0.16
v/c Ratio	0.59			0.56			0.06		0.40			0.56
Uniform Delay, d1	16.7			16.4			16.3		13.9			14.5
Progression Factor	1.00			1.00			1.00		1.00			1.00
Incremental Delay, d2	1.2			1.1			0.1		12.5			1.0
Delay (s)	18.0			17.5			16.4		35.9			15.3
Level of Service	B			B			B		D			B
Approach Delay (s)	18.0			17.5			35.3		15.2			15.2
Approach LOS	B			B			D		D			B
Intersection Summary												
HCM 2000 Control Delay	22.0 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.75											
Actuated Cycle Length (s)	79.1 Sum of lost time (s) 14.0											
Intersection Capacity Utilization	100.4% ICU Level of Service G											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 27: Dreamer's Way & Gerrard Street E

HCM Unsignalized Intersection Capacity Analysis  
 29: Sumach Street & Site Driveway

Existing PM Model  
 10-25-2022

Existing PM Model  
 10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		
Traffic Volume (veh/h)	830	0	0	410	0	0
Future Volume (Veh/h)	830	0	0	410	0	0
Sign Control	Free	Stop	Free	Stop	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	874	0	0	432	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None			None		None
Median type						
Median storage (veh)	90			162		
Upstream signal (m)	0.87			0.87		0.87
pX platoon unblocked	874			1090		437
VC, conflicting volume						
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	570			817		70
IC, single (s)	4.1			6.8		6.9
IC, 2 stage (s)	2.2			3.5		3.3
p0 queue free %	100			100		100
ICM capacity (veh/h)	886			279		861
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volumes Total	583	291	144	288		
Volume Left	0	0	0	0		
Volume Right	0	0	0	0		
cSH	1700	1700	886	1700		
Volumes to Capacity	0.34	0.17	0.00	0.17		
Queue Length 95th (m)	0.0	0.0	0.0	0.0		
Control Delay (s)	0.0	0.0	0.0	0.0		
Lane LOS						
Approach Delay (s)	0.0		0.0			
Approach LOS			A			
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization		26.3%				ICU Level of Service
Analysis Period (min)		15				A

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑↑					↑↑
Traffic Volume (veh/h)	10	5	85	0	0	30
Future Volume (Veh/h)	10	5	85	0	0	30
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	5	89	0	0	32
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None		None			None
Median type						
Median storage (veh)						
Upstream signal (m)						55
pX platoon unblocked						
VC, conflicting volume	121		89			89
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	121		89			89
IC, single (s)	6.4		6.2			4.1
IC, 2 stage (s)	3.5		3.3			2.2
p0 queue free %	99		99			100
ICM capacity (veh/h)	879		975			1519
Direction, Lane #	WB 1	NB 1	SB 1			
Volumes Total	16	89	32			
Volume Left	11	0	0			
Volume Right	5	0	0			
cSH	907	1700	1519			
Volumes to Capacity	0.02	0.05	0.00			
Queue Length 95th (m)	0.4	0.0	0.0			
Control Delay (s)	9.0	0.0	0.0			
Lane LOS	A					
Approach Delay (s)	9.0	0.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization			14.5%			ICU Level of Service
Analysis Period (min)			15			A

1: Parliament Street & Gerrard Street E (North Section)

2032 Future Background AM Model (ExRN)

10-25-2022

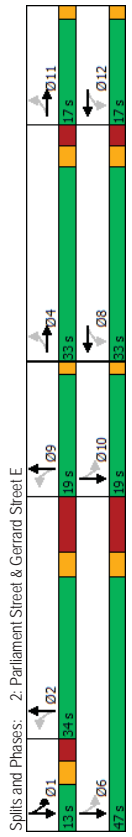
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				4TB	4TB	
Traffic Volume (veh/h)	0	0	5	550	500	60
Future Volume (Veh/h)	0	0	5	550	500	60
Sign Control	Free					
Grade	0%					
Peak Hour Factor	0.92					
Hourly flow rate (vph)	0	0	5	598	543	65
Pedestrians	80					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.1					
Percent Blockage	0					
Right turn flare (veh)	None					
Median type	None					
Median storage (veh)	None					
Upstream signal (m)	39					
dx platoon unblocked	0.93					
VC, conflicting volume	970					
VC1, stage 1 conf vol	384					
VC2, stage 2 conf vol	688					
VCU, unblocked vol	822					
IC, single (s)	6.8					
IC, 2 stage (s)	3.5					
p0 queue free %	100					
CM capacity (veh/h)	292					
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	204	399	362	246		
Volume Left	5	0	0	0		
Volume Right	0	0	0	65		
cSH	791	1700	1700	1700		
Volumes to Capacity	0.01	0.23	0.21	0.14		
Queue Length 95th (m)	0.1	0.0	0.0	0.0		
Control Delay (s)	0.3	0.0	0.0	0.0		
Lane LOS	A					
Approach Delay (s)	0.1					
Approach LOS	C					
<b>Intersection Summary</b>						
Average Delay	0.1					
Intersection Capacity Utilization	22.0%					
Analysis Period (min)	15					
ICU Level of Service	A					

2: Parliament Street & Gerrard Street E

2032 Future Background AM Model (ExRN)

10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		4TB		4TB		4TB		4TB
Traffic Volume (vph)	25	245	65	520	80	330	80	350
Future Volume (vph)	25	245	65	520	80	330	80	350
Turn Type	Perm	NA	Perm	NA	Perm	NA	custom	NA
Protected Phases	4 11							
Permitted Phases	4 8 12							
Detector Phase	4 8 8 2 2 1 1 6							
Switch Phase	4 4 4 8 8 2 2 1 1 6							
Minimum Initial (s)	6.0							
Minimum Split (s)	12.5							
Total Split (s)	13.0							
Total Split (%)	11.2%							
Yellow Time (s)	3.3							
All-Red Time (s)	3.2							
Lost Time Adjust (s)								
Total Lost Time (s)								
Lead/Lag	Lead Lag							
Recall Mode	Yes Yes							
Ad Effct Green (s)	33.1							
Actuated g/C Ratio	0.36							
w/C Ratio	0.34							
Control Delay	21.0							
Queue Delay	0.0							
Total Delay	21.0							
LOS	C							
Approach Delay	21.0							
Approach LOS	C							
<b>Intersection Summary</b>								
Cycle Length	116							
Actuated Cycle Length	92.5							
Natural Cycle	115							
Control Type	Actuated-Uncoordinated							
Maximum w/C Ratio	0.80							
Intersection Signal Delay	25.3							
Intersection Capacity Utilization	99.9%							
Analysis Period (min)	15							



Timings  
2: Parliament Street & Gerrard Street E

2032 Future Background AM Model (ExRN)  
10-25-2022

Lane Group	Ø9	Ø10	Ø11	Ø12
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Turn Type	9	10	11	12
Protected Phases				
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0	5.0	5.0	5.0
Minimum Split (s)	19.0	19.0	17.0	17.0
Total Split (s)	19.0	19.0	17.0	17.0
Total Split (%)	16%	16%	15%	15%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	None	None
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Intersection Summary				

Queues  
2: Parliament Street & Gerrard Street E

2032 Future Background AM Model (ExRN)  
10-25-2022



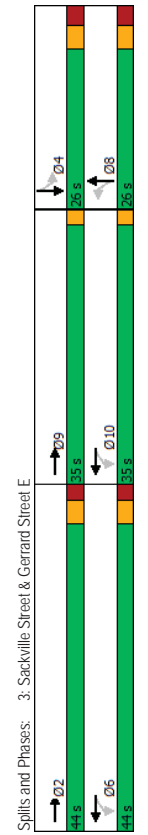
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	358	826	484	526
v/c Ratio	0.34	0.80	0.62	0.37
Control Delay	21.0	32.0	29.8	13.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	21.0	32.0	29.8	13.4
Queue Length 50th (m)	18.3	54.6	32.9	21.4
Queue Length 95th (m)	38.2	102.6	60.8	43.9
Internal Link Dist (m)	33.0	63.9	119.2	15.0
Turn Bay Length (m)				
Base Capacity (vph)	1055	1078	798	1445
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.34	0.77	0.61	0.36
Intersection Summary				

2. Parliament Street & Gerrard Street E 2032 Future Background AM Model (ExRN) 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB			4TB			4TB		4TB
Traffic Volume (vph)	25	245	70	65	520	200	80	330	50	80	350	70
Future Volume (vph)	25	245	70	65	520	200	80	330	50	80	350	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.8	4.8		4.8		10.2					5.5	
Lane Util. Factor	0.95	0.95		0.95		0.95					0.95	
Fpb. ped/bikes	0.98	0.97		0.97		0.98					0.99	
Fpb. ped/bikes	1.00	1.00		1.00		0.99					0.99	
Frt	0.97	0.96		0.96		0.98					0.98	
Frt Protected	1.00	1.00		1.00		0.99					0.99	
Sat'd. Flow (prot)	3321	3197		3197		3171					3203	
Flt Permitted	0.86	0.88		0.88		0.74					0.81	
Sat'd. Flow (perm)	2877	2820		2820		2381					2604	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	26	258	74	68	547	211	84	347	53	84	368	74
RTOR Reduction (vph)	0	20	0	0	29	0	0	7	0	0	13	0
Lane Group Flow (vph)	0	338	0	0	797	0	0	477	0	0	513	0
Conf. Peds. (#/hr)	80	85	85	80	85	80	85	120	120	120	85	85
Conf. Bikes (#/hr)			5		20				5		10	
Heavy Vehicles (%)	13%	0%	2%	1%	1%	11%	6%	8%	0%	11%	6%	2%
Turn Type	Perim	MA	Perm	MA	Perm	NA	Perm	NA	Perm	MA	Perm	MA
Protected Phases	4 11		8 12		8 12		2 9		2 9	1 6 10		6 10
Permitted Phases	4 11		8 12		8 12		2 9		2 9	6 10		6 10
Actuated Green, G (s)	37.6		37.6		37.6		40.3		40.3	53.6		53.6
Effective Green, g (s)	38.6		38.6		38.6		41.3		41.3	44.4		44.4
Actuated g/C Ratio	0.41		0.41		0.41		0.43		0.43	0.47		0.47
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1166		1143		1143		1032		1032	1263		1263
v/s Ratio Prot			c0.28		c0.28		60.20		60.20	c0.03		c0.03
v/s Ratio Perm			0.29		0.70		0.46		0.46	0.16		0.16
v/c Ratio			19.1		23.5		19.1		19.1	16.7		16.7
Uniform Delay, d1			1.00		1.00		1.00		1.00	1.00		1.00
Progression Factor			0.1		1.9		0.3		0.3	0.2		0.2
Incremental Delay, d2			19.2		25.3		19.4		19.4	16.9		16.9
Delay (s)			B		C		B		B	B		B
Level of Service			B		C		B		B	B		B
Approach Delay (s)			19.2		25.3		19.4		19.4	16.9		16.9
Approach LOS			B		C		B		B	B		B
Intersection Summary												
HCM 2000 Control Delay	21.0 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.67											
Actuated Cycle Length (s)	95.2 Sum of lost time (s) 22.5											
Intersection Capacity Utilization	99.9% ICU Level of Service F											
Analysis Period (min)	15											
c Critical Lane Group												

3. Sackville Street & Gerrard Street E 2032 Future Background AM Model (ExRN) 10-25-2022

Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø6	Ø9	Ø10
Lane Configurations	4TB		4TB		4TB		4TB			
Traffic Volume (vph)	345	55	830	5	0	55	35			
Future Volume (vph)	345	55	830	5	0	55	35			
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA			
Protected Phases	2 9		6 10		8		4		2	6
Permitted Phases	2 9		6 10		8		4		2	6
Detector Phase					8		4			
Switch Phase					8		4			
Minimum Initial (s)				20.0	20.0		20.0	16.0	16.0	5.0
Minimum Split (s)				25.7	25.7		25.7	21.0	21.0	35.0
Total Split (s)				26.0	26.0		26.0	44.0	44.0	35.0
Total Split (%)				24.8%	24.8%		24.8%	42%	42%	33%
Yellow Time (s)				3.0	3.0		3.0	3.0	3.0	2.0
All-Red Time (s)				2.7	2.7		2.7	2.0	2.0	0.0
Lost Time Adjust (s)				-1.0	-1.0		-1.0			
Total Lost Time (s)				4.7	4.7		4.7			
Lead-Lag										
Lead-Lag Optimize?										
Recall Mode				Min	Min	Min	Min	Min	Min	None
Act Effect Green (s)	44.7		44.7		22.9		22.9			
Actuated g/C Ratio	0.58		0.58		0.30		0.30			
v/c Ratio	0.21		0.55		0.07		0.29			
Control Delay	6.5		9.7		6.2		30.1			
Queue Delay	0.0		0.0		0.0		0.0			
Total Delay	6.5		9.7		6.2		30.1			
LOS	A		A		A		C			
Approach Delay	6.5		9.7		6.2		30.1			
Approach LOS	A		A		A		C			
Intersection Summary										
Cycle Length: 105										
Actuated Cycle Length: 77.1										
Natural Cycle: 85										
Control Type: Actuated-Uncoordinated										
Maximum v/c Ratio: 0.55										
Intersection Signal Delay: 10.4										
Intersection Capacity Utilization: 65.1%										
Analysis Period (min): 15										
ICU Level of Service: C										





Queues 2032 Future Background AM Model (ExRN) 10-25-2022

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN) 10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	395	972	27	114
Lane Group Flow (vph)	0.21	0.55	0.07	0.29
v/c Ratio	6.5	9.7	6.2	30.1
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	6.5	9.7	6.2	30.1
Total Delay	11.5	37.5	0.0	15.5
Queue Length 50th (m)	16.4	48.7	4.5	34.0
Queue Length 95th (m)	139.8	47.8	34.3	15.3
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2313	2207	417	404
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.17	0.44	0.06	0.28
Intersection Summary				

	EBT	EBR	WBL	WBT	NBL	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	0	345	15	55	830	0	5	0	20
Future Volume (vph)	0	345	15	55	830	0	5	0	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.5	3.0	3.0	3.5	3.0	3.5
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.99	0.99	1.00	1.00	0.86	0.86	0.99	0.99	0.99
Frbp. ped/bikes	1.00	0.99	1.00	0.99	0.99	0.99	0.92	0.92	0.92
Frt	0.99	0.99	1.00	1.00	0.99	0.99	0.97	0.97	0.97
Frt Protected	1.00	1.00	1.00	1.00	0.99	0.99	0.97	0.97	0.97
Sat'd. Flow (prot)	3215	3215	3394	3394	1410	1410	1683	1683	1683
Flt Permitted	1.00	1.00	0.90	0.90	0.96	0.96	0.85	0.85	0.85
Sat'd. Flow (perm)	3215	3215	3070	3070	1366	1366	1375	1375	1375
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	0	379	16	60	912	0	5	0	22
RTOR Reduction (vph)	0	4	0	0	0	0	0	0	5
Lane Group Flow (vph)	0	391	0	0	972	0	8	0	109
Confl. Peds. (#/hr)	40	60	60	60	40	60	140	140	60
Confl. Bikes (#/hr)	15	15	15	15	75	75	140	140	5
Heavy Vehicles (%)	0%	10%	0%	9%	4%	0%	0%	0%	6%
Turn Type	NA	NA	NA	NA	NA	NA	NA	NA	NA
Protected Phases	2.9	6.10	6.10	6.10	8	8	4	4	4
Permitted Phases									
Actuated Green, G (s)	48.0	48.0	48.0	48.0	21.8	21.8	21.8	21.8	21.8
Effective Green, g (s)	49.0	49.0	49.0	49.0	22.8	22.8	22.8	22.8	22.8
Actuated q/C Ratio	0.63	0.63	0.63	0.63	0.29	0.29	0.29	0.29	0.29
Clearance Time (s)					5.7	5.7	5.7	5.7	5.7
Vehicle Extension (s)					3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	2032	2032	1941	1941	401	401	404	404	404
v/s Ratio Prot	0.12	0.12	0.12	0.12	0.01	0.01	0.08	0.08	0.08
v/s Ratio Perm	0.19	0.19	0.50	0.50	0.02	0.02	0.27	0.27	0.27
Uniform Delay, d1	6.0	6.0	7.7	7.7	19.4	19.4	21.0	21.0	21.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.0	0.0	0.2	0.2	0.0	0.0	0.4	0.4	0.4
Delay (s)	6.0	6.0	7.9	7.9	19.4	19.4	21.3	21.3	21.3
Level of Service	A	A	A	A	B	B	C	C	C
Approach Delay (s)	6.0	6.0	7.9	7.9	19.4	19.4	21.3	21.3	21.3
Approach LOS	A	A	A	A	B	B	C	C	C
Intersection Summary									
HCM 2000 Control Delay	8.6		HCM 2000 Level of Service		A				
HCM 2000 Volume to Capacity ratio	0.44								
Actuated Cycle Length (s)	77.5								
Sum of lost time (s)	9.7								
Intersection Capacity Utilization	65.1%		ICU Level of Service		C				
Analysis Period (min)	15								
c Critical Lane Group									

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 4: Gerrard Street E & Gifford Street

10-25-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4↑	1↑		W	
Traffic Volume (veh/h)	5	415	880	20	5	5
Future Volume (Veh/h)	5	415	880	20	5	5
Sign Control		Free	Free	Stop	Stop	
Grade		0%	0%	0%	0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	5	432	917	21	5	5
Pedestrians		5	5		50	
Lane Width (m)		3.5	3.5	3.0		
Walking Speed (m/s)		1.1	1.1	1.1		
Percent Blockage		0	0		4	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)						
Upstream signal (m)		72	141			
PX platoon unblocked	0.93				0.94	0.93
VC conflicting volume	988				1208	524
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	825				963	323
IC single (s)	4.1				7.5	7.3
IC 2 stage (s)						
p0 queue free %	2.2				3.8	3.5
IF (s)	99				97	99
CM capacity (veh/h)	725				182	551
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 1
Volumes Total	149	288	611	327	10	10
Volume Left	5	0	0	0	5	5
Volume Right	0	0	0	21	5	
cSH	725	1700	1700	1700	274	
Volumes to Capacity	0.01	0.17	0.36	0.19	0.04	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.9	
Control Delay (s)	0.4	0.0	0.0	0.0	18.6	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		18.6	
Approach LOS					C	
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			36.6%		ICU Level of Service A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 5: Gerrard Street E & Nasmith Avenue

10-25-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4↑	1↑		W	
Traffic Volume (veh/h)	5	415	890	5	15	10
Future Volume (Veh/h)	5	415	890	5	15	10
Sign Control		Free	Free	Stop	Stop	
Grade		0%	0%	0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	5	428	918	5	15	10
Pedestrians					75	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					6	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)						
Upstream signal (m)		145	68			
PX platoon unblocked	0.90				0.90	0.90
VC conflicting volume	988				1220	536
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	769				1016	255
IC single (s)	4.1				6.9	7.1
IC 2 stage (s)						
p0 queue free %	99				3.6	3.4
IF (s)	723				92	98
CM capacity (veh/h)	723				190	610
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 1
Volumes Total	148	285	612	311	25	25
Volume Left	5	0	0	0	15	10
Volume Right	0	0	0	5	10	
cSH	723	1700	1700	1700	262	
Volumes to Capacity	0.01	0.17	0.36	0.18	0.10	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	2.4	
Control Delay (s)	0.4	0.0	0.0	0.0	20.2	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		20.2	
Approach LOS					C	
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			34.8%		ICU Level of Service A	
Analysis Period (min)			15			

Timings 2032 Future Background AM Model (ExRN) 10-25-2022

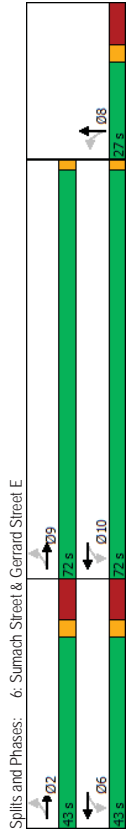
Queues 2032 Future Background AM Model (ExRN) 10-25-2022

	EBL	EBT	WBL	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Group	EBL	EBT	WBL	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	5	410	55	860	10				
Traffic Volume (vph)	5	410	55	860	10				
Future Volume (vph)	Perm	NA	Perm	NA	NA	2	6	9	10
Turn Type	2.9	NA	6.10	8	2	6	9	10	
Protected Phases	2.9	6.10							
Permitted Phases	2	2	6	6	8				
Detector Phase									
Switch Phase									
Minimum Initial (s)						15.0	11.0	5.0	5.0
Minimum Split (s)						25.4	26.2	72.0	72.0
Total Split (s)						27.0	43.0	72.0	72.0
Total Split (%)						19.0%	30%	51%	51%
Yellow Time (s)						3.0	3.0	2.0	2.0
All-Red Time (s)						7.4	7.2	7.2	0.0
Lost Time Adjust (s)						-1.0			
Total Lost Time (s)						9.4			
Lead/Lag									
Lead-Lag Optimize?						None	Min	Min	None
Recall Mode						None			
Act Effct Green (s)		73.1		73.1	18.4				
Actuated g/C Ratio		0.73		0.73	0.18				
v/c Ratio		0.22		0.57	0.26				
Control Delay		6.2		9.6	45.1				
Queue Delay		0.0		0.1	0.0				
Total Delay		6.2		9.7	45.1				
LOS		A		A	D				
Approach Delay		6.2		9.7	45.1				
Approach LOS		A		A	D				

	EBT	WBT	NBT
Lane Group	EBT	WBT	NBT
Lane Group Flow (vph)	494	1196	68
v/c Ratio	0.22	0.57	0.26
Control Delay	6.2	9.6	45.1
Queue Delay	0.0	0.1	0.0
Total Delay	6.2	9.7	45.1
Queue Length 50th (m)	18.3	63.6	7.4
Queue Length 95th (m)	24.0	76.9	29.4
Internal Link Dist (m)	44.0	75.7	33.2
Turn Bay Length (m)			
Base Capacity (vph)	2293	2086	287
Starvation Cap Reductn	0	176	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.22	0.63	0.24

	EBT	WBT	NBT
Lane Group	EBT	WBT	NBT
Lane Group Flow (vph)	494	1196	68
v/c Ratio	0.22	0.57	0.26
Control Delay	6.2	9.6	45.1
Queue Delay	0.0	0.1	0.0
Total Delay	6.2	9.7	45.1
Queue Length 50th (m)	18.3	63.6	7.4
Queue Length 95th (m)	24.0	76.9	29.4
Internal Link Dist (m)	44.0	75.7	33.2
Turn Bay Length (m)			
Base Capacity (vph)	2293	2086	287
Starvation Cap Reductn	0	176	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.22	0.63	0.24

	EBT	WBT	NBT
Lane Group	EBT	WBT	NBT
Lane Group Flow (vph)	494	1196	68
v/c Ratio	0.22	0.57	0.26
Control Delay	6.2	9.6	45.1
Queue Delay	0.0	0.1	0.0
Total Delay	6.2	9.7	45.1
Queue Length 50th (m)	18.3	63.6	7.4
Queue Length 95th (m)	24.0	76.9	29.4
Internal Link Dist (m)	44.0	75.7	33.2
Turn Bay Length (m)			
Base Capacity (vph)	2293	2086	287
Starvation Cap Reductn	0	176	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.22	0.63	0.24



6: Sumach Street & Gerrard Street E HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN) 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4T		4T			4					
Traffic Volume (vph)	5	410	15	55	860	125	35	10	15	0	0	0
Future Volume (vph)	5	410	15	55	860	125	35	10	15	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)		9.2		9.2			9.4					
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp ped/bikes	0.99	0.97	0.97	0.97	0.97	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Frbp, ped/bikes	1.00	0.99	0.99	0.99	0.99	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Frt	1.00	0.99	0.98	0.98	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Flt Protected		1.00		1.00			1.00					
Sat'd Flow (prot)	3852	3251		3251			1492					
Flt Permitted	0.94	0.88		0.88			0.97					
Sat'd Flow (perm)	3148	2882		2882			1492					
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	6	471	17	63	989	144	40	11	17	0	0	0
RTOR Reduction (vph)	0	2	0	7	0	0	9	0	0	0	0	0
Lane Group Flow (vph)	0	492	0	0	1189	0	0	59	0	0	0	0
Conf. Bikes (#/hr)	65	85	85	65	100	65	65	65	65	65	65	100
Heavy Vehicles (%)	0%	5%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perim	MA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases		2.9		6.10		8						
Permitted Phases		2.9		6.10		8						
Actuated Green, G (s)		78.6		78.6		11.8						
Effective Green, g (s)		79.6		79.6		12.8						
Actuated g/C Ratio		0.77		0.77		0.12						
Clearance Time (s)						10.4						
Vehicle Extension (s)						3.0						
Lane Grp Cap (vph)		2437		2231		185						
v/s Ratio Prot		0.16		0.41		0.04						
v/s Ratio Perm		0.20		0.53		0.32						
v/c Ratio		3.1		4.5		41.0						
Uniform Delay, d1		1.00		1.00		1.00						
Progression Factor		0.0		0.2		1.0						
Incremental Delay, d2		3.1		4.7		42.0						
Delay (s)		A		A		D						
Level of Service		A		A		D						
Approach Delay (s)		3.1		4.7		42.0						
Approach LOS		A		A		D						
Intersection Summary												
HCM 2000 Control Delay	5.7 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	0.55											
Actuated Cycle Length (s)	102.8 Sum of lost time (s)											
Intersection Capacity Utilization	78.9% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group												

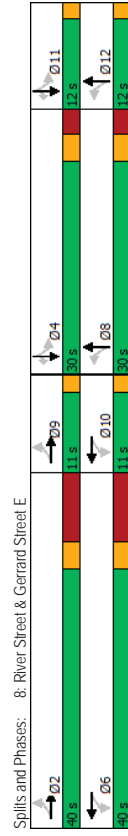
7: Gerrard Street E & Sword Street HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN) 10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4T		4T		
Traffic Volume (veh/h)	0	425	1035	0	10	5
Future Volume (Veh/h)	0	425	1035	0	10	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	447	1089	0	11	5
Pedestrians					75	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					6	
Right turn flare (veh)						
Median type		None		None		
Median storage (veh)						
Upstream signal (m)		100		91		
pK, platoon unblocked		0.80		0.80		0.80
vC, conflicting volume		1164		1388		620
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol		696		928		13
iC, single (s)		4.1		6.8		6.9
iC, 2 stage (s)						
IF (s)		2.2		3.5		3.3
p0 queue free %		100		95		99
dM capacity (veh/h)		683		205		804
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 1
Volumes Total	224	224	544	544	16	16
Volume Left	0	0	0	0	11	11
Volume Right	0	0	0	0	5	5
CSH	1700	1700	1700	1700	267	267
Volumes to Capacity	0.13	0.13	0.32	0.32	0.06	0.06
Queue Length 95th (m)	0.0	0.0	0.0	0.0	1.4	1.4
Control Delay (s)	0.0	0.0	0.0	0.0	19.3	19.3
Lane LOS					C	C
Approach Delay (s)	0.0	0.0	0.0	0.0	19.3	19.3
Approach LOS					C	C
Intersection Summary						
Average Delay	0.2					
Intersection Capacity Utilization	38.6%					
ICU Level of Service	A					
Analysis Period (min)	15					

Timings  
8: River Street & Gerrard Street E

2032 Future Background AM Model (ExRN)  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Lane Configurations	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB			
Traffic Volume (vph)	110	280	150	825	45	365	125	365	165			
Future Volume (vph)	110	280	150	825	45	365	125	365	165			
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm			
Protected Phases	2.9	6.10	6.10	8.12	4.11	4.11	4.11	4.11	4.11	2	4	6
Permitted Phases	2	2	6	6	8	8	4	4	4			
Detector Phase												
Switch Phase												
Minimum Initial (s)										23.0	19.0	23.0
Minimum Split (s)										33.8	30.0	33.8
Total Split (s)										40.0	30.0	40.0
Total Split (%)										43%	32%	43%
Yellow Time (s)										3.0	3.0	3.0
All-Red Time (s)										7.8	3.0	7.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode										Min	Min	Min
Act Effct Green (s)	36.8	36.8	29.9	29.9	29.9	29.9	29.9	29.9	29.9			
Actuated g/C Ratio	0.45	0.45	0.37	0.37	0.37	0.37	0.37	0.37	0.37			
v/C Ratio	0.59	1.01	0.19	0.70	0.63	0.56	0.29					
Control Delay	21.1	53.3	20.8	28.3	37.4	24.6	8.6					
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0					
Total Delay	21.1	53.3	20.8	28.3	37.4	24.6	8.6					
LOS	C	D	C	C	D	C	C	A				
Approach Delay	21.1	53.3	27.6	23.0								
Approach LOS	C	D	C	C								
Intersection Summary												
Cycle Length	93											
Actuated Cycle Length	81.9											
Natural Cycle	90											
Control Type	Actuated-Uncoordinated											
Maximum v/C Ratio	1.01											
Intersection Signal Delay	36.4											
Intersection Capacity Utilization	117.2%											
Analysis Period (min)	15											



Timings  
8: River Street & Gerrard Street E

2032 Future Background AM Model (ExRN)  
10-25-2022

Lane Group	Ø8	Ø9	Ø10	Ø11	Ø12
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Turn Type					
Protected Phases	8	9	10	11	12
Permitted Phases					
Detector Phase					
Switch Phase					
Minimum Initial (s)	19.0	5.0	5.0	5.0	5.0
Minimum Split (s)	30.0	11.0	11.0	12.0	12.0
Total Split (s)	30.0	11.0	11.0	12.0	12.0
Total Split (%)	32%	12%	12%	13%	13%
Yellow Time (s)	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)	3.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode					
Act Effct Green (s)	Min	None	None	None	None
Actuated g/C Ratio					
v/C Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Intersection Summary					

Queues 2032 Future Background AM Model (ExRN) 10-25-2022  
8: River Street & Gerrard Street E

	EBT	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group	454	1203	47	448	130	380	172
Lane Group Flow (vph)	0.59	1.01	0.19	0.70	0.63	0.56	0.29
v/c Ratio	21.1	53.3	20.8	28.3	37.4	24.6	8.6
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	21.1	53.3	20.8	28.3	37.4	24.6	8.6
Total Delay	27.3	99.5	5.4	61.8	17.8	50.4	6.3
Queue Length 50th (m)	46.6	#160.8	13.2	94.4	38.6	76.7	19.2
Queue Length 95th (m)	67.0	81.6		126.5		61.7	
Internal Link Dist (m)			30.0		30.0		
Turn Bay Length (m)	735	1188	252	670	216	708	609
Base Capacity (vph)	0	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.62	1.01	0.19	0.67	0.60	0.54	0.28
<b>Intersection Summary</b>							
#	95th percentile volume exceeds capacity, queue may be longer.						
	Queue shown is maximum after two cycles.						

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN) 10-25-2022  
8: River Street & Gerrard Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB			4TB			4TB		
Traffic Volume (vph)	110	280	45	150	825	180	45	365	65	125	365	165
Future Volume (vph)	110	280	45	150	825	180	45	365	65	125	365	165
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Frbp. psd/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Frt Protected	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Satd. Flow (prot)	3180	3180	3180	3180	3180	3180	3180	3180	3180	3180	3180	3180
Frt Permitted	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52
Satd. Flow (perm)	1684	1684	1684	1684	1684	1684	1684	1684	1684	1684	1684	1684
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	115	292	47	156	859	188	47	380	68	130	380	172
RTOR Reduction (vph)	0	8	0	13	0	0	0	7	0	0	0	67
Lane Group Flow (vph)	0	446	0	0	1190	0	47	441	0	130	380	105
Confl. Peds. (#/hr)	50	95	95	95	95	50	40	85	85	40	15	10
Heavy Vehicles (%)	4%	8%	4%	0%	3%	1%	8%	4%	2%	3%	1%	0%
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2.9	2.9	2.9	6.10	6.10	6.10	8.12	8.12	8.12	8.12	4.11	4.11
Permitted Phases	2.9	2.9	2.9	6.10	6.10	6.10	8.12	8.12	8.12	8.12	4.11	4.11
Actuated Green, G (s)	45.5	45.5	45.5	45.5	45.5	45.5	33.8	33.8	33.8	33.8	33.8	33.8
Effective Green, g (s)	46.5	46.5	46.5	46.5	46.5	46.5	34.8	34.8	34.8	34.8	34.8	34.8
Actuated g/C Ratio	0.56	0.56	0.56	0.56	0.56	0.56	0.42	0.42	0.42	0.42	0.42	0.42
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	940	940	940	1460	1460	1460	277	727	238	777	593	593
v/s Ratio Prot				c0.25	c0.25	c0.25						0.20
v/s Ratio Perm	0.27	0.27	0.27	c0.45	c0.45	c0.45	0.07	0.07	0.23	0.23	0.23	0.07
v/c Ratio	0.47	0.47	0.47	0.82	0.82	0.82	0.17	0.61	0.55	0.49	0.18	0.18
Uniform Delay, d1	11.1	11.1	11.1	14.9	14.9	14.9	15.2	15.2	18.3	17.7	15.2	15.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.4	0.4	3.6	3.6	3.6	0.3	1.4	2.6	0.5	0.1	0.1
Delay (s)	11.4	11.4	11.4	18.5	18.5	18.5	15.5	20.4	20.8	18.2	15.4	15.4
Level of Service	B	B	B	B	B	B	C	C	C	C	B	B
Approach Delay (s)	11.4	11.4	11.4	18.5	18.5	18.5	19.9	19.9	18.0	18.0	18.0	18.0
Approach LOS	B	B	B	B	B	B	B	B	B	B	B	B
<b>Intersection Summary</b>												
HCM 2000 Control Delay	17.5 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.85											
Actuated Cycle Length (s)	83.3											
Intersection Capacity Utilization	117.2% Sum of lost time (s) 16.8 H											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 9: Sackville Street & Site Driveway

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	5	5	5	5	5	5	0	15	5	10	90	5
Future Volume (Veh/h)	5	5	5	5	5	5	0	15	5	10	90	5
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	6	6	6	6	6	6	0	19	6	13	117	6
Pedestrians	70			40			15				10	
Lane Width (m)	3.5			3.5			3.5				3.5	
Walking Speed (m/s)	1.1			1.1			1.1				1.1	
Percent Blockage	6			4			1				1	
Right turn flare (veh)												
Median type							None					
Median storage (veh)												
Upstream signal (m)												58
PX platoon unblocked												
VC conflicting volume	257	281	205	232	281	72	193					65
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	257	281	205	232	281	72	193					65
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1					4.1
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2					2.2
p0 queue free %	99	99	99	99	99	99	100					99
CM capacity (veh/h)	591	566	778	628	566	952	1306					1495
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volumes Total	18	18	25	136								
Volume Left	6	6	0	13								
Volume Right	6	6	6	6								
cSH	632	680	1306	1495								
Volumes to Capacity	0.03	0.03	0.00	0.01								
Queue Length 95th (m)	0.7	0.6	0.0	0.2								
Control Delay (s)	10.9	10.4	0.0	0.8								
Lane LOS	B	B	A	A								
Approach Delay (s)	10.9	10.4	0.0	0.8								
Approach LOS	B	B	A	A								
Intersection Summary												
Average Delay			2.5									
Intersection Capacity Utilization			27.2%									A
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 10: Parliament Street & Oak Street

10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	45	65	395	10	15	470
Future Volume (Veh/h)	45	65	395	10	15	470
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	47	68	416	11	16	495
Pedestrians	260		5			175
Lane Width (m)	3.0		3.5			3.5
Walking Speed (m/s)	1.1		1.1			1.1
Percent Blockage	20		0			15
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			151			143
PX platoon unblocked	0.99	0.99			0.99	
VC conflicting volume	966	648			687	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	894	627			666	
IC, single (s)	6.8	7.0			4.1	
IC, 2 stage (s)						
IF (s)	3.5	3.3			2.2	
p0 queue free %	79	76			98	
CM capacity (veh/h)	219	282			742	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volumes Total	115	277	150	181	330	
Volume Left	47	0	0	16	0	
Volume Right	68	0	11	0	0	
cSH	252	1700	1700	742	1700	
Volumes to Capacity	0.46	0.16	0.09	0.02	0.19	
Queue Length 95th (m)	16.9	0.0	0.0	0.5	0.0	
Control Delay (s)	30.7	0.0	0.0	1.1	0.0	
Lane LOS	D	A	A	A	A	
Approach Delay (s)	30.7	0.0	0.0	0.4	0.0	
Approach LOS	D	A	A	A	A	
Intersection Summary						
Average Delay			3.5			
Intersection Capacity Utilization			43.8%			A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
10-25-2022  
11: Oak Street & Dreamer's Way

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Traffic Volume (veh/h)	0	25	65	0	5	50
Future Volume (Veh/h)	0	25	65	0	5	50
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Hourly flow rate (vph)	0	30	79	0	6	61
Pedestrians	15				40	
Lane Width (m)	3.5	3.5	3.0	3.0	3.0	3.0
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	1				3	
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume	119				149	134
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	119				149	134
IC single (s)	4.1				6.4	6.3
IC 2 stage (s)	2.2				3.5	3.4
p0 queue free %	100				99	93
CM capacity (veh/h)	1437				822	851
Direction_Lane #	EB 1	WB 1	SB 1			
Volumes Total	30	79	67			
Volume Left	0	0	6			
Volume Right	0	0	61			
CSH	1700	1700	848			
Volumes to Capacity	0.02	0.05	0.08			
Queue Length 95th (m)	0.0	0.0	2.0			
Control Delay (s)	0.0	0.0	9.6			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.6			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utilization			26.9%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
10-25-2022  
12: Regent Street & Oak Street

Movement	EBT	EBR	WBT	WBR	NBL	NBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	30	0	0	35	30	25
Future Volume (Veh/h)	30	0	0	35	30	25
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78
Hourly flow rate (vph)	38	0	0	45	38	32
Pedestrians	15			10	30	
Lane Width (m)	3.5	3.5	3.5	3.0	3.0	3.0
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	1			1	2	
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume				68	128	78
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol				68	128	78
IC single (s)				4.1	6.4	6.2
IC 2 stage (s)				2.2	3.5	3.3
p0 queue free %	100			100	95	97
CM capacity (veh/h)	1511			1511	831	947
Direction_Lane #	EB 1	WB 1	NB 1			
Volumes Total	38	45	70			
Volume Left	0	0	38			
Volume Right	0	0	32			
CSH	1700	1700	880			
Volumes to Capacity	0.02	0.03	0.08			
Queue Length 95th (m)	0.0	0.0	2.0			
Control Delay (s)	0.0	0.0	9.4			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.4			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			4.3			
Intersection Capacity Utilization			26.9%		ICU Level of Service	A
Analysis Period (min)			15			



HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
10-25-2022  
13: Oak Street & Site Driveway

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	50	30	0	0	5
Future Volume (Veh/h)	5	50	30	0	0	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.71	0.71	0.71	0.71	0.71	0.71
Hourly flow rate (vph)	7	70	42	0	0	7
Pedestrians			110		25	
Lane Width (m)			3.5		3.0	
Walking Speed (m/s)			1.1		1.1	
Percent Blockage			10		2	
Right turn flare (veh)			None		None	
Median type			None		None	
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	67				261	67
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	67				261	67
IC, single (s)	4.1				6.4	6.2
IC, 2 stage (s)	2.2				3.5	3.3
p0 queue free %	100				100	99
CM capacity (veh/h)	1518				646	983
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	77	42	7			
Volume Left	7	0	0			
Volume Right	0	0	7			
cSH	1518	1700	983			
Volume to Capacity	0.00	0.02	0.01			
Queue Length 95th (m)	0.1	0.0	0.2			
Control Delay (s)	0.7	0.0	8.7			
Lane LOS	A	A	A			
Approach Delay (s)	0.7	0.0	8.7			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			19.0%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
10-25-2022  
14: Sackville Street & Oak Street

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Volume (vph)	15	25	10	0	20	5
Future Volume (vph)	15	25	10	0	20	5
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	19	31	12	0	25	6
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	62	31	117			
Volume Left (vph)	19	0	12			
Volume Right (vph)	12	6	6			
Head (s)	0.02	-0.05	0.07			
Departure Headway (s)	4.2	4.2	4.2			
Degree Utilization, x	0.07	0.04	0.14			
Capacity (veh/h)	827	832	836			
Control Delay (s)	7.5	7.3	7.8			
Approach Delay (s)	7.5	7.3	7.8			
Approach LOS	A	A	A			
Intersection Summary						
Delay			7.7			
Level of Service			A			
Intersection Capacity Utilization			32.4%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 15: Sumach Street & Oak Street

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Volume (vph)	5	20	10	0	0	0	25	55	35	5	60	0
Future Volume (vph)	5	20	10	0	0	0	25	55	35	5	60	0
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Hourly flow rate (vph)	7	27	13	0	0	0	33	73	47	7	80	0
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total (vph)	47	153	87									
Volume Left (vph)	7	33	7									
Volume Right (vph)	13	47	0									
Head (s)		-0.07	-0.04	0.13								
Departure Headway (s)		4.3	4.1	4.3								
Degree Utilization, x		0.06	0.17	0.10								
Capacity (veh/h)		782	865	823								
Control Delay (s)		7.6	7.9	7.8								
Approach Delay (s)		7.6	7.9	7.8								
Approach LOS		A	A	A								
Intersection Summary												
Delay			7.8									
Level of Service			A									
Intersection Capacity Utilization			30.6%									A
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 16: Tubman Avenue & Oak Street

10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Free	Free	Free	Free	Stop	Stop
Traffic Volume (veh/h)	60	0	0	0	0	0
Future Volume (veh/h)	60	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75
Hourly flow rate (vph)	80	0	0	0	0	0
Pedestrians	40	0	0	10	45	0
Lane Width (m)	3.5	0.0	0.0	0.0	0.0	0.0
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	4	0	0	0	0	0
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
PX, platoon unblocked						
VC, conflicting volume		125	165	135		
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol		125	165	135		
IC, single (s)		4.1	6.4	6.7		
IC, 2 stage (s)						
p0 queue free %		2.2	3.5	3.8		
IF (s)		100	100	100		
pM capacity (veh/h)		1474	801	800		
Direction, Lane #	EB 1					
Volume Total	80					
Volume Left	0					
Volume Right	0					
CSH	1700					
Volume to Capacity	0.05					
Queue Length 95th (m)	0.0					
Control Delay (s)	0.0					
Lane LOS						
Approach Delay (s)	0.0					
Approach LOS						
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			23.9%			A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
17: Oak Street & Site Driveway

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4			5	0
Traffic Volume (veh/h)	5	55	0	0	5	0
Future Volume (Veh/h)	5	55	0	0	5	0
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	61	0	0	6	0
Pedestrians			10		10	
Lane Width (m)			0.0		3.0	
Walking Speed (m/s)			1.1		1.1	
Percent Blockage			0		1	
Right turn flare (veh)			None		None	
Median type			None		None	
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked					93	10
VC, conflicting volume	10					
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	10				93	10
IC, single (s)	4.1				6.4	6.2
IC, 2 stage (s)						
IF (s)	2.2				3.5	3.3
p0 queue free %	100				99	100
CM capacity (veh/h)	1610				902	1069
Direction, Lane #	EB 1	SB 1				
Volumes Total	67	6				
Volume Left	6	6				
Volume Right	0	0				
cSH	1610	902				
Volumes to Capacity	0.00	0.01				
Queue Length 95th (m)	0.1	0.2				
Control Delay (s)	0.7	9.0				
Lane LOS	A	A				
Approach Delay (s)	0.7	9.0				
Approach LOS	A	A				
Intersection Summary						
Average Delay		1.4				
Intersection Capacity Utilization		17.0%			ICU Level of Service	A
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
18: River Street & Oak Street

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations		4	4		4						4
Traffic Volume (veh/h)	40	0	20	35	0	65	0	370	25	40	520
Future Volume (Veh/h)	40	0	20	35	0	65	0	370	25	40	520
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	44	0	22	38	0	71	0	407	27	44	571
Pedestrians		75		65		35		35		5	
Lane Width (m)		3.5		3.5		3.5		3.5		3.5	
Walking Speed (m/s)		1.1		1.1		1.1		1.1		1.1	
Percent Blockage		7		6		3		3		0	
Right turn flare (veh)											
Median type								None		None	
Median storage (veh)											
Upstream signal (m)								143			151
pX platoon unblocked	0.87	0.87	0.85	0.87	0.87	0.79	0.85			0.79	
VC, conflicting volume	1230	1233	681	1202	1220	490	646			499	
VC1, stage 1 conf vol											
VC2, stage 2 conf vol											
VCu, unblocked vol	797	800	536	764	784	223	494			234	
IC, single (s)	7.2	6.5	6.2	7.1	7.0	6.2	4.1			4.1	
IC, 2 stage (s)											
IF (s)	3.6	4.0	3.3	3.5	4.5	3.3	2.2			2.2	
p0 queue free %	76	100	95	82	100	88	100			96	
CM capacity (veh/h)	184	234	422	209	200	606	856			1003	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1							
Volumes Total	66	109	434	615							
Volume Left	44	38	0	44							
Volume Right	22	71	27	0							
cSH	227	364	1700	1003							
Volumes to Capacity	0.29	0.30	0.26	0.04							
Queue Length 95th (m)	8.8	9.4	0.0	1.0							
Control Delay (s)	27.3	19.0	0.0	1.2							
Lane LOS	D	C	A	A							
Approach Delay (s)	27.3	19.0	0.0	1.2							
Approach LOS	D	C	C	A							
Intersection Summary											
Average Delay			3.8								
Intersection Capacity Utilization			72.3%			ICU Level of Service				C	
Analysis Period (min)			15								

19: Parliament Street & Cole Street

2032 Future Background AM Model (ExRN)

10-25-2022

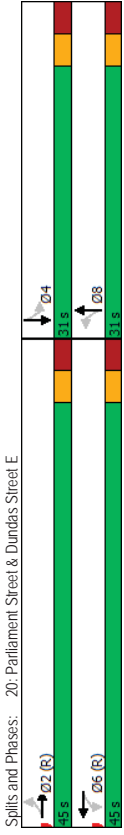
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			←	←	←	←
Traffic Volume (veh/h)	0	0	405	20	25	490
Future Volume (Veh/h)	0	0	405	20	25	490
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	440	22	27	533
Pedestrians	250	10				15
Lane Width (m)	0.0	3.5	3.5			3.5
Walking Speed (m/s)	1.1	1.1	1.1			1.1
Percent Blockage	0	1				1
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			80			214
pX platoon unblocked	0.94	0.94	0.94	0.94	0.94	0.94
VC1 conflicting volume	1032	496				712
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU unblocked vol	902	331				562
IC, single (s)	6.8	6.9				4.2
IC, 2 stage (s)						
p0 queue free %	3.5	3.3				2.2
IF (s)	100	100				97
CM capacity (veh/h)	254	621				931
Direction_Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	293	169	205	355		
Volume Left	0	0	27	0		
Volume Right	0	22	0	0		
cSH	1700	1700	931	1700		
Volumes to Capacity	0.17	0.10	0.03	0.21		
Queue Length 95th (m)	0.0	0.0	0.7	0.0		
Control Delay (s)	0.0	0.0	1.4	0.0		
Lane LOS			A			
Approach Delay (s)	0.0		0.5			
Approach LOS						
Intersection Summary						
Average Delay	0.3					
Intersection Capacity Utilization	44.9%					
ICU Level of Service	A					
Analysis Period (min)	15					

20: Parliament Street & Dundas Street E

2032 Future Background AM Model (ExRN)

10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	←	←	←	←	←	←	←	←
Traffic Volume (vph)	35	300	35	335	40	310	60	390
Future Volume (vph)	35	300	35	335	40	310	60	390
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2		6		8		4	
Permitted Phases	2	2	6	6	8	8	4	4
Detector Phase								
Switch Phase								
Minimum Initial (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Minimum Split (s)	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	45.0	45.0	45.0	45.0	31.0	31.0	31.0	31.0
Total Split (%)	59.2%	59.2%	59.2%	59.2%	40.8%	40.8%	40.8%	40.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)								
Lead-Lag	5.0		5.0		5.0		5.0	5.0
Lead-Lag Optimize?								
Recall Mode								
Act Effct Green (s)	41.4	41.4	41.4	41.4	24.6	24.6	24.6	24.6
Actuated g/C Ratio	0.54	0.54	0.54	0.54	0.32	0.32	0.32	0.32
v/c Ratio	0.25	0.25	0.31	0.31	0.45	0.45	0.57	0.57
Control Delay	9.5	12.9	21.8	21.8	23.5	23.5	23.5	23.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.5	12.9	21.8	21.8	23.5	23.5	23.5	23.5
LOS	A	B	B	B	C	C	C	C
Approach Delay	9.5		12.9		21.8		23.5	
Approach LOS	A		B		C		C	
Intersection Summary								
Cycle Length: 76								
Actuated Cycle Length: 76								
Offset: 54 (7%), Referenced to phase 2EBTL and 6:WBTL, Start of 1st Green								
Natural Cycle: 60								
Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0.57								
Intersection Signal Delay: 17.2								
Intersection Capacity Utilization 91.4%								
ICU Level of Service F								
Analysis Period (min) 15								



Queues 2032 Future Background AM Model (ExRN)  
20: Parliament Street & Dundas Street E

10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	383	478	389	522
Lane Group Flow (vph)	0.25	0.31	0.45	0.57
v/c Ratio	9.5	12.9	21.8	23.5
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	9.5	12.9	21.8	23.5
Total Delay	13.2	26.5	22.8	31.8
Queue Length 50th (m)	22.7	35.4	32.6	43.7
Queue Length 95th (m)	85.0	103.5	45.0	56.1
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1539	1559	919	971
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.25	0.31	0.42	0.54
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
20: Parliament Street & Dundas Street E

10-25-2022

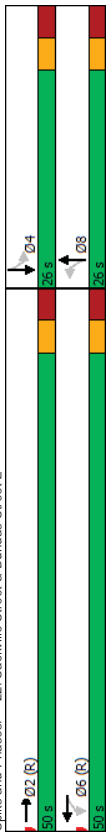
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4TB			4TB			4TB					
Traffic Volume (vph)	35	300	25	35	335	80	40	310	15	60	390	40	
Future Volume (vph)	35	300	25	35	335	80	40	310	15	60	390	40	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Frbp. ped/bikes	0.99	0.99	0.99	0.97	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Frbp. psd/bikes	0.99	0.99	1.00	1.00	0.99	1.00	0.99	0.99	0.99	0.99	0.99	0.99	
Frt	0.99	0.99	0.97	0.97	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Flt Protected	1.00	1.00	1.00	1.00	0.99	1.00	0.99	0.99	0.99	0.99	0.99	0.99	
Satd. Flow (prot)	3167	3100	3100	3100	3112	3112	3112	3112	3112	3112	3112	3112	
Flt Permitted	0.88	0.90	0.90	0.90	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	
Satd. Flow (perm)	2800	2800	2806	2806	2654	2654	2654	2654	2654	2654	2787	2787	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	37	319	27	37	356	85	43	330	16	64	415	43	
RTOR Reduction (vph)	0	7	0	0	23	0	0	4	0	0	9	0	
Lane Group Flow (vph)	0	376	0	0	455	0	0	385	0	0	513	0	
Confl. Peds. (#/hr)	155	110	110	110	155	140	160	160	160	160	140	140	
Confl. Bikes (#/hr)	10	10	10	10	25	25	25	25	25	25	25	25	
Heavy Vehicles (%)	6%	10%	4%	22%	5%	13%	18%	11%	13%	2%	5%	5%	
Parking (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0	
Turn Type	Perm	NA	NA	Perm	NA	NA	Perm	NA	Perm	NA	NA	NA	
Protected Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Permitted Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Actuated Green, G (s)	40.4	40.4	40.4	40.4	40.4	40.4	23.6	23.6	23.6	23.6	23.6	23.6	
Effective Green, g (s)	41.4	41.4	41.4	41.4	41.4	41.4	24.6	24.6	24.6	24.6	24.6	24.6	
Actuated g/C Ratio	0.54	0.54	0.54	0.54	0.54	0.54	0.32	0.32	0.32	0.32	0.32	0.32	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1525	1528	1528	1528	859	859	859	859	859	859	902	902	
v/s Ratio Prot	0.13	0.13	0.13	0.16	0.16	0.15	0.15	0.15	0.15	0.15	0.18	0.18	
v/c Ratio Perm	0.25	0.30	0.30	0.30	0.45	0.45	0.57	0.57	0.57	0.57	0.57	0.57	
Uniform Delay, d1	9.1	9.1	9.1	9.4	20.3	20.3	21.3	21.3	21.3	21.3	21.3	21.3	
Progression Factor	1.00	1.00	1.00	1.42	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.4	0.4	0.4	0.5	0.4	0.4	0.8	0.8	0.8	0.8	0.8	0.8	
Delay (s)	9.5	9.5	9.5	13.9	13.9	20.7	22.1	22.1	22.1	22.1	22.1	22.1	
Level of Service	A	A	A	B	B	B	C	C	C	C	C	C	
Approach Delay (s)	9.5	9.5	9.5	13.9	13.9	20.7	22.1	22.1	22.1	22.1	22.1	22.1	
Approach LOS	A	A	A	B	B	B	C	C	C	C	C	C	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	16.9											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.40												
Actuated Cycle Length (s)	76.0											Sum of lost time (s)	10.0
Intersection Capacity Utilization	91.4%											ICU Level of Service	F
Analysis Period (min)	15												
c Critical Lane Group													

10-25-2022  
 HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 21: Regent Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4T		4T				4T			4T	
Traffic Volume (veh/h)	10	370	10	10	425	35	15	10	5	20	10	20
Future Volume (Veh/h)	10	370	10	10	425	35	15	10	5	20	10	20
Sign Control	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	402	11	11	462	38	16	11	5	22	11	22
Pedestrians	20			5			70				80	
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	2			0			6				7	
Right turn flare (veh)												
Median type	None	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)	127			124								
pX platoon unblocked												
VC, conflicting volume	580			483			800	1102	282	822	1088	350
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	580			483			800	1102	282	822	1088	350
IC, single (s)	4.1			4.1			7.6	6.7	6.9	7.6	6.5	7.1
IC, 2 stage (s)	2.2			2.2			3.6	4.1	3.3	3.5	4.0	3.4
p0 queue free %	99			99			92	93	99	89	94	96
CM capacity (veh/h)	933			1023			201	168	674	202	185	569
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1	SB 1					
Volumes Total	212	212	242	269	32	55						
Volume Left	11	0	11	0	16	22						
Volume Right	0	11	0	38	5	22						
cSH	933	1700	1023	1700	210	266						
Volumes to Capacity	0.01	0.12	0.01	0.16	0.15	0.21						
Queue Length 95th (m)	0.3	0.0	0.2	0.0	4.0	5.8						
Control Delay (s)	0.6	0.0	0.5	0.0	25.2	22.0						
Lane LOS	A	A	A	D	C	C						
Approach Delay (s)	0.3		0.2	25.2	22.0							
Approach LOS			D	C								
Intersection Summary												
Average Delay	2.2											
Intersection Capacity Utilization	35.7%											
ICU Level of Service	A											
Analysis Period (min)	15											

10-25-2022  
 2032 Future Background AM Model (ExRN)  
 22: Sackville Street & Dundas Street E

Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4T			4T		4T	
Traffic Volume (vph)	405	10	400	45	0	35	50
Future Volume (vph)	405	10	400	45	0	35	50
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2		6		8		4
Permitted Phases	2	6	6	8	8	4	4
Detector Phase							
Switch Phase							
Minimum Initial (s)	17.0	17.0	17.0	7.0	7.0	7.0	7.0
Minimum Split (s)	24.0	24.0	24.0	26.0	26.0	26.0	26.0
Total Split (s)	50.0	50.0	50.0	26.0	26.0	26.0	26.0
Total Spill (%)	65.8%	65.8%	65.8%	34.2%	34.2%	34.2%	34.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag							
Lead-Lag Optimize?							
Recall Mode	C-Min	C-Min	C-Min	None	None	None	None
Ad Effct Green (s)	47.6	47.6	47.6	18.4	18.4	18.4	18.4
Actuated g/C Ratio	0.63	0.63	0.63	0.24	0.24	0.24	0.24
v/C Ratio	0.22	0.22	0.22	0.40	0.40	0.40	0.40
Control Delay	6.6	6.6	6.6	12.9	12.9	12.9	20.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	6.6	6.6	6.6	12.9	12.9	12.9	20.2
LOS	A	A	A	B	B	B	C
Approach Delay	6.6	6.6	6.6	12.9	12.9	12.9	20.2
Approach LOS	A	A	A	B	B	B	C
Intersection Summary							
Cycle Length: 76							
Actuated Cycle Length: 76							
Offset: T1 (14%), Referenced to phase 2EBT and 6,WBTL Start of 1st Green							
Natural Cycle: 50							
Control Type: Actuated-Coordinated							
Maximum v/C Ratio: 0.40							
Intersection Signal Delay: 8.3							
Intersection Capacity Utilization 53.1%							
ICU Level of Service A							
Analysis Period (min) 15							



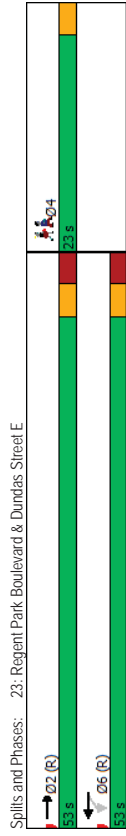
Queues 2032 Future Background AM Model (ExRN) 10-25-2022  
 22: Sackville Street & Dundas Street E

	EBT	WBT	NBT	SBT
Lane Group	452	432	73	122
Lane Group Flow (vph)	0.22	0.40	0.22	0.32
v/c Ratio	6.6	5.6	12.9	20.2
Control Delay	0.0	0.3	0.0	0.0
Queue Delay	6.6	5.9	12.9	20.2
Total Delay	8.9	30.5	3.2	11.0
Queue Length 50th (m)	30.1	8.4	12.5	23.9
Queue Length 95th (m)	99.8	79.6	36.2	126.8
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2020	1075	376	426
Station Cap Reductn	0	217	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.22	0.50	0.19	0.29
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN) 10-25-2022  
 22: Sackville Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	4					4				4
Traffic Volume (vph)	0	405	25	10	400	0	45	0	25	35	50	30
Future Volume (vph)	0	405	25	10	400	0	45	0	25	35	50	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.0	3.5
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.99	0.99	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.98	0.98	0.98
Frbp. psd/bikes	1.00	1.00	1.00	1.00	1.00	0.96	0.96	0.96	0.96	0.97	0.97	0.97
Frt	0.99	0.99	1.00	1.00	0.95	0.95	0.95	0.95	0.95	0.96	0.96	0.96
Flt Protected	1.00	1.00	1.00	1.00	0.97	0.97	0.97	0.97	0.97	0.99	0.99	0.99
Sat'd. Flow (prot)	3229	3229	1736	1736	1545	1545	1545	1545	1545	1630	1630	1630
Flt Permitted	1.00	0.99	0.99	0.99	0.99	0.78	0.78	0.78	0.78	0.89	0.89	0.89
Sat'd. Flow (perm)	3229	3229	1717	1717	1250	1250	1250	1250	1250	1481	1481	1481
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	0	426	26	11	421	0	47	0	26	37	53	32
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	452	0	0	432	0	40	0	40	0	105	0
Confl. Peds. (#/hr)	65	85	85	85	65	65	65	65	60	60	60	65
Confl. Bikes (#/hr)	10	10	10	10	25	25	25	25	25	25	25	25
Heavy Vehicles (%)	0%	8%	13%	0%	8%	0%	0%	0%	8%	6%	2%	4%
Turn Type	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
Protected Phases	2	6	6	6	6	8	8	8	8	4	4	4
Permitted Phases												
Actuated Green, G (s)	46.6	46.6	46.6	46.6	46.6	17.4	17.4	17.4	17.4	17.4	17.4	17.4
Effective Green, g (s)	47.6	47.6	47.6	47.6	47.6	18.4	18.4	18.4	18.4	18.4	18.4	18.4
Actuated g/C Ratio	0.63	0.63	0.63	0.63	0.63	0.24	0.24	0.24	0.24	0.24	0.24	0.24
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	2022	1075	1075	1075	302	302	302	302	302	358	358	358
v/s Ratio Prot	0.14											
v/s Ratio Perm	0.22	0.25	0.25	0.25	0.25	0.03	0.03	0.03	0.03	0.07	0.07	0.07
v/c Ratio	0.22	0.40	0.40	0.40	0.40	0.13	0.13	0.13	0.13	0.29	0.29	0.29
Uniform Delay, d1	6.2	7.1	7.1	7.1	22.6	23.5	23.5	23.5	23.5	23.5	23.5	23.5
Progression Factor	0.92	0.56	0.56	0.56	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.3	1.1	1.1	1.1	0.2	0.2	0.2	0.2	0.2	0.5	0.5	0.5
Delay (s)	5.9	5.0	5.0	5.0	22.8	23.9	23.9	23.9	23.9	23.9	23.9	23.9
Level of Service	A	A	A	A	C	C	C	C	C	C	C	C
Approach Delay (s)	5.9	5.0	5.0	5.0	22.8	23.9	23.9	23.9	23.9	23.9	23.9	23.9
Approach LOS	A	A	A	A	C	C	C	C	C	C	C	C
<b>Intersection Summary</b>												
HCM 2000 Control Delay	8.7 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.37											
Actuated Cycle Length (s)	76.0 Sum of lost time (s) 10.0											
Intersection Capacity Utilization	53.1% ICU Level of Service A											
Analysis Period (min)	15											
c Critical Lane Group												

Lane Group	EBT	WBL	WBT	Ø4
Lane Configurations	4	2	4	
Traffic Volume (vph)	450	20	410	
Future Volume (vph)	450	20	410	
Turn Type	NA	Perm	NA	
Protected Phases	2	6	6	4
Permitted Phases		6	6	
Detector Phase	2	6	6	
Switch Phase				
Minimum Initial (s)	18.0	18.0	18.0	20.0
Minimum Split (s)	23.9	23.9	23.9	23.0
Total Split (s)	53.0	53.0	53.0	23.0
Total Split (%)	69.7%	69.7%	69.7%	30%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.9	4.9	4.9	
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	C-Min	C-Min	C-Min	None
Act Effct Green (s)	64.8	64.8	64.8	
Actuated g/C Ratio	0.85	0.85	0.85	
v/c Ratio	0.33	0.31	0.31	
Control Delay	3.9	4.6	4.6	
Queue Delay	0.0	0.0	0.0	
Total Delay	3.9	4.7	4.7	
LOS	A	A	A	
Approach Delay	3.9	4.7	4.7	
Approach LOS	A	A	A	
<b>Intersection Summary</b>				
Cycle Length:	76			
Actuated Cycle Length:	76			
Offset:	8 (1%) Referenced to phase 2:EBT and 6:WBL, Start of 1st Green			
Natural Cycle:	50			
Control Type:	Actuated-Coordinated			
Maximum v/c Ratio:	0.33			
Intersection Signal Delay:	4.3		Intersection LOS: A	
Intersection Capacity Utilization:	57.3%		ICU Level of Service B	
Analysis Period (min):	15			



Lane Group	EBT	WBT
Lane Group Flow (vph)	485	448
v/c Ratio	0.33	0.31
Control Delay	3.9	4.6
Queue Delay	0.0	0.0
Total Delay	3.9	4.7
Queue Length 50th (m)	6.9	0.0
Queue Length 95th (m)	26.3	45.4
Internal Link Dist (m)	79.6	86.0
Turn Bay Length (m)		
Base Capacity (vph)	1469	1434
Starvation Cap Reductn	36	112
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.34	0.34
<b>Intersection Summary</b>		

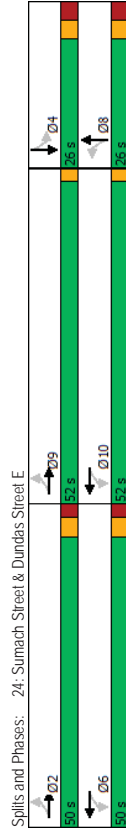


23: Regent Park Boulevard & Dundas Street E  
 HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	EB	EB	WB	WB	NB	NB
Traffic Volume (vph)	450	15	20	410	0	0
Future Volume (vph)	450	15	20	410	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.0	3.0	3.5	3.0	3.0
Total Lost time (s)	4.9		4.9			
Lane Util. Factor	1.00		1.00			
Frbp. ped/bikes	0.99		1.00			
Frbp. ped/bikes	1.00		0.99			
Frt	1.00		1.00			
Flt Protected	1.00		1.00			
Sat'd Flow (prot)	1720		1724			
Flt Permitted	1.00		0.97			
Sat'd Flow (perm)	1720		1680			
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	469	16	21	427	0	0
RTOR Reduction (vph)	1	0	0	0	0	0
Lane Group Flow (vph)	484	0	0	448	0	0
Conf. Peds. (#/hr)	125	125	40	60		
Heavy Vehicles (%)	8%	0%	9%	8%	0%	0%
Turn Types	NA	Perm	NA			
Protected Phases	2		6			
Permitted Phases	6		6			
Actuated Green, G (s)	59.1		59.1			
Effective Green, g (s)	60.1		60.1			
Actuated g/C Ratio	0.79		0.79			
Clearance Time (s)	5.9		5.9			
Vehicle Extension (s)	3.0		3.0			
Lane Grp Cap. (vph)	1360		1328			
v/s Ratio Prot	0.28		0.27			
v/s Ratio Perm	0.36		0.34			
Uniform Delay, d1	2.3		2.3			
Progression Factor	0.80		1.00			
Incremental Delay, d2	0.7		0.7			
Delay (s)	2.6		3.0			
Level of Service	A		A			
Approach Delay (s)	2.6		3.0	0.0		
Approach LOS	A		A	A		
Intersection Summary						
HCM 2000 Control Delay	2.8 HCM 2000 Level of Service A					
HCM 2000 Volume to Capacity ratio	0.31					
Actuated Cycle Length (s)	76.0 Sum of lost time (s)					
Intersection Capacity Utilization	57.3% ICU Level of Service B					
Analysis Period (min)	15					
Critical Lane Group						

24: Sumach Street & Dundas Street E  
 2032 Future Background AM Model (ExRN)  
 10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	EB	EB	WB	WB	NB	NB	SB	SB				
Traffic Volume (vph)	15	415	25	385	15	30	50	30				
Future Volume (vph)	15	415	25	385	15	30	50	30				
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA				
Protected Phases	2,9		6,10		8		4					
Detector Phase	2		6		8		4					
Switch Phase												
Minimum Initial (s)	20.0		20.0		20.0		20.0		16.0	16.0	5.0	5.0
Minimum Split (s)	26.0		26.0		26.0		26.0		21.0	21.0	52.0	52.0
Total Split (s)	26.0		26.0		26.0		26.0		50.0	50.0	52.0	52.0
Total Split (%)	20.3%		20.3%		20.3%		20.3%		39%	39%	41%	41%
Yellow Time (s)	3.0		3.0		3.0		3.0		3.0	3.0	2.0	2.0
All-Red Time (s)	3.0		3.0		3.0		3.0		2.0	2.0	0.0	0.0
Lost Time Adjust (s)	-1.0		-1.0		-1.0		-1.0					
Total Lost Time (s)	5.0		5.0		5.0		5.0					
Lead-Lag												
Lead-Lag Optimize?												
Recall Mode	None		None		None		None		Min	Min	None	None
Act Effct Green (s)	61.4		61.4		22.9		22.9					
Actuated g/C Ratio	0.65		0.65		0.24		0.24					
v/C Ratio	0.45		0.48		0.31		0.39					
Control Delay	8.0		8.3		27.8		38.6					
Queue Delay	1.4		0.3		0.0		0.0					
Total Delay	9.5		8.6		27.8		38.6					
LOS	A		A		C		D					
Approach Delay	9.5		8.6		27.8		38.6					
Approach LOS	A		A		C		D					
Intersection Summary												
Cycle Length	128											
Actuated Cycle Length	94.1											
Natural Cycle	100											
Control Type	Actuated-Uncoordinated											
Maximum v/c Ratio	0.48											
Intersection Signal Delay	13.6											
Intersection Capacity Utilization	58.7%											
Analysis Period (min)	15											



Queues 2032 Future Background AM Model (ExRN)  
24: Sumach Street & Dundas Street E

10-25-2022

	→	←	↑	↓
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	489	499	109	120
v/c Ratio	0.45	0.48	0.31	0.39
Control Delay	8.0	8.3	27.8	38.6
Queue Delay	1.4	0.3	0.0	0.0
Total Delay	9.5	8.6	27.8	38.6
Queue Length 50th (m)	34.6	35.6	11.5	18.8
Queue Length 95th (m)	50.5	52.5	29.3	39.6
Internal Link Dist (m)	86.0	75.5	76.2	121.5
Turn Bay Length (m)				
Base Capacity (vph)	1248	1194	353	311
Station Cap Reductn	544	246	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.69	0.53	0.31	0.39
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
24: Sumach Street & Dundas Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4	4		4	4		4			4	4	
Traffic Volume (vph)	15	415	20	25	385	50	15	30	55	50	30	30	
Future Volume (vph)	15	415	20	25	385	50	15	30	55	50	30	30	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Flpb. ped/bikes	0.98	0.98	0.98	0.97	0.97	0.93	0.93	0.93	0.92	0.92	0.96	0.92	
Flpb. psd/bikes	1.00	1.00	1.00	0.99	0.99	0.97	0.97	0.97	0.96	0.96	0.96	0.96	
Flt	0.99	0.99	0.99	0.99	0.99	0.93	0.93	0.93	0.98	0.98	0.98	0.98	
Flt Protected	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.99	0.98	0.98	0.98	0.98	
Sat'd. Flow (prot)	1698	1698	1659	1659	1659	1463	1463	1463	1504	1504	1504	1504	
Flt Permitted	0.98	0.98	0.98	0.96	0.96	0.95	0.95	0.95	0.84	0.84	0.84	0.84	
Sat'd. Flow (perm)	1668	1668	1603	1603	1603	1373	1373	1373	1295	1295	1295	1295	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	16	451	22	27	418	54	16	33	60	54	33	33	
RTOR Reduction (vph)	0	2	0	0	5	0	0	31	0	0	10	0	
Lane Group Flow (vph)	0	487	0	0	494	0	0	78	0	0	110	0	
Confl. Peds. (#/hr)	70	135	135	70	120	120	50	50	50	50	120	120	
Confl. Bikes (#/hr)		5			30								
Heavy Vehicles (%)	13%	7%	14%	9%	8%	3%	11%	3%	10%	0%	3%	10%	
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	Perm	NA	
Protected Phases	2.9			6.10			8				4		
Permitted Phases	2.9			6.10			8				4		
Actuated Green, G (s)	63.9			63.9			21.8				21.8		
Effective Green, g (s)	64.9			64.9			22.8				22.8		
Actuated g/C Ratio	0.69			0.69			0.24				0.24		
Clearance Time (s)							6.0				6.0		
Vehicle Extension (s)							3.0				3.0		
Lane Grp Cap (vph)	1155			1110			334				315		
v/s Ratio Prot													
v/s Ratio Perm	0.29			0.31			0.06				0.09		
v/c Ratio	0.42			0.45			0.23				0.35		
Uniform Delay, d1	6.3			6.4			28.4				29.3		
Progression Factor	1.00			1.00			1.00				1.00		
Incremental Delay, d2	0.2			0.3			0.4				0.7		
Delay (s)	6.5			6.7			28.8				30.0		
Level of Service	A			A			C				C		
Approach Delay (s)	6.5			6.7			28.8				30.0		
Approach LOS	A			A			C				C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay	10.9											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.43												
Actuated Cycle Length (s)	93.7											Sum of lost time (s)	10.0
Intersection Capacity Utilization	58.7%											ICU Level of Service	B
Analysis Period (min)	15												
c Critical Lane Group													

25: Tubman Avenue & Dundas Street E

10-25-2022

2032 Future Background AM Model (ExRN)

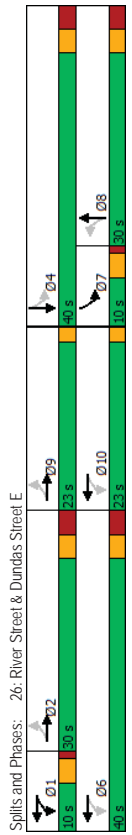
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4									4	
Traffic Volume (veh/h)	0	505	15	5	460	0	0	0	0	0	0	0
Future Volume (Veh/h)	0	505	15	5	460	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	561	17	6	511	0	0	0	0	0	0	0
Pedestrians	5			5			115				45	
Lane Width (m)	3.5			3.5			0.0				3.5	
Walking Speed (m/s)	1.1			1.1			1.1				1.1	
Percent Blockage	0			0			0				4	
Right turn flare (veh)	None			None								
Median type	None			None								
Median storage (veh)												
Upstream signal (m)	99			98								
pX platoon unblocked	0.84			0.89			0.90	0.90	0.89	0.90	0.90	0.84
VC, conflicting volume	556			693			1212	1252	690	1142	1261	561
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	378			590			895	940	586	817	949	384
IC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
IC, 2 stage (s)												
p0 queue free %	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
IC queue free %	100			99			100	100	100	100	100	100
IC capacity (veh/h)	963			883			227	228	454	246	225	537
Direction, Lane #	EB 1	WB 1	SB 1									
Volume Total	578	517	0									
Volume Left	0	6	0									
Volume Right	17	0	0									
cSH	963	883	1700									
Volume to Capacity	0.00	0.01	0.00									
Queue Length 95th (m)	0.0	0.2	0.0									
Control Delay (s)	0.0	0.2	0.0									
Lane LOS	A	A	A									
Approach Delay (s)	0.0	0.2	0.0									
Approach LOS	A	A	A									
Intersection Summary												
Average Delay	0.1											
Intersection Capacity Utilization	39.7%											
Analysis Period (min)	15											
ICU Level of Service	A											

26: River Street & Dundas Street E

10-25-2022

2032 Future Background AM Model (ExRN)

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations		4						4				
Traffic Volume (vph)	10	445	110	315	70	325	120	410				
Future Volume (vph)	10	445	110	315	70	325	120	410				
Turn Type	Perm	NA	custom	NA	Perm	NA	pm+pt	NA				
Protected Phases	2	9	1	1	6	10	8	4				
Permitted Phases	2	2	1	1	6	8	8	7				
Detector Phase												
Switch Phase												
Minimum Initial (s)	6.0				21.0	21.0	5.0	21.0	21.0	21.0	5.0	5.0
Minimum Split (s)	10.0				27.0	27.0	9.0	27.0	27.0	27.0	23.0	23.0
Total Split (s)	30.0				30.0	30.0	10.0	40.0	30.0	40.0	23.0	23.0
Total Split (%)	9.7%				29.1%	29.1%	9.7%	38.8%	29%	39%	22%	22%
Yellow Time (s)	3.0				3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
All-Red Time (s)	1.0				3.0	3.0	1.0	3.0	3.0	3.0	0.0	0.0
Lost Time Adjust (s)	-1.0				-1.0	-1.0	-1.0	-1.0				
Total Lost Time (s)					5.0	5.0	3.0	5.0				
Lead/Lag					Lead	Lag	Lead	Lag				
Lead-Lag Optimize?					Yes	Yes	Yes	Yes				
Recall Mode	None				Min	Min	None	Min	Min	Min	None	None
Act Effct Green (s)	28.4				40.5	25.3	25.3	37.4	35.4			
Actuated g/C Ratio	0.34				0.48	0.30	0.30	0.45	0.42			
v/C Ratio	0.94				0.82	0.43	0.98	0.58	0.77			
Control Delay	52.7				29.0	36.4	66.5	29.5	31.6			
Queue Delay	0.0				0.0	0.0	0.0	0.0	0.0			
Total Delay	52.7				29.0	36.4	66.5	29.5	31.6			
LOS	D				C	D	E	C	C			
Approach Delay					29.0	62.6	62.6	31.2				
Approach LOS					C	E	E	C				
Intersection Summary												
Cycle Length	103											
Actuated Cycle Length	84											
Natural Cycle	100											
Control Type	Actuated-Uncoordinated											
Maximum v/C Ratio	0.98											
Intersection Signal Delay	43.7											
Intersection Capacity Utilization	114.3%											
Analysis Period (min)	15											



Queues 2032 Future Background AM Model (ExRN) 10-25-2022

26: River Street & Dundas Street E

	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group	537	505	74	490	128	521
Lane Group Flow (vph)	0.94	0.82	0.43	0.98	0.58	0.77
v/c Ratio	57.7	29.0	36.4	66.5	29.5	31.6
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	52.7	29.0	36.4	66.5	29.5	31.6
Total Delay	118.9	80.7	#30.4	#179.1	#37.1	#164.4
Queue Length 50th (m)	82.8	54.5	8.8	69.6	10.9	62.2
Queue Length 95th (m)	118.9	80.7	#30.4	#179.1	#37.1	#164.4
Internal Link Dist (m)	73.4	80.5		131.0		118.9
Turn Bay Length (m)			20.0		25.0	
Base Capacity (vph)	583	624	171	500	220	678
Station Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.92	0.81	0.43	0.98	0.58	0.77

Intersection Summary  
 - Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN) 10-25-2022

26: River Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		+										
Traffic Volume (vph)	10	445	50	110	315	50	70	325	135	120	410	80
Future Volume (vph)	10	445	50	110	315	50	70	325	135	120	410	80
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0											
Lane Util. Factor	1.00											
Frbp. ped/bikes	0.98											
Frbp. psd/bikes	1.00											
Frt	0.99											
Flt Protected	1.00											
Satd. Flow (prot)	1701											
Flt Permitted	0.99											
Satd. Flow (perm)	1681											
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	11	473	53	117	335	53	74	346	144	128	436	85
RTOR Reduction (vph)	0	4	0	0	5	0	0	13	0	0	6	0
Lane Group Flow (vph)	0	533	0	0	500	0	74	477	0	128	515	0
Conf. Peds. (#/hr)	65	90	90	65	65	65	65	65	45	45	65	15
Conf. Bikes (#/hr)		5			30							
Heavy Vehicles (%)	0%	7%	4%	3%	7%	0%	9%	9%	1%	5%	12%	7%
Turn Type	Perm	NA	NA	custom	NA	NA	Perm	NA	NA	pm+pl	NA	NA
Protected Phases	2.9			1	1.6	1.0		8		7		4
Permitted Phases				6	10			8		4		4
Actuated Green, G (s)	33.1			43.2			24.3	24.3		34.4		34.4
Effective Green, g (s)	34.1			39.2			25.3	25.3		35.4		35.4
Actuated g/C Ratio	0.40			0.46			0.30	0.30		0.41		0.41
Clearance Time (s)							6.0	6.0		4.0		6.0
Vehicle Extension (s)							3.0	3.0		3.0		3.0
Lane Grp Cap (vph)	669			582			168	481		211		663
v/s Ratio Prot	c0.32			c0.32			c0.29			c0.32		c0.32
v/c Ratio Perm	0.80			0.86			0.44	0.99		0.61		0.78
Uniform Delay, d1	22.7			20.7			24.4	30.0		18.9		21.7
Progression Factor	1.00			1.00			1.00	1.00		1.00		1.00
Incremental Delay, d2	6.5			12.0			1.8	38.5		4.9		5.7
Delay (s)	29.2			32.7			26.3	68.6		23.8		27.4
Level of Service	C			C			C	E		C		C
Approach Delay (s)	29.2			32.7			63.0			26.7		
Approach LOS	C			C			E			C		C
Intersection Summary												
HCM 2000 Control Delay	37.7 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	0.95											
Actuated Cycle Length (s)	85.6 Sum of lost time (s)											
Intersection Capacity Utilization	114.3% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 10-25-2022  
 27: Dreamer's Way & Gerrard Street E

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		
Traffic Volume (veh/h)	350	25	5	785	0	0
Future Volume (Veh/h)	350	25	5	785	0	0
Sign Control	Free	Free	Stop	Stop		
Grade	0%	0%	0%	0%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	389	28	6	872	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None			None		
Median type						
Median storage (veh)	88			164		
Upstream signal (m)	0.97			0.90	0.97	
pX platoon unblocked	417			851	208	
VC, conflicting volume						
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	345			471	130	
IC, single (s)	4.1			6.8	6.9	
IC, 2 stage (s)						
p0 queue free %	2.2			3.5	3.3	
IF (s)	99			100	100	
pM capacity (veh/h)	1192			471	877	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volumes Total	259	158	297	581		
Volume Left	0	0	6	0		
Volume Right	0	28	0	0		
cSH	1700	1700	1192	1700		
Volumes to Capacity	0.15	0.09	0.01	0.34		
Queue Length 95th (m)	0.0	0.0	0.1	0.0		
Control Delay (s)	0.0	0.0	0.2	0.0		
Lane LOS	A	A	A	A		
Approach Delay (s)	0.0		0.1			
Approach LOS			A			
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	28.5%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (ExRN)  
 10-25-2022  
 29: Sumach Street & Site Driveway

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑				↑	↑
Traffic Volume (veh/h)	0	0	60	0	5	65
Future Volume (Veh/h)	0	0	60	0	5	65
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	67	0	6	72
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None		None		None	
Median type						
Median storage (veh)						57
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	151		67		67	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	151		67		67	
IC, single (s)	6.4		6.2		4.1	
IC, 2 stage (s)						
p0 queue free %	3.5		3.3		2.2	
IF (s)	100		100		100	
pM capacity (veh/h)	842		1002		1547	
Direction, Lane #	WB 1	NB 1	SB 1			
Volumes Total	0	67	78			
Volume Left	0	0	6			
Volume Right	0	0	0			
cSH	1700	1700	1547			
Volumes to Capacity	0.00	0.04	0.00			
Queue Length 95th (m)	0.0	0.0	0.1			
Control Delay (s)	0.0	0.0	0.6			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	0.6			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	0.3					
Intersection Capacity Utilization	10.9%					
ICU Level of Service	A					
Analysis Period (min)	15					

1: Parliament Street & Gerrard Street E (North Section)

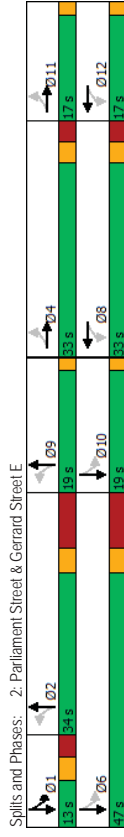
10-25-2022

2: Parliament Street & Gerrard Street E

10-25-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				4TB	4TB	
Traffic Volume (veh/h)	0	0	5	550	500	60
Future Volume (Veh/h)	0	0	5	550	500	60
Sign Control	Slop Free Free					
Grade	0% 0% 0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	5	598	543	65
Pedestrians	80					
Lane Width (m)	0.0					
Walking Speed (m/s)	1.1					
Percent Blockage	0					
Right turn flare (veh)	None					
Median type	None					
Median storage (veh)	None					
Upstream signal (m)	39					
pX platoon unblocked	0.93					
VC1, stage 1 conf vol	970					
VC2, stage 2 conf vol	384					
VC, conflicting volume	688					
VC1, stage 1 unblocked vol	817					
VC2, stage 2 unblocked vol	384					
IC, single (s)	6.8					
IC, 2 stage (s)	6.9					
p0 queue free %	3.5					
IF (s)	3.3					
CM capacity (veh/h)	100					
Direction, Lane #	NB 1		NB 2		SB 2	
Volumes Total	204		399		362	
Volume Left	5		0		0	
Volume Right	0		0		65	
cSH	791		1700		1700	
Volumes to Capacity	0.01		0.23		0.21	
Queue Length 95th (m)	0.1		0.0		0.0	
Control Delay (s)	0.3		0.0		0.0	
Lane LOS	A		A		A	
Approach Delay (s)	0.1		0.0		0.0	
Approach LOS	A		A		A	
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	22.0%					
Analysis Period (min)	15					
ICU Level of Service	A					

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø4	Ø6	Ø8
Lane Configurations	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB				
Traffic Volume (vph)	25	240	65	520	80	335	80	350				
Future Volume (vph)	25	240	65	520	80	335	80	350				
Turn Type	Perm	NA	Perm	NA	Perm	NA	custom	NA				
Protected Phases	4 11		8 12		2 9		1 6 10		2	4	6	8
Permitted Phases	4	4	8	8	2	2	1	1 6				
Switch Phase												
Minimum Initial (s)	6.0											
Minimum Spilt (s)	12.5											
Total Spilt (s)	13.0											
Total Split (%)	11.2%											
Yellow Time (s)	3.3											
All-Red Time (s)	3.2											
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag	Lag											
Lead	Yes											
Lag	Yes											
Recall Mode	None											
Ad Effct Green (s)	33.1		33.1		30.3		48.7					
Actuated g/C Ratio	0.36		0.36		0.33		0.53					
w/C Ratio	0.34		0.79		0.62		0.37					
Control Delay	20.7		31.9		29.8		13.4					
Queue Delay	0.0		0.0		0.0		0.0					
Total Delay	20.7		31.9		29.8		13.4					
LOS	C		C		C		B					
Approach Delay	20.7		31.9		29.8		13.4					
Approach LOS	C		C		C		B					
Intersection Summary												
Cycle Length	116											
Actuated Cycle Length	92.5											
Natural Cycle	115											
Control Type	Actuated-Uncoordinated											
Maximum w/C Ratio	0.79											
Intersection Signal Delay	25.2											
Intersection Capacity Utilization	99.7%											
Analysis Period (min)	15											



Timings  
2: Parliament Street & Gerrard Street E

Queues  
2: Parliament Street & Gerrard Street E

2032 Future Background AM Model (FutRN)  
10-25-2022

2032 Future Background AM Model (FutRN)  
10-25-2022

Lane Group	Ø9	Ø10	Ø11	Ø12
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Turn Type	9	10	11	12
Protected Phases				
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0	5.0	5.0	5.0
Minimum Split (s)	19.0	19.0	17.0	17.0
Total Split (s)	19.0	19.0	17.0	17.0
Total Split (%)	16%	16%	15%	15%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	None	None
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Intersection Summary				

Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	358	820	484	526
v/c Ratio	0.34	0.79	0.62	0.37
Control Delay	20.7	31.9	29.8	13.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	20.7	31.9	29.8	13.4
Queue Length 50th (m)	18.1	54.3	33.0	21.4
Queue Length 95th (m)	37.8	101.8	61.0	43.9
Internal Link Dist (m)	33.0	63.9	119.2	15.0
Turn Bay Length (m)				
Base Capacity (vph)	1055	1078	800	1446
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.34	0.76	0.60	0.36
Intersection Summary				

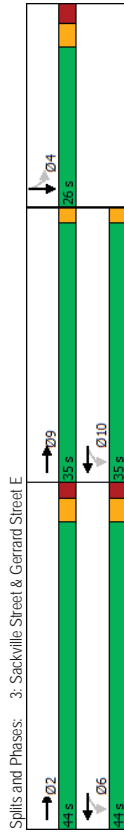


2. Parliament Street & Gerrard Street E 2032 Future Background AM Model (FuFRN) 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB			4TB			4TB		
Traffic Volume (vph)	25	240	75	65	520	195	80	335	45	80	350	70
Future Volume (vph)	25	240	75	65	520	195	80	335	45	80	350	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Fpb. ped/bikes	0.98	0.97	0.97	1.00	0.99	0.98	0.98	0.99	0.99	0.99	0.99	0.99
Fpb. ped/bikes	1.00	0.97	0.96	1.00	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Flt	1.00	0.97	0.96	1.00	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Flt Protected	3308	3203	3181	3203	3181	3203	3181	3203	3181	3203	3181	3203
Sat'd Flow (prot)	0.86	0.88	0.88	0.86	0.88	0.88	0.86	0.88	0.88	0.86	0.88	0.88
Flt Permitted	2867	2825	2389	2867	2825	2389	2867	2825	2389	2867	2825	2389
Sat'd Flow (perm)	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	26	253	79	68	547	205	84	353	47	84	368	74
RTOR Reduction (vph)	0	22	0	0	27	0	0	6	0	0	13	0
Lane Group Flow (vph)	0	336	0	0	793	0	0	478	0	0	513	0
Conf. Peds. (#/hr)	80	85	85	80	85	80	85	120	120	120	85	85
Conf. Bikes (#/hr)	5	20	20	5	20	20	5	20	20	20	5	20
Heavy Vehicles (%)	13%	0%	2%	1%	1%	11%	6%	8%	0%	11%	6%	2%
Turn Type	Perim	MA	MA	Perm	NA	Perm	NA	Perm	NA	custom	MA	MA
Protected Phases	4 11			8 12		2 9				1 6 10		
Permitted Phases		4 11		8 12		2 9				6 10		
Actuated Green, G (s)	37.5	37.5	37.5	38.5	38.5	41.3	40.3	41.3	44.4	44.4	53.6	53.6
Effective Green, g (s)	0.40	0.40	0.40	0.40	0.40	0.43	0.43	0.43	0.47	0.47	0.47	0.47
Actuated g/C Ratio												
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1160			1143		1037				1264		
v/s Ratio Prot				c0.28		60.20				c0.03		
v/s Ratio Perm				0.69		0.46				0.16		
v/c Ratio	0.29	19.1	23.4	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay, d1	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
Delay (s)	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2	19.2
Level of Service	B	B	B	C	C	B	B	B	B	B	B	B
Approach Delay (s)	19.2	19.2	19.2	25.3	25.3	19.4	19.4	19.4	19.4	16.9	16.9	16.9
Approach LOS	B	B	B	C	C	B	B	B	B	B	B	B
Intersection Summary												
HCM 2000 Control Delay	21.0 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.66											
Actuated Cycle Length (s)	95.1 Sum of lost time (s)											
Intersection Capacity Utilization	99.7% ICU Level of Service F											
Analysis Period (min)	15											
c Critical Lane Group												

3. Sackville Street & Gerrard Street E 2032 Future Background AM Model (FuFRN) 10-25-2022

Lane Group	EBT	WBL	WBT	SBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	4TB		4TB	4TB				
Traffic Volume (vph)	330	105	830	35				
Future Volume (vph)	330	105	830	35				
Turn Type	NA	Perm	NA	NA				
Protected Phases	2 9		6 10	4 2	6	9	10	
Permitted Phases			6 10					
Detector Phase	2	6	6	4				
Switch Phase								
Minimum Initial (s)	20.0	16.0	16.0	5.0	5.0			
Minimum Split (s)	25.7	21.0	21.0	35.0	35.0			
Total Split (s)	26.0	44.0	44.0	35.0	35.0			
Total Spill (%)	24.8%	42%	42%	33%	33%			
Yellow Time (s)	3.0	3.0	3.0	2.0	2.0			
All-Red Time (s)	2.7	2.0	2.0	0.0	0.0			
Lost Time Adjust (s)	-1.0							
Total Lost Time (s)	4.7							
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode								
Act Effct Green (s)	48.9	22.9						
Actuated g/C Ratio	0.60	0.60	0.28					
v/c Ratio	0.20	10.2	0.26					
Control Delay	6.0	10.2	31.6					
Queue Delay	0.0	0.0	0.0					
Total Delay	6.0	10.2	31.6					
LOS	A	B	C					
Approach Delay	6.0	10.2	31.6					
Approach LOS	A	B	C					
Intersection Summary								
Cycle Length: 105								
Actuated Cycle Length: 81.2								
Natural Cycle: 85								
Control Type: Actuated-Uncoordinated								
Maximum v/c Ratio: 0.60								
Intersection Signal Delay: 10.8								
Intersection Capacity Utilization 66.6%								
Analysis Period (min) 15								





Queues  
3. Sackville Street & Gerrard Street E

2032 Future Background AM Model (FutRN)  
10-25-2022

	EBT	WBT	SBT
Lane Group	385	1027	114
Lane Group Flow (vph)	0.20	0.60	0.26
v/c Ratio	6.0	10.2	31.6
Control Delay	0.0	0.0	0.0
Queue Delay	6.0	10.2	31.6
Total Delay	11.0	42.2	16.3
Queue Length 50th (m)	15.6	54.8	35.3
Queue Length 95th (m)	56.3	47.8	15.3
Internal Link Dist (m)			
Turn Bay Length (m)	2278	2026	439
Base Capacity (vph)	0	0	0
Starvation Cap Reductn	0	0	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.17	0.51	0.26
<b>Intersection Summary</b>			

HCM Signalized Intersection Capacity Analysis  
3. Sackville Street & Gerrard Street E

2032 Future Background AM Model (FutRN)  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4↑	4↑		4↑	4↑							
Traffic Volume (vph)	0	330	20	105	830	0	0	0	0	55	35	15	
Future Volume (vph)	0	330	20	105	830	0	0	0	0	55	35	15	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)		4.0			4.0						4.7		
Lane Util. Factor		0.95			0.95						1.00		
Frbp. ped/bikes		0.99			1.00						0.99		
Frbp. psd/bikes		1.00			0.99						0.91		
Frt		0.99			1.00						0.98		
Flt Protected		1.00			0.99						0.97		
Sat'd. Flow (prot)		3202			3361						1569		
Flt Permitted		1.00			0.84						0.97		
Sat'd. Flow (perm)		3202			2854						1569		
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	
Adj. Flow (vph)	0	363	22	115	912	0	0	0	0	60	38	16	
RTOR Reduction (vph)	0	5	0	0	0	0	0	0	0	0	5	0	
Lane Group Flow (vph)	0	380	0	0	1027	0	0	0	0	0	109	0	
Confl. Peds. (#/hr)	40	60	60	60	40	60	140	140	60	140	60	5	
Confl. Bikes (#/hr)	0%	10%	0%	9%	4%	0%	0%	0%	0%	0%	6%	6%	
Heavy Vehicles (%)													
Turn Type	NA	Perm	NA	NA	NA	NA	Perm	NA	NA	Perm	NA	NA	
Protected Phases	2.9			6.10						4			
Permitted Phases				6.10									
Actuated Green, G (s)	52.1			52.1						21.8			
Effective Green, g (s)	53.1			53.1						22.8			
Actuated g/C Ratio	0.65			0.65						0.28			
Clearance Time (s)											5.7		
Vehicle Extension (s)											3.0		
Lane Grp Cap (vph)	2083			1857						438			
v/s Ratio Prot	0.12												
v/s Ratio Perm				0.36						0.07			
v/c Ratio	0.18			0.55						0.25			
Uniform Delay, d1	5.6			7.8						22.8			
Progression Factor	1.00			1.00						1.00			
Incremental Delay, d2	0.0			0.4						0.3			
Delay (s)	5.7			8.1						23.1			
Level of Service	A			A						C			
Approach Delay (s)	5.7			8.1			0.0			23.1			
Approach LOS	A			A			A			C			
<b>Intersection Summary</b>													
HCM 2000 Control Delay	8.6											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.48												
Actuated Cycle Length (s)	81.6											Sum of lost time (s)	9.7
Intersection Capacity Utilization	66.6%											ICU Level of Service	C
Analysis Period (min)	15												
c Critical Lane Group													

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 4: Gerrard Street E & Gifford Street

10-25-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4↑	4↑		W	
Traffic Volume (veh/h)	5	380	930	20	5	5
Future Volume (Veh/h)	5	380	930	20	5	5
Sign Control		Free	Free	Stop		
Grade		0%	0%	0%		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	5	396	969	21	5	5
Pedestrians		5	5		50	
Lane Width (m)		3.5	3.5	3.0		
Walking Speed (m/s)		1.1	1.1	1.1		
Percent Blockage		0	0		4	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)		72	141			
PX platoon unblocked	0.91			0.92	0.91	
VC conflicting volume	1040			1242	550	
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	841			981	301	
IC single (s)	4.1			7.5	7.3	
IC 2 stage (s)						
p0 queue free %	2.2			3.8	3.5	
IF (s)	99			97	99	
CM capacity (veh/h)	701			173	560	
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	137	264	646	344	10	10
Volume Left	5	0	0	0	5	5
Volume Right	0	0	0	21	5	5
cSH	701	1700	1700	1700	265	265
Volumes to Capacity	0.01	0.16	0.38	0.20	0.04	0.04
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.9	0.9
Control Delay (s)	0.4	0.0	0.0	0.0	19.1	19.1
Lane LOS	A				C	C
Approach Delay (s)	0.2		0.0		19.1	
Approach LOS					C	
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			38.0%		ICU Level of Service	
Analysis Period (min)			15		A	

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 5: Gerrard Street E & Nasmith Avenue

10-25-2022



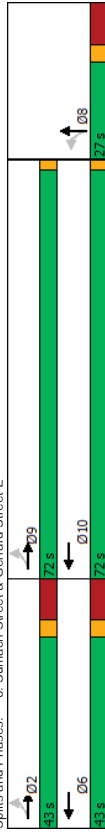
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4↑	4↑		W	
Traffic Volume (veh/h)	5	380	940	5	15	10
Future Volume (Veh/h)	5	380	940	5	15	10
Sign Control		Free	Free	Stop		
Grade		0%	0%	0%		
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	5	392	969	5	15	10
Pedestrians					75	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					6	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)		145	68			
PX platoon unblocked	0.89			0.89	0.89	0.89
VC conflicting volume	1049			1252	562	
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	797			1027	248	
IC single (s)	4.1			6.9	7.1	
IC 2 stage (s)						
p0 queue free %	2.2			3.6	3.4	
IF (s)	99			92	98	
CM capacity (veh/h)	696			184	609	
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	136	261	646	328	25	25
Volume Left	5	0	0	0	15	15
Volume Right	0	0	0	5	10	10
cSH	696	1700	1700	1700	255	255
Volumes to Capacity	0.01	0.15	0.38	0.19	0.10	0.10
Queue Length 95th (m)	0.2	0.0	0.0	0.0	2.4	2.4
Control Delay (s)	0.5	0.0	0.0	0.0	20.6	20.6
Lane LOS	A				C	C
Approach Delay (s)	0.2		0.0		20.6	
Approach LOS					C	
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			36.2%		ICU Level of Service	
Analysis Period (min)			15		A	

Timings 2032 Future Background AM Model (FutRN) 10-25-2022

6: Summach Street & Gerrard Street E

Lane Group	EBL	EBT	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations		4↑	1↑	4↑				
Traffic Volume (vph)	5	390	905	10				
Future Volume (vph)	5	390	905	10				
Turn Type	Perm	NA	NA	NA				
Protected Phases		2, 9	6, 10	8	2	6	9	10
Permitted Phases	2, 9							
Detector Phase	2	2	6	8				
Switch Phase								
Minimum Initial (s)				15.0	11.0	11.0	5.0	5.0
Minimum Split (s)				25.4	26.2	26.2	72.0	72.0
Total Split (s)				27.0	43.0	43.0	72.0	72.0
Total Split (%)				19.0%	30%	30%	51%	51%
Yellow Time (s)				3.0	3.0	3.0	2.0	2.0
All-Red Time (s)				7.4	7.2	7.2	0.0	0.0
Lost Time Adjust (s)				-1.0				
Total Lost Time (s)				9.4				
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode				None	Min	Min	None	None
Act Effct Green (s)		69.1	69.1	19.0				
Actuated g/C Ratio		0.71	0.71	0.20				
v/c Ratio		0.20	0.51	0.32				
Control Delay		6.6	8.9	43.2				
Queue Delay		0.0	0.1	0.0				
Total Delay		6.6	9.0	43.2				
LOS		A	A	D				
Approach Delay		6.6	9.0	43.2				
Approach LOS		A	A	D				
Intersection Summary								
Cycle Length	142							
Actuated Cycle Length	96.8							
Natural Cycle	125							
Control Type	Actuated-Uncoordinated							
Maximum v/c Ratio	0.51							
Intersection Signal Delay	10.2							
Intersection Capacity Utilization	58.1%							
Analysis Period (min)	15							

Splits and Phases: 6: Summach Street & Gerrard Street E



Queues 2032 Future Background AM Model (FutRN) 10-25-2022

6: Summach Street & Gerrard Street E

Lane Group	EBT	WBT	NBT
Lane Group Flow (vph)	454	1184	91
v/c Ratio	0.20	0.51	0.32
Control Delay	6.6	8.9	43.2
Queue Delay	0.0	0.1	0.0
Total Delay	6.6	9.0	43.2
Queue Length 50th (m)	16.8	57.4	8.0
Queue Length 95th (m)	22.2	68.8	36.5
Internal Link Dist (m)	44.0	75.7	33.2
Turn Bay Length (m)			
Base Capacity (vph)	2276	2325	307
Starvation Cap Reductn	0	255	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.20	0.57	0.30
Intersection Summary			

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
6. Sumach Street & Gerrard Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	4		4	4		4				
Traffic Volume (vph)	5	390	0	0	905	125	40	10	30	0	0	0
Future Volume (vph)	5	390	0	0	905	125	40	10	30	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	9.2			9.2			9.4					
Lane Util. Factor	0.95			0.95			1.00					
Frbp. ped/bikes	1.00			0.97			0.95					
Frbp. ped/bikes	1.00			1.00			0.90					
Frt	1.00			0.98			0.95					
Flt Protected	1.00			1.00			0.98					
Sat'd. Flow (prot)	3398			3277			1482					
Flt Permitted	0.94			1.00			0.98					
Sat'd. Flow (perm)	3188			3277			1482					
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	6	448	0	0	1040	144	46	11	34	0	0	0
R/TOR Reduction (vph)	0	0	0	0	7	0	0	15	0	0	0	0
Lane Group Flow (vph)	0	454	0	0	1177	0	0	76	0	0	0	0
Conf. Bikes (#/hr)	65	85	85	65	100	65	100	65	65	65	65	100
Heavy Vehicles (%)	0%	5%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Permi	MA	NA	NA	Permi	NA	NA	Permi	NA	NA	NA	NA
Protected Phases	2.9			6.10			8					
Permitted Phases	2.9			74.3			75.3					
Effective Green, G (s)	74.3			75.3			75.3					
Actuated g/C Ratio	0.76			0.76			0.76					
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	2427			2495			197					
v/s Ratio Prot				c0.36								
v/s Ratio Perm	0.14			0.47			0.39					
v/c Ratio	0.19			0.47			0.39					
Uniform Delay, d1	3.3			4.4			39.2					
Progression Factor	1.00			1.00			1.00					
Incremental Delay, d2	0.0			0.1			1.3					
Delay (s)	3.3			4.5			40.4					
Level of Service	A			A			D					
Approach Delay (s)	3.3			4.5			40.4					0.0
Approach LOS	A			A			D					A
Intersection Summary												
HCM 2000 Control Delay				6.1			HCM 2000 Level of Service					A
HCM 2000 Volume to Capacity ratio				0.50								
Actuated Cycle Length (s)				98.9			Sum of lost time (s)					19.6
Intersection Capacity Utilization				58.1%			ICU Level of Service					B
Analysis Period (min)				15								
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
7. Site Laneway/Sword Street & Gerrard Street E

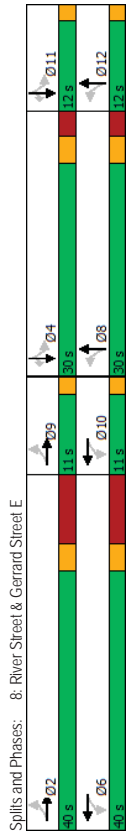
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	4		4	4		4				
Traffic Volume (veh/h)	0	415	5	0	1025	0	0	0	0	10	0	5
Future Volume (Veh/h)	0	415	5	0	1025	0	0	0	0	10	0	5
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	0%	0%	0%	0%
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	437	5	0	1079	0	0	0	0	11	0	5
Pedestrians												75
Lane Width (m)												3.0
Walking Speed (m/s)												1.1
Percent Blockage												6
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)	100			91								
pX, platoon unblocked	0.80			0.99			0.80	0.80	0.99	0.80	0.80	0.80
vC, conflicting volume	1154			442			984	1594	221	1372	1596	614
vC2, stage 1 conf vol												
vC2, stage 2 conf vol												
vCU, unblocked vol	691			414			435	1192	190	917	1195	16
IC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
IC, 2 stage (s)												
IF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	100	100	93	100	99
dM capacity (veh/h)	689			1143			390	143	816	167	143	803
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1							
Volumes Total	291	151	540	540	16							
Volume Left	0	0	0	0	11							
Volume Right	0	5	0	0	5							
cSH	1700	1700	1700	1700	222							
Volumes to Capacity	0.17	0.09	0.32	0.32	0.07							
Queue Length 95th (m)	0.0	0.0	0.0	0.0	1.8							
Control Delay (s)	0.0	0.0	0.0	0.0	22.5							
Lane LOS					C							
Approach Delay (s)	0.0			0.0	22.5							
Approach LOS					C							
Intersection Summary												
Average Delay				0.2								
Intersection Capacity Utilization				38.3%								A
Analysis Period (min)				15								

Timings  
8: River Street & Gerrard Street E

2032 Future Background AM Model (FutRN)  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Lane Configurations												
Traffic Volume (vph)	110	275	160	815	45	365	125	365	165			
Future Volume (vph)	110	275	160	815	45	365	125	365	165			
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm			
Protected Phases	2.9	6.10	6.10	8.12	4.11	4.11	4.11	4.11	4.11	2	4	6
Permitted Phases	2	2	6	6	8	8	4	4	4			
Detector Phase												
Switch Phase												
Minimum Initial (s)										23.0	19.0	23.0
Minimum Split (s)										33.8	30.0	33.8
Total Split (s)										40.0	30.0	40.0
Total Split (%)										43%	32%	43%
Yellow Time (s)										3.0	3.0	3.0
All-Red Time (s)										7.8	3.0	7.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode										Min	Min	Min
Act Effct Green (s)	36.8		36.8	29.9	29.9	29.9	29.9	29.9	29.9			
Actuated g/C Ratio	0.45		0.45	0.37	0.37	0.37	0.37	0.37	0.37			
v/C Ratio	0.58		1.02	0.19	0.71	0.64	0.56	0.29	0.29			
Control Delay	20.8		55.3	20.8	28.7	38.2	24.6	8.6	8.6			
Queue Delay	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Total Delay	20.8		55.3	20.8	28.7	38.2	24.6	8.6	8.6			
LOS	C		E	C	C	D	C	C	A			
Approach Delay	20.8		55.3	28.0	28.0	23.2	23.2	23.2	23.2			
Approach LOS	C		E	C	C	C	C	C	C			
Intersection Summary												
Cycle Length: 93												
Actuated Cycle Length: 81.9												
Natural Cycle: 90												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 1.02												
Intersection Signal Delay: 37.3												
Intersection Capacity Utilization: 117.6%												
Analysis Period (min): 15												



Timings  
8: River Street & Gerrard Street E

2032 Future Background AM Model (FutRN)  
10-25-2022

Lane Group	Ø8	Ø9	Ø10	Ø11	Ø12
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Turn Type					
Protected Phases	8	9	10	11	12
Permitted Phases					
Detector Phase					
Switch Phase					
Minimum Initial (s)	19.0	5.0	5.0	5.0	5.0
Minimum Split (s)	30.0	11.0	11.0	12.0	12.0
Total Split (s)	30.0	11.0	11.0	12.0	12.0
Total Split (%)	32%	12%	12%	13%	13%
Yellow Time (s)	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)	3.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode					
Act Effct Green (s)	Min	None	None	None	None
Actuated g/C Ratio					
v/C Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Intersection Summary					

Queues  
8: River Street & Gerrard Street E

2032 Future Background AM Model (FutRN)  
10-25-2022

	EBT	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group	443	1204	47	453	130	380	172
Lane Group Flow (vph)	0.58	1.02	0.19	0.71	0.64	0.56	0.29
v/c Ratio	20.8	55.3	20.8	28.7	38.2	24.6	8.6
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	20.8	55.3	20.8	28.7	38.2	24.6	8.6
Total Delay	26.5	100.3	5.4	62.8	17.9	50.4	6.3
Queue Length 50th (m)	45.2	#161.7	13.2	95.6	38.9	76.7	19.2
Queue Length 95th (m)	67.0	81.6		126.5		61.7	
Internal Link Dist (m)			30.0				
Turn Bay Length (m)	740	1181	252	668	213	708	609
Base Capacity (vph)	0	0	0	0	0	0	0
Station Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.60	1.02	0.19	0.68	0.61	0.54	0.28

**Intersection Summary**  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
8: River Street & Gerrard Street E

2032 Future Background AM Model (FutRN)  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4TB			4TB								
Traffic Volume (vph)	110	275	40	160	815	180	45	365	70	125	365	165	
Future Volume (vph)	110	275	40	160	815	180	45	365	70	125	365	165	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	9.8	9.8	9.8	9.8	9.8	9.8	5.0	5.0	5.0	5.0	5.0	5.0	
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00	
Frbp. ped/bikes	0.98	0.98	0.98	0.98	0.98	0.98	1.00	0.98	1.00	0.96	1.00	0.94	
Frbp. psd/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.98	0.96	1.00	0.96	
Frt	0.99	0.99	0.99	0.99	0.99	0.99	1.00	0.98	1.00	0.95	1.00	0.85	
Flt Protected													
Satd. Flow (prot)	3189	3261	3261	1528	1738	1528	1738	1528	1738	1577	1860	1420	
Flt Permitted	0.52	0.52	0.52	0.79	0.79	0.79	0.41	1.00	0.34	1.00	1.00	1.00	
Satd. Flow (perm)	1684	1684	1684	2601	2601	2601	665	1738	562	1860	1420	1420	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	
Adj. Flow (vph)	115	286	42	167	849	188	47	380	73	130	380	172	
RTOR Reduction (vph)	0	7	0	0	13	0	0	7	0	0	0	67	
Lane Group Flow (vph)	0	436	0	0	1191	0	47	446	0	130	380	105	
Confl. Peds. (#/hr)	50	95	95	95	95	95	50	40	85	85	40	40	
Confl. Bikes (#/hr)	10	15	15	15	15	15	10	15	15	15	10	10	
Heavy Vehicles (%)	4%	8%	4%	0%	3%	1%	8%	4%	2%	3%	1%	0%	
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	Perm	NA	
Protected Phases	2.9	6.10	6.10	6.10	8.12	8.12	8.12	8.12	4.11	4.11	4.11	4.11	
Permitted Phases	2.9	6.10	6.10	6.10	8.12	8.12	8.12	8.12	4.11	4.11	4.11	4.11	
Actuated Green, G (s)	45.5	45.5	45.5	45.5	33.8	33.8	33.8	33.8	33.8	33.8	33.8	33.8	
Effective Green, g (s)	46.5	46.5	46.5	46.5	34.8	34.8	34.8	34.8	34.8	34.8	34.8	34.8	
Actuated g/C Ratio	0.56	0.56	0.56	0.56	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	
Clearance Time (s)													
Vehicle Extension (s)													
Lane Grp Cap (vph)	940	1451	1451	1451	277	277	277	277	234	777	593	593	
v/s Ratio Prot					c0.26	c0.26							
v/s Ratio Perm	0.26	0.46	0.46	0.46	0.07	0.07	0.07	0.07	0.23	0.23	0.23	0.23	
v/c Ratio	0.46	0.82	0.82	0.82	0.17	0.61	0.17	0.61	0.56	0.49	0.18	0.18	
Uniform Delay, d1	11.0	15.0	15.0	15.0	15.2	19.0	15.2	19.0	18.4	17.7	15.2	15.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.4	3.9	3.9	3.9	0.3	1.6	0.3	1.6	2.8	0.5	0.1	0.1	
Delay (s)	11.3	18.9	18.9	18.9	15.5	20.5	15.5	20.5	21.2	18.2	15.4	15.4	
Level of Service	B	B	B	B	C	C	B	C	C	B	B	B	
Approach Delay (s)	11.3	18.9	18.9	18.9	20.1	20.1	20.1	20.1	18.1	18.1	18.1	18.1	
Approach LOS	B	B	B	B	C	C	B	C	C	B	B	B	
Intersection Summary													
HCM 2000 Control Delay	17.7											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.86												
Actuated Cycle Length (s)	83.3											Sum of lost time (s)	16.8
Intersection Capacity Utilization	117.6%											ICU Level of Service	H
Analysis Period (min)	15												
c Critical Lane Group													

9: Sackville Street & Site Driveway  
 HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	5	10	10	5	0	0	0	0	15	140	5
Future Volume (Veh/h)	0	5	10	10	5	0	0	0	0	15	140	5
Sign Control	Stop	0%	Stop	0%	0%	Free	0%	0%	0%	Free	0%	0%
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	0	6	13	13	6	0	0	0	0	19	182	6
Pedestrians	70			40			15					10
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	0.0	1.1	1.1	1.1	3.5	3.5	1.1
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	6			4			0					1
Right turn flare (veh)												
Median type							None					None
Median storage (veh)												
Upstream signal (m)												58
pX platoon unblocked												
VC, conflicting volume	306	333	270	294	336	50	258			40		
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	306	333	270	294	336	50	258			40		
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	99	98	98	99	100	100			99		
CM capacity (veh/h)	551	527	726	571	525	979	1237			1527		
Direction, Lane #	EB 1	WB 1	SB 1									
Volumes Total	19	19	207									
Volume Left	0	13	19									
Volume Right	13	0	6									
cSH	649	556	1527									
Volumes to Capacity	0.03	0.03	0.01									
Queue Length 95th (m)	0.7	0.8	0.3									
Control Delay (s)	10.7	11.7	0.8									
Lane LOS	B	B	A									
Approach Delay (s)	10.7	11.7	0.8									
Approach LOS	B	B	A									
Intersection Summary												
Average Delay			2.4									
Intersection Capacity Utilization			31.1%									A
Analysis Period (min)			15									

10: Parliament Street & Oak Street  
 HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	45	65	395	10	15	475
Future Volume (Veh/h)	45	65	395	10	15	475
Sign Control	Stop	0%	Free	0%	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	47	68	416	11	16	500
Pedestrians	260		5			175
Lane Width (m)	3.0	3.0	3.5	1.1	1.1	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	20		0			15
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			151			143
pX platoon unblocked	0.99	0.99			0.99	
VC, conflicting volume	968	648			687	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	898	628			666	
IC, single (s)	6.8	7.0			4.1	
IC, 2 stage (s)						
IF (s)	3.5	3.3			2.2	
p0 queue free %	78	76			98	
CM capacity (veh/h)	218	282			742	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volumes Total	115	277	150	183	333	
Volume Left	47	0	0	16	0	
Volume Right	68	0	11	0	0	
cSH	252	1700	1700	742	1700	
Volumes to Capacity	0.46	0.16	0.09	0.02	0.20	
Queue Length 95th (m)	17.0	0.0	0.0	0.5	0.0	
Control Delay (s)	30.8	0.0	0.0	1.1	0.0	
Lane LOS	D	B	A	A	A	
Approach Delay (s)	30.8	0.0	0.0	0.4	0.0	
Approach LOS	D	B	A	A	A	
Intersection Summary						
Average Delay			3.5			
Intersection Capacity Utilization			44.0%			A
Analysis Period (min)			15			

11: Oak Street & Dreamer's Way

12: Regent Street & Oak Street

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
10-25-2022

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Traffic Volume (veh/h)	0	25	65	0	5	50
Future Volume (Veh/h)	0	25	65	0	5	50
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Hourly flow rate (vph)	0	30	79	0	6	61
Pedestrians	15				40	
Lane Width (m)	3.5				3.0	
Walking Speed (m/s)	1.1				1.1	
Percent Blockage	1				3	
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume	119				149	134
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	119				149	134
IC single (s)	4.1				6.4	6.3
IC 2 stage (s)	2.2				3.5	3.4
p0 queue free %	100				99	93
CM capacity (veh/h)	1437				822	851
Direction_Lane #	EB 1	WB 1	SB 1			
Volumes Total	30	79	67			
Volume Left	0	0	6			
Volume Right	0	0	61			
CSH	1700	1700	848			
Volumes to Capacity	0.02	0.05	0.08			
Queue Length 95th (m)	0.0	0.0	2.0			
Control Delay (s)	0.0	0.0	9.6			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.6			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utilization			26.9%			ICU Level of Service A
Analysis Period (min)			15			

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑		↑	↑	↑
Traffic Volume (veh/h)	30	0	0	35	30	25
Future Volume (Veh/h)	30	0	0	35	30	25
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78
Hourly flow rate (vph)	38	0	0	45	38	32
Pedestrians	15			10	30	
Lane Width (m)	3.5			3.5	3.0	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	1			1	1	2
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume			68		128	78
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol			68		128	78
IC single (s)			4.1		6.4	6.2
IC 2 stage (s)			2.2		3.5	3.3
p0 queue free %			100		95	97
CM capacity (veh/h)			1511		831	947
Direction_Lane #	EB 1	WB 1	NB 1			
Volumes Total	38	45	70			
Volume Left	0	0	38			
Volume Right	0	0	32			
CSH	1700	1700	880			
Volumes to Capacity	0.02	0.03	0.08			
Queue Length 95th (m)	0.0	0.0	2.0			
Control Delay (s)	0.0	0.0	9.4			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.4			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			4.3			
Intersection Capacity Utilization			26.9%			ICU Level of Service A
Analysis Period (min)			15			



14: Sackville Street & Oak Street

15: Sumach Street & Oak Street

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop											
Traffic Volume (vph)	0	40	10	0	20	0	0	0	0	20	130	5
Future Volume (vph)	0	40	10	0	20	0	0	0	0	20	130	5
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	0	49	12	0	25	0	0	0	0	25	160	6
Direction, Lane #	EB 1	WB 1	SB 1									
Volume Total (vph)	61	25	191									
Volume Left (vph)	0	0	25									
Volume Right (vph)	12	0	6									
Head (s)	-0.02	0.00	0.10									
Departure Headway (s)	4.3	4.4	4.2									
Degree Utilization, x	0.07	0.03	0.22									
Capacity (veh/h)	791	767	836									
Control Delay (s)	7.7	7.5	8.4									
Approach Delay (s)	7.7	7.5	8.4									
Approach LOS	A	A	A									
Intersection Summary												
Delay	8.2											
Level of Service	A											
Intersection Capacity Utilization	32.5%											
Analysis Period (min)	15											
												A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop											
Traffic Volume (vph)	30	20	10	0	0	0	20	55	35	0	0	0
Future Volume (vph)	30	20	10	0	0	0	20	55	35	0	0	0
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Hourly flow rate (vph)	40	27	13	0	0	0	27	73	47	0	0	0
Direction, Lane #	EB 1	NB 1										
Volume Total (vph)	80	147										
Volume Left (vph)	40	27										
Volume Right (vph)	13	47										
Head (s)	0.08	-0.05										
Departure Headway (s)	4.3	4.0										
Degree Utilization, x	0.10	0.16										
Capacity (veh/h)	807	868										
Control Delay (s)	7.8	7.8										
Approach Delay (s)	7.8	7.8										
Approach LOS	A	A										
Intersection Summary												
Delay	7.8											
Level of Service	A											
Intersection Capacity Utilization	32.1%											
Analysis Period (min)	15											
												A

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
16: Tubman Avenue/Site Laneway & Oak Street

10-25-2022



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	55	0	0	0	0	0	0	0	0	0	5
Future Volume (Veh/h)	0	55	0	0	0	0	0	0	0	0	0	5
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Hourly flow rate (vph)	0	73	0	0	0	0	0	0	0	0	0	7
Pedestrians	40	40	40	40	40	40	45	45	45	45	45	45
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	4	4	4	4	4	4	0	0	0	0	0	0
Right turn flare (veh)												
Median type	None											
Median storage (veh)												
Upstream signal (m)												
pX platoon unblocked												
VC conflicting volume	0	118	118	162	118	128	83	118	40			
VC1 stage 1 conf vol												
VC2 stage 2 conf vol												
VCU unblocked vol	0	118	118	162	118	128	83	118	40			
IC single (s)	4.1	4.1	4.1	7.1	6.5	6.7	7.1	6.5	6.2			
IC 2 stage (s)	2.2	2.2	2.2	3.5	4.0	3.8	3.5	4.0	3.3			
p0 queue free %	100	100	100	100	100	100	100	100	100	99	100	100
CM capacity (veh/h)	1636	1483	1483	774	776	808	909	776	1000			
Direction_Lane #	EB 1	SB 1										
Volumes Total	73	7										
Volume Left	0	0										
Volume Right	0	0										
cSH	1700	776										
Volumes to Capacity	0.04	0.01										
Queue Length 95th (m)	0.0	0.2										
Control Delay (s)	0.0	9.7										
Lane LOS	A	A										
Approach Delay (s)	0.0	9.7										
Approach LOS	A	A										
Intersection Summary												
Average Delay	0.8											
Intersection Capacity Utilization	28.5%											
ICU Level of Service	A											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
17: Oak Street & Site Driveway

10-25-2022



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	5	50	0	0	0	0	0	0	0	5	0	0
Future Volume (Veh/h)	5	50	0	0	0	0	0	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	56	0	0	0	0	0	0	0	6	0	0
Pedestrians	10	10	10	10	10	10	10	10	10	10	10	10
Lane Width (m)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	0	0	0	0	0	0	0	0	0	0	0	0
Right turn flare (veh)												
Median type	None											
Median storage (veh)												
Upstream signal (m)												
pX platoon unblocked												
VC conflicting volume	10	10	10	10	10	10	88	10	10			
VC1 stage 1 conf vol												
VC2 stage 2 conf vol												
VCU unblocked vol	10	10	10	10	10	10	88	10	10			
IC single (s)	4.1	4.1	4.1	4.1	4.1	4.1	6.4	6.2	6.2			
IC 2 stage (s)	2.2	2.2	2.2	2.2	2.2	2.2	3.5	3.3	3.3			
p0 queue free %	100	100	100	100	100	100	99	100	100			
CM capacity (veh/h)	1610	1610	1610	1610	1610	1610	907	1069	1069			
Direction_Lane #	EB 1	SB 1										
Volumes Total	62	6										
Volume Left	6	6										
Volume Right	0	0										
cSH	1610	907										
Volumes to Capacity	0.00	0.01										
Queue Length 95th (m)	0.1	0.2										
Control Delay (s)	0.7	9.0										
Lane LOS	A	A										
Approach Delay (s)	0.7	9.0										
Approach LOS	A	A										
Intersection Summary												
Average Delay	1.5											
Intersection Capacity Utilization	16.8%											
ICU Level of Service	A											
Analysis Period (min)	15											

18: River Street & Oak Street

19: Parliament Street & Cole Street

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
10-25-2022

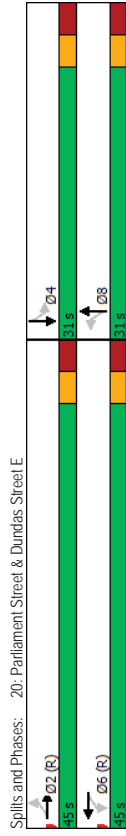
HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	40	0	15	35	0	65	0	375	25	40	525	0
Future Volume (Veh/h)	40	0	15	35	0	65	0	375	25	40	525	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	44	0	16	38	0	71	0	412	27	44	577	0
Pedestrians	75			65			35				5	
Lane Width (m)	3.5			3.5			3.5				3.5	
Walking Speed (m/s)	1.1			1.1			1.1				1.1	
Percent Blockage	7			6			3				0	
Right turn flare (veh)												
Median type							None					
Median storage (veh)												
Upstream signal (m)							14.3					151
PX platoon unblocked	0.87	0.87	0.85	0.87	0.87	0.79	0.85			0.79		
VC, conflicting volume	1242	1244	687	1206	1230	496	652			504		
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	810	812	543	769	797	229	501			240		
IC, single (s)	7.2	6.5	6.2	7.1	7.0	6.2	4.1			4.1		
IC, 2 stage (s)												
pl queue free %	3.6	4.0	3.3	3.5	4.5	3.3	2.2			2.2		
pl queue free %	76	100	96	82	100	88	100			96		
pl capacity (veh/h)	180	230	418	210	197	601	851			997		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volumes Total	60	109	439	621								
Volume Left	44	38	0	44								
Volume Right	16	71	27	0								
cSH	213	364	1700	997								
Volumes to Capacity	0.28	0.30	0.26	0.04								
Queue Length 95th (m)	8.5	9.4	0.0	1.1								
Control Delay (s)	28.5	19.0	0.0	1.2								
Lane LOS	D	C	C	A								
Approach Delay (s)	28.5	19.0	0.0	1.2								
Approach LOS	D	C	C	A								
Intersection Summary												
Average Delay			3.7									
Intersection Capacity Utilization			72.7%									C
Analysis Period (min)			15									

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	0	405	20	25	495
Future Volume (Veh/h)	0	0	405	20	25	495
Sign Control	Stop	Stop	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	440	22	27	538
Pedestrians	260		10			15
Lane Width (m)	0.0		3.5			3.5
Walking Speed (m/s)	1.1		1.1			1.1
Percent Blockage	0		1			1
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			80			214
PX platoon unblocked	0.94	0.94			0.94	
VC, conflicting volume	1034	496			712	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	905	332			562	
IC, single (s)	6.8	6.9			4.2	
IC, 2 stage (s)						
pl queue free %	3.5	3.3			2.2	
pl queue free %	100	100			97	
pl capacity (veh/h)	253	620			931	
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	293	169	206	359		
Volume Left	0	0	27	0		
Volume Right	0	22	0	0		
cSH	1700	1700	931	1700		
Volumes to Capacity	0.17	0.10	0.03	0.21		
Queue Length 95th (m)	0.0	0.0	0.7	0.0		
Control Delay (s)	0.0	0.0	1.4	0.0		
Lane LOS			A			
Approach Delay (s)	0.0		0.5			
Approach LOS						
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			45.0%			A
Analysis Period (min)			15			

Timings 20: Parliament Street & Dundas Street E 2032 Future Background AM Model (FutRN) 10-25-2022

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	30	305	35	335	40	310	65	390
Traffic Volume (vph)	30	305	35	335	40	310	65	390
Future Volume (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	2	6	6	8	8	8	4	4
Protected Phases	2	6	6	8	8	8	4	4
Detector Phase	2	6	6	8	8	8	4	4
Switch Phase	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Minimum Initial (s)	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Minimum Split (s)	45.0	45.0	45.0	45.0	31.0	31.0	31.0	31.0
Total Split (s)	59.2%	59.2%	59.2%	59.2%	40.8%	40.8%	40.8%	40.8%
Total Split (%)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Lost Time Adjust (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Total Lost Time (s)								
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode	C-Min	C-Min	C-Min	C-Min	Min	Min	Min	Min
Act Effct Green (s)	41.3	41.3	24.7	24.7	24.7	24.7	24.7	24.7
Actuated g/C Ratio	0.54	0.54	0.32	0.32	0.32	0.32	0.32	0.32
v/c Ratio	0.25	0.31	0.45	0.45	0.58	0.58	0.58	0.58
Control Delay	9.5	12.4	21.7	21.7	23.7	23.7	23.7	23.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.5	12.4	21.7	21.7	23.7	23.7	23.7	23.7
LOS	A	B	B	C	C	C	C	C
Approach Delay	9.5	12.4	21.7	21.7	23.7	23.7	23.7	23.7
Approach LOS	A	B	B	C	C	C	C	C
Intersection Summary								
Cycle Length: 76								
Actuated Cycle Length: 76								
Offset: 54 (71%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green								
Natural Cycle: 60								
Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0.58								
Intersection Signal Delay: 17.1								
Intersection Capacity Utilization 91.7%								
ICU Level of Service F								
Analysis Period (min) 15								



Queues 20: Parliament Street & Dundas Street E 2032 Future Background AM Model (FutRN) 10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	383	483	389	527
v/c Ratio	0.25	0.31	0.45	0.58
Control Delay	9.5	12.4	21.7	23.7
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	9.5	12.4	21.7	23.7
Queue Length 50th (m)	13.1	26.0	22.8	32.3
Queue Length 95th (m)	22.8	35.5	32.4	44.1
Internal Link Dist (m)	85.0	103.5	45.0	56.1
Turn Bay Length (m)				
Base Capacity (vph)	1561	1556	922	963
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.25	0.31	0.42	0.55
Intersection Summary				

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 20: Parliament Street & Dundas Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	305	25	35	335	85	40	310	15	65	390	40
Future Volume (vph)	30	305	25	35	335	85	40	310	15	65	390	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.0	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Fpb. ped/bikes	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Flt Protected	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	3171	3090	3112	3112	3090	3112	3112	3090	3112	3090	3112	3090
Flt Permitted	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Satd. Flow (perm)	2840	2798	2798	2798	2798	2798	2798	2798	2798	2798	2798	2798
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	32	324	27	37	356	90	43	330	16	69	415	43
RTOR Reduction (vph)	0	7	0	0	25	0	0	4	0	0	9	0
Lane Group Flow (vph)	0	376	0	0	458	0	0	385	0	0	518	0
Conf. Peds. (#/hr)	155	110	110	110	155	140	155	140	160	160	140	155
Conf. Bikes (#/hr)	10	10	10	10	10	25	10	25	10	10	10	10
Heavy Vehicles (%)	6%	10%	4%	22%	5%	13%	18%	11%	13%	2%	5%	5%
Parking (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Turn Type	Perm	NA	Perm	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	2	2	2	6	6	6	8	8	8	8	8	4
Permitted Phases	2	2	2	6	6	6	8	8	8	8	8	4
Actuated Green, G (s)	40.3	40.3	40.3	41.3	41.3	41.3	23.7	23.7	23.7	23.7	23.7	23.7
Effective Green, g (s)	41.3	41.3	41.3	41.3	41.3	41.3	24.7	24.7	24.7	24.7	24.7	24.7
Actuated g/C Ratio	0.54	0.54	0.54	0.54	0.54	0.54	0.32	0.32	0.32	0.32	0.32	0.32
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1543	1520	1520	1520	1520	1520	861	861	861	861	896	896
v/s Ratio Prot	0.13	0.13	0.13	0.16	0.16	0.16	0.15	0.15	0.15	0.15	0.19	0.19
v/s Ratio Perm	0.24	0.24	0.24	0.30	0.30	0.30	0.45	0.45	0.45	0.45	0.58	0.58
Uniform Delay, d1	9.1	9.1	9.1	9.5	9.5	9.5	20.3	20.3	20.3	20.3	21.3	21.3
Progression Factor	1.00	1.00	1.00	1.37	1.37	1.37	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.4	0.4	0.5	0.5	0.5	0.4	0.4	0.4	0.4	0.9	0.9
Delay (s)	9.5	9.5	9.5	13.5	13.5	13.5	20.6	20.6	20.6	20.6	22.2	22.2
Level of Service	A	A	A	B	B	B	C	C	C	C	C	C
Approach Delay (s)	9.5	9.5	9.5	13.5	13.5	13.5	20.6	20.6	20.6	20.6	22.2	22.2
Approach LOS	A	A	A	B	B	B	C	C	C	C	C	C
Intersection Summary												
HCM 2000 Control Delay	16.8 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.40											
Actuated Cycle Length (s)	76.0 Sum of lost time (s) 10.0											
Intersection Capacity Utilization	91.7% ICU Level of Service F											
Analysis Period (min)	15											
c Critical Lane Group												

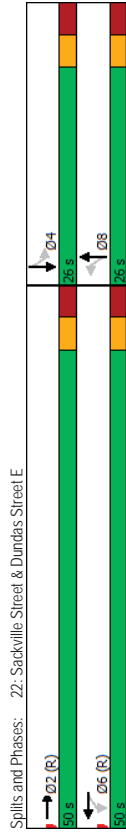
HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 21: Regent Street & Dundas Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	10	380	10	10	430	35	15	10	5	20	10	20
Future Volume (Veh/h)	10	380	10	10	430	35	15	10	5	20	10	20
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	413	11	11	467	38	16	11	5	22	11	22
Pedestrians	20	20	20	20	20	20	20	20	20	20	20	20
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	2	2	2	2	2	2	2	2	2	2	2	2
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None	None	None
Median type	None	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)	127	127	127	127	127	127	127	127	127	127	127	127
Upstream signal (m)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Px. platoon unblocked	585	585	585	494	494	494	814	1118	287	832	1104	352
vC. conflicting volume	585	585	585	494	494	494	814	1118	287	832	1104	352
VC1. stage 1 conf vol	585	585	585	494	494	494	814	1118	284	829	1102	352
VC2. stage 2 conf vol	4.1	4.1	4.1	4.1	4.1	4.1	7.6	6.7	6.9	7.6	6.5	7.1
IC. unblocked vol	2.2	2.2	2.2	2.2	2.2	2.2	3.6	4.1	3.3	3.5	4.0	3.4
IF (s)	99	99	99	99	99	99	92	93	99	89	94	96
p0 capacity (veh/h)	929	929	929	1015	1015	1015	197	164	671	199	182	567
Direction_Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2	SB 1	SB 2				
Volumes Total	218	218	244	272	32	32	55	55				
Volume Left	11	0	11	0	16	22	22	22				
Volume Right	0	11	0	38	5	22	22	22				
CSH	929	1700	1015	1700	205	262	262	262				
Volumes to Capacity	0.01	0.13	0.01	0.16	0.16	0.21	0.21	0.21				
Queue Length 95th (m)	0.3	0.0	0.2	0.0	0.4	1.1	5.9	5.9				
Control Delay (s)	0.6	0.0	0.5	0.0	0.5	22.3	22.3	22.3				
Lane LOS	A	A	A	A	D	C	C	C				
Approach Delay (s)	0.3	0.2	0.2	25.7	22.3	22.3	22.3	22.3				
Approach LOS	D	D	D	C	C	C	C	C				
Intersection Summary												
Average Delay	2.2											
Intersection Capacity Utilization	35.9% ICU Level of Service A											
Analysis Period (min)	15											

Timings 2032 Future Background AM Model (FutRN) 10-25-2022  
 22: Sackville Street & Dundas Street E

	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	↑↑	↑	↔	↔	↔	↔	↔
Traffic Volume (vph)	415	10	395	45	0	75	50
Future Volume (vph)	415	10	395	45	0	75	50
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2		6		8		4
Permitted Phases		6	6	8	8	4	4
Detector Phase		2	6	6	8	8	4
Switch Phase							
Minimum Initial (s)	17.0	17.0	17.0	7.0	7.0	7.0	7.0
Minimum Split (s)	24.0	24.0	24.0	26.0	26.0	26.0	26.0
Total Split (s)	50.0	50.0	50.0	26.0	26.0	26.0	26.0
Total Split (%)	65.8%	65.8%	65.8%	34.2%	34.2%	34.2%	34.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag							
Lead-Lag Optimize?							
Recall Mode	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	47.5	47.5	47.5	18.5	18.5	18.5	18.5
Actuated g/C Ratio	0.62	0.62	0.62	0.24	0.24	0.24	0.24
v/c Ratio	0.23	0.40	0.40	0.22	0.51	0.51	0.51
Control Delay	6.8	6.2	6.2	12.8	26.3	26.3	26.3
Queue Delay	0.0	0.3	0.3	0.0	0.0	0.0	0.0
Total Delay	6.8	6.5	6.5	12.8	26.3	26.3	26.3
LOS	A	A	A	B	B	C	C
Approach Delay	6.8	6.5	6.5	12.8	26.3	26.3	26.3
Approach LOS	A	A	A	B	B	C	C
Intersection Summary							
Cycle Length: 76							
Actuated Cycle Length: 76							
Offset: T1 (14%), Referenced to phase 2EBT and 6WBT, Start of 1st Green							
Natural Cycle: 50							
Control Type: Actuated-Coordinated							
Maximum v/c Ratio: 0.51							
Intersection Signal Delay: 10.1							
Intersection Capacity Utilization 53.2%							
Analysis Period (min) 15							



Queues 2032 Future Background AM Model (FutRN) 10-25-2022  
 22: Sackville Street & Dundas Street E

	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	463	427	427	73	174		
v/c Ratio	0.23	0.40	0.40	0.22	0.51		
Control Delay	6.8	6.2	6.2	12.8	26.3		
Queue Delay	0.0	0.3	0.3	0.0	0.0		
Total Delay	6.8	6.5	6.5	12.8	26.3		
Queue Length 50th (m)	9.3	30.1	30.1	3.2	17.9		
Queue Length 95th (m)	31.0	12.6	12.6	12.5	35.3		
Internal Link Dist (m)	99.8	79.6	79.6	36.2	126.8		
Turn Bay Length (m)							
Base Capacity (vph)	2019	1072	1072	373	385		
Starvation Cap Reductn	0	220	220	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.23	0.50	0.50	0.20	0.45		
Intersection Summary							

2032 Future Background AM Model (FutRN)  
 10-25-2022  
 HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 22: Sackville Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	415	25	10	395	0	45	0	25	75	50	40
Future Volume (vph)	0	415	25	10	395	0	45	0	25	75	50	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)		5.0			5.0			5.0			5.0	
Lane Util. Factor	0.95				1.00			1.00			1.00	
Frbp. ped/bikes	0.99				1.00			0.95			0.98	
Frbp. ped/bikes	1.00				1.00			0.97			0.95	
Frt	0.99				1.00			0.95			0.97	
Frt Protected	1.00				1.00			0.97			0.98	
Sat'd. Flow (prot)	3230				1736			1553			1591	
FIL Permitted	1.00				0.99			0.77			0.82	
Sat'd. Flow (perm)	3230				1716			1234			1338	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	0	437	26	11	416	0	47	0	26	79	53	42
RTOR Reduction (vph)	0	0	0	0	0	0	0	33	0	0	16	0
Lane Group Flow (vph)	0	463	0	0	427	0	0	40	0	0	158	0
Conf. Peds. (#/hr)	65		85	85	65		65	65	60	60	60	65
Conf. Bikes (#/hr)	0		10		25			25			25	
Heavy Vehicles (%)	0%	8%	13%	0%	8%	0%	0%	0%	8%	6%	2%	4%
Turn Type	MA	MA	NA	NA	MA	NA	NA	MA	MA	MA	MA	MA
Protected Phases	2				6			8			4	
Permitted Phases		6			6		8		4		4	
Actuated Green, G (s)	46.5				46.5		17.5		17.5		17.5	
Effective Green, g (s)	47.5				47.5		18.5		18.5		18.5	
Actuated g/C Ratio	0.62				0.62		0.24		0.24		0.24	
Clearance Time (s)	6.0				6.0		6.0		6.0		6.0	
Vehicle Extension (s)	3.0				3.0		3.0		3.0		3.0	
Lane Grp Cap (vph)	2018				1072		300		300		325	
v/s Ratio Prot	0.14				0.25		0.03		0.03		0.12	
v/s Ratio Perm	0.23				0.40		0.13		0.13		0.49	
v/c Ratio	0.23				0.40		0.13		0.13		0.49	
Uniform Delay, d1	6.2				7.1		22.5		24.7		24.7	
Progression Factor	0.94				0.62		1.00		1.00		1.00	
Incremental Delay, d2	0.3				1.1		0.2		0.2		1.1	
Delay (s)	6.1				5.5		22.7		25.8		25.8	
Level of Service	A				A		C		C		C	
Approach Delay (s)	6.1				5.5		22.7		25.8		25.8	
Approach LOS	A				A		C		C		C	
Intersection Summary												
HCM 2000 Control Delay	10.0 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.42											
Actuated Cycle Length (s)	76.0 Sum of lost time (s)											
Intersection Capacity Utilization	53.2% ICU Level of Service A											
Analysis Period (min)	15											
c Critical Lane Group												

2032 Future Background AM Model (FutRN)  
 10-25-2022  
 Timings  
 23: Regent Park Boulevard & Dundas Street E

Lane Group	EBT	WBL	WBT	Ø4
Lane Configurations	4			
Traffic Volume (vph)	495	15	405	
Future Volume (vph)	495	15	405	
Turn Type	NA	Perm	NA	
Protected Phases	2		6	4
Permitted Phases		6		
Detector Phase	2	6	6	
Switch Phase				
Minimum Initial (s)	18.0	18.0	18.0	20.0
Minimum Spilt (s)	23.9	23.9	23.9	23.0
Total Spilt (s)	53.0	53.0	53.0	23.0
Total Spilt (%)	69.7%	69.7%	69.7%	30%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.9		4.9	
Lead-Lag				
Lead-Lag Optimize?				
Recall Mode	C-Min	C-Min	C-Min	None
Act Effct Green (s)	64.8		64.8	
Actuated g/C Ratio	0.85		0.85	
v/c Ratio	0.37		0.30	
Control Delay	4.2		4.6	
Queue Delay	0.0		0.0	
Total Delay	4.2		4.6	
LOS	A		A	
Approach Delay	4.2		4.6	
Approach LOS	A		A	
Intersection Summary				
Cycle Length: 76				
Actuated Cycle Length: 76				
Offset: 8 (11%) Referenced to phase 2:EBT and 6:WBL, Start of 1st Green				
Natural Cycle: 55				
Control Type: Actuated-Coordinated				
Maximum v/c Ratio: 0.37				
Intersection Signal Delay: 4.4				
Intersection Capacity Utilization 52.9%				
ICU Level of Service A				
Analysis Period (min) 15				



Queues 2032 Future Background AM Model (FutRN)  
23: Regent Park Boulevard & Dundas Street E

10-25-2022

	EBT	WBT
Lane Group	537	438
Lane Group Flow (vph)	0.37	0.30
v/c Ratio	4.2	4.6
Control Delay	0.0	0.0
Queue Delay	4.2	4.6
Total Delay	9.2	0.0
Queue Length 50th (m)	28.5	43.8
Queue Length 95th (m)	79.6	86.0
Internal Link Dist (m)	1465	1447
Turn Bay Length (m)	32	116
Base Capacity (vph)	0	0
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0.37	0.33
Reduced v/c Ratio		
<b>Intersection Summary</b>		

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
23: Regent Park Boulevard & Dundas Street E

10-25-2022

	EBT	EBR	WBL	WBT	NBL	NBR
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	495	20	15	405	0	0
Traffic Volume (vph)	495	20	15	405	0	0
Future Volume (vph)	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.5	3.0	3.0	3.5	3.0	3.0
Lane Width	4.9	4.9	4.9	4.9	4.9	4.9
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	0.99	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. psd/bikes	0.99	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	1717	1717	1717	1717	1717	1717
Satd. Flow (prot)	1.00	0.98	0.98	1.00	0.98	0.98
Flt Permitted	1717	1717	1717	1717	1717	1717
Satd. Flow (perm)	0.96	0.96	0.96	0.96	0.96	0.96
Peak-hour factor, PHF	516	21	16	422	0	0
Adj. Flow (vph)	1	0	0	0	0	0
RTOR Reduction (vph)	536	0	0	438	0	0
Lane Group Flow (vph)	125	125	125	40	60	60
Conf. Peds. (#/hr)	8%	0%	9%	8%	0%	0%
Heavy Vehicles (%)	NA	NA	NA	NA	NA	NA
Turn Type	2	6	6	6	6	6
Protected Phases	59.1	60.1	60.1	59.1	60.1	60.1
Permitted Phases	0.79	5.9	5.9	0.79	5.9	5.9
Actuated Green, G (s)	3.0	3.0	3.0	3.0	3.0	3.0
Effective Green, g (s)	1357	1357	1357	1357	1357	1357
Actuated g/C Ratio	c0.31	c0.31	c0.31	c0.31	c0.31	c0.31
Clearance Time (s)	0.39	0.33	0.33	0.39	0.33	0.33
Vehicle Extension (s)	2.4	2.2	2.2	2.4	2.2	2.2
Lane Grp Cap (vph)	0.82	0.7	0.7	0.82	0.7	0.7
v/s Ratio Prot	2.8	2.9	2.9	2.8	2.9	2.9
v/s Ratio Perm	A	A	A	A	A	A
W/C Ratio	2.8	2.9	2.9	2.8	2.9	2.9
Uniform Delay, d1	A	A	A	A	A	A
Progression Factor	2.8	2.9	2.9	2.8	2.9	2.9
Incremental Delay, d2	2.8	2.9	2.9	2.8	2.9	2.9
Delay (s)	A	A	A	A	A	A
Level of Service	2.8	2.9	2.9	2.8	2.9	2.9
Approach Delay (s)	A	A	A	A	A	A
Approach LOS	A	A	A	A	A	A
<b>Intersection Summary</b>						
HCM 2000 Control Delay	2.9	2.9	2.9	2.9	2.9	2.9
HCM 2000 Volume to Capacity ratio	0.35	0.35	0.35	0.35	0.35	0.35
Actuated Cycle Length (s)	76.0	76.0	76.0	76.0	76.0	76.0
Intersection Capacity Utilization	52.9%	52.9%	52.9%	52.9%	52.9%	52.9%
Analysis Period (min)	15	15	15	15	15	15
c. Critical Lane Group						

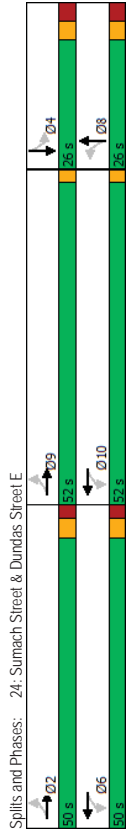


Timings 2032 Future Background AM Model (FutRN) 10-25-2022

24: Sumach Street & Dundas Street E

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	15	435	30	395	15	30	40	40	4	4	4	4
Traffic Volume (vph)	15	435	30	395	15	30	40	40	0	0	0	0
Future Volume (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA	NA	NA	10
Protected Phases	2.9		6.10		8		8		4	2	6	9
Permitted Phases	2	2	6	6	8	8	4	4	4			
Detector Phase												
Switch Phase												
Minimum Initial (s)	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	16.0	16.0	5.0	5.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0	21.0	21.0	52.0	52.0
Total Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0	50.0	50.0	52.0	52.0
Total Split (%)	20.3%	20.3%	20.3%	20.3%	20.3%	20.3%	20.3%	20.3%	39%	39%	41%	41%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0	0.0	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0				
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode												
Act Effct Green (s)	71.8	71.8	71.8	71.8	71.8	71.8	71.8	71.8	25.4	25.4	25.4	25.4
Actuated g/C Ratio	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.78	0.27	0.27	0.27	0.27
v/c Ratio	0.43	0.43	0.42	0.42	0.42	0.42	0.42	0.42	0.17	0.17	0.17	0.17
Control Delay	6.9	6.9	6.8	6.8	6.8	6.8	6.8	6.8	28.1	28.1	20.5	20.5
Queue Delay	1.4	1.4	0.3	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.4	8.4	7.1	7.1	28.1	28.1	20.5	20.5				
LOS	A	A	A	A	C	C	C	C				
Approach Delay	8.4	8.4	7.1	7.1	28.1	28.1	20.5	20.5				
Approach LOS	A	A	A	A	C	C	C	C				

Intersection Summary	Value
Cycle Length	128
Actuated Cycle Length	92.6
Natural Cycle	100
Control Type	Actuated-Uncoordinated
Maximum v/c Ratio	0.43
Intersection Signal Delay	10.1
Intersection Capacity Utilization	61.6%
Analysis Period (min)	15



Queues 2032 Future Background AM Model (FutRN) 10-25-2022

24: Sumach Street & Dundas Street E

Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	538	511	109	54
v/c Ratio	0.43	0.42	0.28	0.17
Control Delay	6.9	6.8	28.1	20.5
Queue Delay	1.4	0.3	0.0	0.0
Total Delay	8.4	7.1	28.1	20.5
Queue Length 50th (m)	39.9	37.2	11.7	3.3
Queue Length 95th (m)	58.2	54.7	29.8	15.0
Internal Link Dist (m)	86.0	75.5	76.2	121.5
Turn Bay Length (m)				
Base Capacity (vph)	1248	1218	390	309
Starvation Cap Reductn	495	246	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.71	0.53	0.28	0.17

Intersection Summary	Value
Cycle Length	128
Actuated Cycle Length	92.6
Natural Cycle	100
Control Type	Actuated-Uncoordinated
Maximum v/c Ratio	0.43
Intersection Signal Delay	10.1
Intersection Capacity Utilization	61.6%
Analysis Period (min)	15



HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 24: Sumach Street & Dundas Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	15	435	45	30	395	45	15	30	55	40	40	10
Future Volume (vph)	15	435	45	30	395	45	15	30	55	40	40	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.96	0.96	0.98	0.98	0.98	0.93	0.93	0.93	0.96	0.94	0.94	0.94
Frbp. ped/bikes	1.00	0.99	0.99	0.99	0.99	0.96	0.96	0.93	0.96	0.93	0.93	0.93
Frt	0.99	0.99	0.99	0.99	0.99	0.93	0.93	0.93	0.97	0.97	0.97	0.97
Flt Protected	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.99	0.96	0.96	0.96	0.96
Sat'd. Flow (prot)	1650	1662	1662	1662	1662	1423	1423	1423	1501	1501	1501	1501
Flt Permitted	0.98	0.98	0.95	0.95	0.95	0.95	0.95	0.95	0.69	0.69	0.69	0.69
Sat'd. Flow (perm)	1624	1584	1584	1584	1584	1364	1364	1364	1083	1083	1083	1083
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	16	473	49	33	429	49	16	33	60	43	43	11
RTOR Reduction (vph)	0	3	0	0	3	0	0	35	0	0	29	0
Lane Group Flow (vph)	0	535	0	0	508	0	0	74	0	0	25	0
Conf. Bikes (#/hr)	70	135	135	70	120	120	50	50	50	50	120	120
Heavy Vehicles (%)	13%	7%	14%	9%	8%	3%	11%	3%	10%	0%	3%	10%
Turn Type	Perm	MA	Perm	NA	Perm	NA	Perm	NA	Perm	MA	Perm	MA
Protected Phases												
Permitted Phases	2,9			6,10			8			4		4
Actuated Green, G (s)	72.2			72.2			13.9			13.9		13.9
Effective Green, g (s)	73.2			73.2			14.9			14.9		14.9
Actuated g/C Ratio	0.78			0.78			0.16			0.16		0.16
Clearance Time (s)							6.0			6.0		6.0
Vehicle Extension (s)							3.0			3.0		3.0
Lane Grp Cap (vph)	1263			1232			215			171		171
v/s Ratio Prot												
v/s Ratio Perm	60.33			0.32			60.05			0.02		0.02
v/c Ratio	0.42			0.41			0.35			0.15		0.15
Uniform Delay, d1	3.5			3.4			35.3			34.1		34.1
Progression Factor	1.00			1.00			1.00			1.00		1.00
Incremental Delay, d2	0.2			0.2			1.0			0.4		0.4
Delay (s)	3.7			3.6			36.2			34.5		34.5
Level of Service	A			A			D			C		C
Approach Delay (s)	3.7			3.6			36.2			34.5		34.5
Approach LOS	A			A			D			C		C
Intersection Summary												
HCM 2000 Control Delay	8.0 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	0.42											
Actuated Cycle Length (s)	94.1 Sum of lost time (s)											
Intersection Capacity Utilization	61.6% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 25: Tubman Avenue & Dundas Street E

10-25-2022

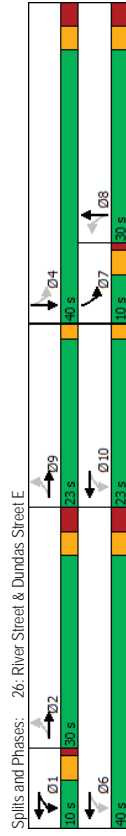
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	515	15	5	470	0	0	0	0	5	0	0
Future Volume (Veh/h)	0	515	15	5	470	0	0	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Slpp	Slpp	0%	0%	0%	0%
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	572	17	6	522	0	0	0	0	6	0	0
Pedestrians	5			5			115			45		45
Lane Width (m)	3.5			3.5			0.0			3.5		3.5
Walking Speed (m/s)	1.1			1.1			1.1			1.1		1.1
Percent Blockage	0			0			0			0		4
Right turn flare (veh)	None			None			None			None		None
Median type	None			None			None			None		None
Median storage (veh)												
Upstream signal (m)	99			98								
pX platoon unblocked	0.84			0.91			0.89			0.91		0.89
vC, conflicting volume	567			704			1234			700		1164
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	391			627			976			1021		623
IC, single (s)	4.1			4.1			7.1			6.2		7.1
IC, 2 stage (s)												
IF (s)	2.2			2.2			3.5			4.0		3.3
p0 queue free %	100			99			100			100		97
dM capacity (veh/h)	952			879			198			201		444
Direction, Lane #	EB 1	WB 1	SB 1									
Volumes Total	589	528	6									
Volume Left	0	6	6									
Volume Right	17	0	0									
ESH	952	879	215									
Volumes to Capacity	0.00	0.01	0.03									
Queue Length 95th (m)	0.0	0.2	0.7									
Control Delay (s)	0.0	0.2	22.3									
Lane LOS	A	A	C									
Approach Delay (s)	0.0	0.2	22.3									
Approach LOS	C	C	C									
Intersection Summary												
Average Delay	0.2											
Intersection Capacity Utilization	41.0%											
ICU Level of Service	A											
Analysis Period (min)	15											

Timings 2032 Future Background AM Model (FutRN) 10-25-2022

Queues 2032 Future Background AM Model (FutRN) 10-25-2022

26: River Street & Dundas Street E

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø6	Ø9	Ø10
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Lane Configurations	15	455	110	315	70	325	110	410					
Traffic Volume (vph)	15	455	110	315	70	325	110	410					
Future Volume (vph)	Perm	NA	custom	NA	Perm	NA	prh+pt	NA					
Turn Type	2.9	1	1.6	10	8	8	7	4					
Protected Phases	2	2	1	1.6	8	8	7	4					
Detector Phase													
Switch Phase													
Minimum Initial (s)	6.0	21.0	21.0	5.0	21.0	21.0	21.0	21.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	10.0	27.0	27.0	9.0	27.0	27.0	27.0	27.0	23.0	23.0	23.0	23.0	23.0
Total Split (s)	10.0	30.0	30.0	10.0	40.0	40.0	40.0	40.0	30.0	30.0	30.0	30.0	23.0
Total Split (%)	9.7%	29.1%	29.1%	9.7%	38.8%	29%	39%	39%	22%	22%	22%	22%	22%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
All-Red Time (s)	1.0	3.0	3.0	1.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	0.0	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0					
Total Lost Time (s)	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag					
Lead/Lag	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes					
Lead-Lag Optimize?	None	Min	Min	None	Min	Min	Min	Min					
Recall Mode	40.7	25.3	25.3	37.4	35.4								
Act Effct Green (s)	0.34	0.48	0.30	0.30	0.44	0.42							
Actuated g/C Ratio	0.97	0.82	0.45	0.98	0.53	0.79							
v/c Ratio	59.0	29.2	37.7	67.1	27.0	32.8							
Control Delay	59.0	29.2	37.7	67.1	27.0	32.8							
Queue Delay	E	C	D	E	C	C							
Total Delay	59.0	29.2	37.7	67.1	27.0	32.8							
LOS	E	C	D	E	C	C							
Approach Delay	59.0	29.2	63.3	31.8									
Approach LOS	E	C	E	C									
Intersection Summary													
Cycle Length	103												
Actuated Cycle Length	84.2												
Natural Cycle	100												
Control Type	Actuated-Uncoordinated												
Maximum v/c Ratio	0.98												
Intersection Signal Delay	45.7												
Intersection Capacity Utilization	115.8%												
Analysis Period (min)	15												



	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	553	505	74	490	117	532
v/c Ratio	0.97	0.82	0.45	0.98	0.53	0.79
Control Delay	59.0	29.2	37.7	67.1	27.0	32.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	59.0	29.2	37.7	67.1	27.0	32.8
Queue Length 50th (m)	-92.7	54.5	8.9	69.6	9.9	64.2
Queue Length 95th (m)	124.9	80.7	#31.6	#179.1	#31.4	#169.2
Internal Link Dist (m)	73.4	80.5		131.0		118.9
Turn Bay Length (m)			20.0		25.0	
Base Capacity (vph)	578	619	164	499	220	674
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.96	0.82	0.45	0.98	0.53	0.79
Intersection Summary						
-	Volume exceeds capacity, queue is theoretically infinite.					
-	Queue shown is maximum after two cycles.					
#	95th percentile volume exceeds capacity, queue may be longer.					
-	Queue shown is maximum after two cycles.					

HCM Signalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 26: River Street & Dundas Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	15	455	50	110	315	50	70	325	135	110	410	90
Future Volume (vph)	15	455	50	110	315	50	70	325	135	110	410	90
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0	5.0	3.0	3.0	5.0	5.0	5.0	5.0	3.0	3.0	5.0	5.0
Lane Util. Factor	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	0.98	0.99	0.99	0.99	0.99	0.95	1.00	0.97	1.00	0.97	1.00	0.97
Frb. ped/bikes	1.00	0.99	0.99	0.99	0.99	0.95	1.00	0.96	1.00	0.97	1.00	0.97
Frb. protected	1.00	0.99	0.99	0.99	0.99	0.95	1.00	0.95	1.00	0.95	1.00	0.97
Satd. Flow (prot)	1702	1705	1705	1705	1705	1462	1630	1601	1601	1597	1601	1597
Frb. permitted	0.98	0.98	0.68	0.68	0.35	1.00	0.14	1.00	0.14	1.00	0.14	1.00
Satd. Flow (perm)	1671	1166	1166	1166	546	1630	238	1597	238	1597	238	1597
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	16	484	53	117	335	53	74	346	144	117	436	96
RTOR Reduction (vph)	0	4	0	0	5	0	0	13	0	0	7	0
Lane Group Flow (vph)	0	549	0	0	500	0	74	477	0	117	525	0
Conf. Bikes (#/hr)	65	90	90	90	65	65	65	45	45	45	65	65
Heavy Vehicles (%)	0%	7%	4%	3%	7%	0%	9%	9%	1%	5%	12%	7%
Turn Type	Perim	MA	MA	custom	NA	Perm	NA	NA	pm+pl	MA	MA	MA
Protected Phases	2.9	1	1	6.10	1	6.10	8	8	7	4	4	4
Permitted Phases	2.9	6.10	6.10	8	8	8	8	8	4	4	4	4
Actuated Green, G (s)	33.3	43.4	43.4	24.3	24.3	24.3	24.3	24.3	34.4	34.4	34.4	34.4
Effective Green, g (s)	34.3	39.4	39.4	25.3	25.3	25.3	25.3	25.3	35.4	35.4	35.4	35.4
Actuated g/C Ratio	0.40	0.46	0.46	0.29	0.29	0.29	0.29	0.29	0.41	0.41	0.41	0.41
Clearance Time (s)				6.0	6.0	6.0	6.0	6.0	4.0	4.0	6.0	6.0
Vehicle Extension (s)				3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	668	580	580	161	480	161	480	210	658	210	658	658
v/s Ratio Prot		c0.07	c0.07	0.05	0.05	0.05	0.05	c0.33	0.05	0.05	c0.33	c0.33
v/s Ratio Perm	0.82	0.86	0.86	0.46	0.46	0.46	0.46	0.56	0.80	0.56	0.80	0.80
v/c Ratio	23.0	20.8	20.8	24.7	30.2	24.7	30.2	18.8	22.1	18.8	22.1	22.1
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	8.0	12.5	12.5	2.1	39.0	2.1	39.0	3.2	6.7	3.2	6.7	6.7
Incremental Delay, d2	31.0	33.2	33.2	26.7	69.2	26.7	69.2	22.0	28.8	22.0	28.8	28.8
Level of Service	C	C	C	C	E	C	E	C	C	C	C	C
Approach Delay (s)	31.0	33.2	33.2	63.6	63.6	63.6	63.6	27.5	27.5	27.5	27.5	27.5
Approach LOS	C	C	C	E	E	E	E	C	C	C	C	C
Intersection Summary												
HCM 2000 Control Delay	38.6 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	0.95											
Actuated Cycle Length (s)	85.8 Sum of lost time (s)											
Intersection Capacity Utilization	115.8% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 27: Dreamer's Way & Gerrard Street E

10-25-2022

Movement	EBT	EBR	WBT	WBR	NBT	NBR
Lane Configurations						
Traffic Volume (veh/h)	340	25	5	780	0	0
Future Volume (Veh/h)	340	25	5	780	0	0
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	378	28	6	867	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None	None	None	None	None	None
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)	88			164		
pX platoon unblocked	0.98	0.98	0.98	0.90	0.98	0.98
vC, conflicting volume	406	406	406	838	203	203
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	344	344	344	487	136	136
iC, single (s)	4.1	4.1	4.1	6.8	6.9	6.9
iC, 2 stage (s)						
IF (s)	2.2	2.2	2.2	3.5	3.3	3.3
p0 queue free %	99	99	99	100	100	100
dM capacity (veh/h)	1198	1198	1198	462	873	873
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volume Total	252	154	295	578		
Volume Left	0	0	6	0		
Volume Right	0	28	0	0		
ESH	1700	1700	1198	1700		
Volumes to Capacity	0.15	0.09	0.01	0.34		
Queue Length 95th (m)	0.0	0.0	0.1	0.0		
Control Delay (s)	0.0	0.0	0.2	0.0		
Lane LOS	A	A	A	A		
Approach Delay (s)	0.0	0.1				
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	28.4%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 29: Sumach Street & Site Driveway

10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	0	80	5	0	0
Future Volume (Veh/h)	0	0	80	5	0	0
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	0	89	6	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None		
Median type				None		
Median storage (veh)					57	
Upstream signal (m)						
pX platoon unblocked					95	
VC conflicting volume	92	92				
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol	92	92			95	
IC single (s)	6.4	6.2			4.1	
IC 2 stage (s)						
IF (s)	3.5	3.3			2.2	
p0 queue free %	100	100			100	
CM capacity (veh/h)	913	971			1512	
Direction Lane #	WB 1	NB 1				
Volumes Total	0	95				
Volume Left	0	0				
Volume Right	0	6				
cSH	1700	1700				
Volumes to Capacity	0.00	0.06				
Queue Length 95th (m)	0.00	0.00				
Control Delay (s)	0.00	0.00				
Lane LOS	A	A				
Approach Delay (s)	0.00	0.00				
Approach LOS	A	A				
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utilization			7.8%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 31: Site Laneway & Gerrard Street E

10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	345	5	0	845	0	5
Future Volume (Veh/h)	345	5	0	845	0	5
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	383	6	0	939	0	6
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				None		
Median type				None		
Median storage (veh)						
Upstream signal (m)				80		
pX platoon unblocked						
VC conflicting volume				389		194
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol				389		194
IC single (s)				4.1		6.8
IC 2 stage (s)						
IF (s)				2.2		3.5
p0 queue free %				100		99
CM capacity (veh/h)				1181		429
Direction Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volumes Total	255	134	470	470	6	
Volume Left	0	0	0	0	0	
Volume Right	0	6	0	0	6	
cSH	1700	1700	1700	1700	820	
Volumes to Capacity	0.15	0.08	0.28	0.28	0.01	
Queue Length 95th (m)	0.00	0.00	0.00	0.00	0.2	
Control Delay (s)	0.00	0.00	0.00	0.00	9.4	
Lane LOS	A	A	A	A	A	
Approach Delay (s)	0.00	0.00	0.00	0.00	9.4	
Approach LOS	A	A	A	A	A	
Intersection Summary						
Average Delay				0.0		
Intersection Capacity Utilization				26.7%		ICU Level of Service
Analysis Period (min)				15		A

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background AM Model (FutRN)  
 32: Oak Street & Site Laneway

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	50	35	0	5	0
Future Volume (Veh/h)	5	50	35	0	5	0
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	56	39	0	6	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None		
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked					107	39
VC conflicting volume						
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol					107	39
IC single (s)					6.4	6.2
IC 2 stage (s)						
IF (s)					3.5	3.3
p0 queue free %					99	100
CM capacity (veh/h)					892	1038
Direction Lane #	EB 1	WB 1	SB 1			
Volumes Total	62	39	6			
Volume Left	6	0	6			
Volume Right	0	0	0			
cSH	1584	1700	892			
Volumes to Capacity	0.00	0.02	0.01			
Queue Length 95th (m)	0.1	0.0	0.2			
Control Delay (s)	0.7	0.0	9.1			
Lane LOS	A	A	A			
Approach Delay (s)	0.7	0.0	9.1			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			16.8%			ICU Level of Service A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 1: Parliament Street & Gerrard Street E (North Section)

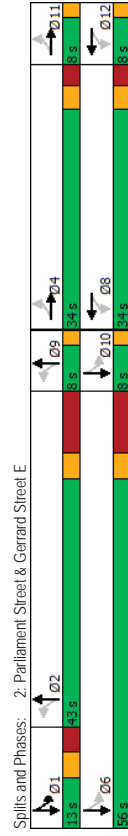
10-25-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	0	10	515	585	75
Future Volume (Veh/h)	0	0	10	515	585	75
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	0	0	11	579	657	84
Pedestrians	150				5	10
Lane Width (m)	0.0				3.5	3.5
Walking Speed (m/s)	1.1				1.1	1.1
Percent Blockage	0				0	1
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)					39	
pX platoon unblocked						
VC conflicting volume					1170	891
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol					1078	891
IC single (s)					6.8	4.3
IC 2 stage (s)						
IF (s)					3.5	2.3
p0 queue free %					100	98
CM capacity (veh/h)					500	702
Direction Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	204	386	438	303		
Volume Left	11	0	0	0		
Volume Right	0	0	0	84		
cSH	702	1700	1700	1700		
Volumes to Capacity	0.02	0.23	0.26	0.18		
Queue Length 95th (m)	0.4	0.0	0.0	0.0		
Control Delay (s)	0.7	0.0	0.0	0.0		
Lane LOS	A	A	A	A		
Approach Delay (s)	0.2		0.0			
Approach LOS	A		A			
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			32.9%			ICU Level of Service A
Analysis Period (min)			15			

Timings  
2: Parliament Street & Gerrard Street E

2032 Future Background PM Model (ExRN)  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø4	Ø6	Ø8
Lane Configurations												
Traffic Volume (vph)	30	545	40	250	75	355	230	285	41%			
Future Volume (vph)	30	545	40	250	75	355	230	285				
Turn Type	Perm	NA	Perm	NA	Perm	NA	custom	NA				
Protected Phases	4 11	8 12	2 9	1 16	10	2	4	6	8			
Permitted Phases	4	4	8	8	2	2	1	1	6			
Detector Phase												
Switch Phase												
Minimum Initial (s)					6.0	22.0	21.0	22.0	21.0	22.0	21.0	21.0
Minimum Split (s)					12.5	33.2	31.8	33.2	31.8	33.2	31.8	31.8
Total Split (s)					13.0	43.0	34.0	56.0	34.0	56.0	34.0	34.0
Total Split (%)					12.3%	41%	32%	53%	32%	53%	32%	32%
Yellow Time (s)					3.3	3.3	3.0	3.3	3.0	3.3	3.0	3.0
All-Red Time (s)					3.2	7.9	2.8	7.9	2.8	7.9	2.8	2.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag												
Lead-Lag Optimize?					Yes	Lag	Yes	Lag				
Recall Mode					None		None		Min	Min	Min	Min
Act Effct Green (s)	31.4	31.4	0.33	0.30	28.9	53.0						
Actuated g/C Ratio	0.33	0.33	0.49	0.76	0.51							
v/c Ratio	0.64	0.64	0.49	0.76	0.51							
Control Delay	30.0	22.5	37.1	13.4								
Queue Delay	0.0	0.0	0.0	0.0								
Total Delay	30.0	22.5	37.1	13.4								
LOS	C	C	D	B								
Approach Delay	30.0	22.5	37.1	13.4								
Approach LOS	C	C	D	B								
Intersection Summary												
Cycle Length: 106												
Actuated Cycle Length: 94.9												
Natural Cycle: 95												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.76												
Intersection Signal Delay: 25.9												
Intersection Capacity Utilization 99.2%												
Analysis Period (min) 15												



Timings  
2: Parliament Street & Gerrard Street E

2032 Future Background PM Model (ExRN)  
10-25-2022

Lane Group	Ø9	Ø10	Ø11	Ø12
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Turn Type				
Protected Phases	9	10	11	12
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0	5.0	5.0	5.0
Minimum Split (s)	8.0	8.0	8.0	8.0
Total Split (s)	8.0	8.0	8.0	8.0
Total Split (%)	8%	8%	8%	8%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	None	None
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Intersection Summary				

Queues 2032 Future Background PM Model (ExRN)  
 2: Parliament Street & Gerrard Street E

10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	665	443	562	603
Lane Group Flow (vph)	0.64	0.49	0.76	0.51
v/c Ratio	30.0	22.5	37.1	13.4
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	30.0	22.5	37.1	13.4
Total Delay	54.7	28.0	50.0	30.9
Queue Length 50th (m)	76.4	43.9	72.2	43.6
Queue Length 95th (m)	33.0	65.6	119.2	15.0
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1099	934	847	1235
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.61	0.47	0.66	0.49
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 2: Parliament Street & Gerrard Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4TB			4TB			4TB					
Traffic Volume (vph)	30	545	70	40	250	140	75	365	115	230	285	70	
Future Volume (vph)	30	545	70	40	250	140	75	365	115	230	285	70	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)		4.8			4.8			10.2				5.5	
Lane Util. Factor		0.95			0.95			0.95				0.95	
Frbp. ped/bikes		0.98			0.95			0.93				0.99	
Frbp. psd/bikes		1.00			1.00			0.99				0.97	
Frt		0.98			0.95			0.97				0.98	
Flt Protected		1.00			1.00			0.99				0.98	
Sat'd. Flow (prot)		3400			3094			3105				3172	
Flt Permitted		0.91			0.83			0.75				0.60	
Sat'd. Flow (perm)		3117			2592			2347				1949	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	31	562	72	41	258	144	77	366	119	237	294	72	
RTOR Reduction (vph)	0	9	0	0	53	0	0	18	0	0	0	11	
Lane Group Flow (vph)	0	656	0	0	390	0	0	544	0	0	0	592	
Confl. Peds. (#/hr)	130	120	120	130	125	125	235	235	125	235	235	125	
Confl. Bikes (#/hr)			25			10			15			5	
Heavy Vehicles (%)	10%	0%	5%	0%	0%	10%	4%	2%	2%	6%	4%	0%	
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	4 11		8 12		8 12		2 9		2 9		1 6 10		
Permitted Phases												6 10	
Actuated Green, G (s)		35.1			35.1			43.8				56.9	
Effective Green, g (s)		36.1			36.1			44.8				47.7	
Actuated g/C Ratio		0.38			0.38			0.47				0.50	
Clearance Time (s)													
Vehicle Extension (s)													
Lane Grp Cap (vph)		1172			974			1095				1065	
v/s Ratio Prot												60.04	
v/s Ratio Perm		60.21			0.15			60.23				60.23	
v/c Ratio		0.56			0.40			0.50				0.56	
Uniform Delay, d1		23.7			22.0			17.8				16.8	
Progression Factor		1.00			1.00			1.00				1.00	
Incremental Delay, d2		0.6			0.3			0.4				0.6	
Delay (s)		24.3			22.3			18.1				17.4	
Level of Service		C			C			B				B	
Approach Delay (s)		24.3			22.3			18.1				17.4	
Approach LOS		C			C			B				B	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	20.5											HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.63												
Actuated Cycle Length (s)	96.0											Sum of lost time (s)	22.5
Intersection Capacity Utilization	99.2%											ICU Level of Service	F
Analysis Period (min)	15												
c Critical Lane Group													

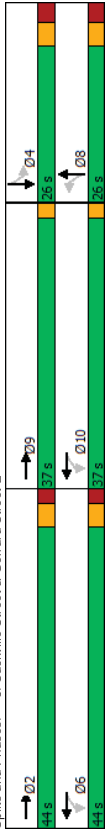


**Timings**  
**3. Sackville Street & Gerrard Street E**  
 2032 Future Background PM Model (ExRN)  
 10-25-2022

Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø6	Ø9	Ø10
Lane Configurations	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑	↑↑			
Traffic Volume (vph)	940	35	535	5	0	135	40			
Future Volume (vph)	940	35	535	5	0	135	40			
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA			
Protected Phases	2.9		6.10		8		4		2	6
Permitted Phases		6	10		8		4		2	6
Detector Phase		2	6		8		4			
Switch Phase										
Minimum Initial (s)				20.0	20.0	20.0	20.0	16.0	16.0	5.0
Minimum Split (s)				25.7	25.7	25.7	25.7	21.0	21.0	37.0
Total Split (s)				26.0	26.0	26.0	26.0	44.0	44.0	37.0
Total Split (%)				24.3%	24.3%	24.3%	24.3%	41%	41%	35%
Yellow Time (s)				3.0	3.0	3.0	3.0	3.0	3.0	2.0
All-Red Time (s)				2.7	2.7	2.7	2.7	2.0	2.0	0.0
Lost Time Adjust (s)				-1.0	-1.0	-1.0	-1.0			
Total Lost Time (s)				4.7	4.7	4.7	4.7			

Lead/Lag	Min	Min	Min	Min	Min	Min	Min
Lead-Lag Optimize?							
Recall Mode							
Act Effct Green (s)	37.3		37.3	21.7	21.7		
Actuated g/C Ratio	0.55		0.55	0.32	0.32		
v/c Ratio	0.53		0.37	0.08	0.50		
Control Delay	10.2		8.8	7.3	26.4		
Queue Delay	10.2		0.0	0.0	0.0		
Total Delay	10.2		8.8	7.3	26.4		
LOS	B		A	A	C		
Approach Delay	10.2		8.8	7.3	26.4		
Approach LOS	B		A	A	C		

Intersection Summary  
 Cycle Length: 107  
 Actuated Cycle Length: 67.9  
 Natural Cycle: 85  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 0.53  
 Intersection Signal Delay: 11.5  
 Intersection Capacity Utilization 66.9%  
 Analysis Period (min) 15



**Queues**  
**3. Sackville Street & Gerrard Street E**  
 2032 Future Background PM Model (ExRN)  
 10-25-2022

Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	995	593	36	209
v/c Ratio	0.53	0.37	0.08	0.50
Control Delay	10.2	8.8	7.3	26.4
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	10.2	8.8	7.3	26.4
Queue Length 50th (m)	36.7	19.6	0.0	22.0
Queue Length 95th (m)	49.7	28.2	5.9	47.8
Internal Link Dist (m)	138.2	47.8	34.3	15.3
Turn Bay Length (m)				
Base Capacity (vph)	2215	1868	469	426
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.45	0.32	0.08	0.49

Intersection Summary

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 3: Sackville Street & Gerrard Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4↑	4↑		4↑	4↑		4↑			4↑	4↑
Traffic Volume (vph)	0	940	15	35	535	0	5	0	30	135	40	25
Future Volume (vph)	0	940	15	35	535	0	5	0	30	135	40	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.5	3.0	3.0	3.0	3.5	3.0
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.7	4.7	4.7	4.7	4.7	4.7
Lane Util. Factor	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	0.94	0.94	0.94	0.94	0.99	0.99	0.99
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.98	0.98	0.96	0.96	0.96
Frt	1.00	1.00	1.00	1.00	1.00	0.88	0.88	0.97	0.97	0.97	0.97	0.97
Sat'd Flow (prot)	3448	3448	3448	3363	3363	1466	1466	1468	1468	1468	1468	1468
FIL Permitted	1.00	0.86	0.86	0.86	0.86	0.96	0.96	0.78	0.78	0.78	0.78	0.78
Sat'd Flow (perm)	3448	2911	2911	2911	2911	1419	1419	1349	1349	1349	1349	1349
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	979	16	36	557	0	5	0	31	141	42	26
RTOR Reduction (vph)	0	2	0	0	0	0	0	25	0	0	4	0
Lane Group Flow (vph)	0	993	0	0	593	0	0	11	0	0	205	0
Conf. Peds. (#/hr)	85	70	70	70	85	90	60	60	60	60	90	90
Conf. Bikes (#/hr)	0	60	60	60	60	15	5	5	5	5	10	10
Heavy Vehicles (%)	0%	3%	0%	0%	6%	0%	0%	0%	5%	1%	0%	0%
Turn Type	MA	MA	NA	NA	NA	NA	NA	NA	NA	MA	MA	MA
Protected Phases	2.9	6.10	6.10	6.10	6.10	8	8	8	8	4	4	4
Permitted Phases	39.8	39.8	39.8	20.6	20.6	20.6	20.6	20.6	20.6	20.6	20.6	20.6
Effective Green, G (s)	40.8	40.8	40.8	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Actuated g/C Ratio	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Clearance Time (s)	7.9	7.9	7.9	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	2065	1744	1744	450	450	427	427	427	427	427	427	427
v/s Ratio Prot	c0.29											
v/s Ratio Perm	0.48	0.20	0.34	0.03	0.03	c0.15	c0.15	c0.15	c0.15	c0.15	c0.15	c0.15
v/c Ratio	7.7	6.9	6.9	16.0	16.0	18.7	18.7	18.7	18.7	18.7	18.7	18.7
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	0.2	0.1	0.1	0.0	0.0	0.9	0.9	0.9	0.9	0.9	0.9	0.9
Incremental Delay, d2	7.9	7.0	7.0	16.0	16.0	19.6	19.6	19.6	19.6	19.6	19.6	19.6
Delay (s)	A	A	A	B	B	B	B	B	B	B	B	B
Level of Service	A	A	A	B	B	B	B	B	B	B	B	B
Approach Delay (s)	7.9	7.0	7.0	16.0	16.0	19.6	19.6	19.6	19.6	19.6	19.6	19.6
Approach LOS	A	A	A	B	B	B	B	B	B	B	B	B
Intersection Summary												
HCM 2000 Control Delay	9.1 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	0.50											
Actuated Cycle Length (s)	68.1 Sum of lost time (s)											
Intersection Capacity Utilization	66.9% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 4: Gerrard Street E & Gifford Street

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4↑	4↑		4↑	4↑		4↑			4↑	4↑
Traffic Volume (veh/h)	10	1095	565	5	15	5	5	15	5	15	5	5
Future Volume (Veh/h)	10	1095	565	5	15	5	5	15	5	15	5	5
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	12	1288	665	6	18	6	6	18	6	18	6	6
Pedestrians	5	5	5	5	5	5	5	5	5	5	5	5
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	0	0	0	0	0	0	0	0	0	0	0	0
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None	None	None
Median type	None	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)	72	141	141	141	141	141	141	141	141	141	141	141
Upstream signal (m)	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
pX platoon unblocked	736	736	736	736	736	736	736	736	736	736	736	736
vC, conflicting volume	0	0	0	0	0	0	0	0	0	0	0	0
vC1, stage 1 conf vol	0	0	0	0	0	0	0	0	0	0	0	0
vC2, stage 2 conf vol	0	0	0	0	0	0	0	0	0	0	0	0
vCu, unblocked vol	678	678	678	678	678	678	678	678	678	678	678	678
IC, single (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
IC, 2 stage (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2	2.2
p0 queue free %	99	99	99	99	99	99	99	99	99	99	99	99
dM capacity (veh/h)	856	856	856	856	856	856	856	856	856	856	856	856
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	441	859	443	228	24	24	441	859	443	228	24	24
Volume Left	12	0	0	0	18	18	12	0	0	0	18	18
Volume Right	0	0	0	0	6	6	0	0	0	0	6	6
gSH	856	1700	1700	1700	238	238	856	1700	1700	1700	238	238
Volumes to Capacity	0.01	0.51	0.26	0.13	0.10	0.10	0.01	0.51	0.26	0.13	0.10	0.10
Queue Length 95th (m)	0.3	0.0	0.0	0.0	2.5	2.5	0.3	0.0	0.0	0.0	2.5	2.5
Control Delay (s)	0.4	0.0	0.0	0.0	21.9	21.9	0.4	0.0	0.0	0.0	21.9	21.9
Lane LOS	A	A	A	A	C	C	A	A	A	A	C	C
Approach Delay (s)	0.1	0.0	0.0	0.0	21.9	21.9	0.1	0.0	0.0	0.0	21.9	21.9
Approach LOS	C	C	C	C	C	C	C	C	C	C	C	C
Intersection Summary												
Average Delay	0.4											
Intersection Capacity Utilization	48.8%											
ICU Level of Service	A											
Analysis Period (min)	15											

5: Gerrard Street E & Nasmith Avenue

10-25-2022

2032 Future Background PM Model (ExRN)

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4A	4A	4A	4A	W	W
Traffic Volume (veh/h)	5	1105	565	5	5	5
Future Volume (Veh/h)	5	1105	565	5	5	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	6	1270	649	6	6	6
Pedestrians					60	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					5	
Right turn flare (veh)						
Median type		None		None		
Median storage (veh)						
Upstream signal (m)		145	68			
pX platoon unblocked	0.95			0.88	0.95	
VC, conflicting volume	715			1359	388	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	589			860	244	
IC, single (s)	4.1			6.8	6.9	
IC, 2 stage (s)	2.2			3.5	3.3	
p0 queue free %	99			98	99	
pM capacity (veh/h)	901			249	690	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	429	847	433	222	12	
Volume Left	6	0	0	0	6	
Volume Right	0	0	0	0	6	
cSH	901	1700	1700	1700	366	
Volumes to Capacity	0.01	0.50	0.25	0.13	0.03	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.8	
Control Delay (s)	0.2	0.0	0.0	0.0	15.2	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		15.2	
Approach LOS					C	
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	44.0%					
Analysis Period (min)	15					
ICU Level of Service	A					

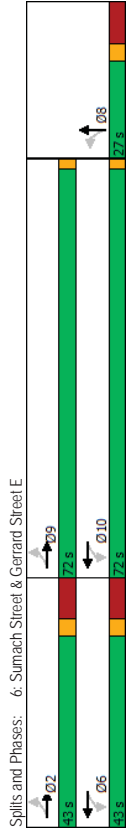
6: Summach Street & Gerrard Street E

10-25-2022

2032 Future Background PM Model (ExRN)

10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	4A	4A	4A	4A	4A				
Traffic Volume (vph)	10	1080	20	550	25				
Future Volume (vph)	10	1080	20	550	25				
Turn Type	Perm	NA	Perm	NA	NA				
Protected Phases	2.9		6.10		8	2	6	9	10
Permitted Phases	2.9	2.9	6.10	6.10	8				
Detector Phase									
Switch Phase									
Minimum Initial (s)			15.0	11.0	11.0	5.0	5.0	5.0	5.0
Minimum Split (s)			25.4	26.2	26.2	7.20	7.20	7.20	7.20
Total Split (s)			27.0	43.0	43.0	7.20	7.20	7.20	7.20
Total Split (%)			19.0%	30%	30%	30%	51%	51%	51%
Yellow Time (s)			3.0	3.0	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)			7.4	7.2	7.2	0.0	0.0	0.0	0.0
Lost Time Adjust (s)			-1.0						
Total Lost Time (s)			9.4						
Lead-Lag									
Lead-Lag Optimize?									
Recall Mode			None	Min	Min	None	None	None	None
Act Effct Green (s)	61.2		61.2	16.0					
Actuated g/C Ratio	0.64		0.64	0.17					
v/C Ratio	0.55		0.37	0.36					
Control Delay	10.7		8.3	29.2					
Queue Delay	0.0		0.0	0.0					
Total Delay	10.7		8.3	29.2					
LOS	B		A	C					
Approach Delay	10.7		8.3	29.2					
Approach LOS	B		A	C					
Intersection Summary									
Cycle Length	142								
Actuated Cycle Length	95.9								
Natural Cycle	725								
Control Type	Actuated-Uncoordinated								
Maximum v/C Ratio	0.55								
Intersection Signal Delay	10.9								
Intersection Capacity Utilization	67.3%								
Analysis Period (min)	15								



Queues 2032 Future Background PM Model (ExRN) 10-25-2022

6: Sumach Street & Gerrard Street E 2032 Future Background PM Model (ExRN) 10-25-2022

	EBT	WBT	NBT
Lane Group	1144	676	104
Lane Group Flow (vph)	0.55	0.37	0.36
v/c Ratio	10.7	8.3	29.2
Control Delay	0.0	0.0	0.0
Queue Delay	10.7	8.3	29.2
Total Delay	55.7	26.5	11.7
Queue Length 50th (m)	70.9	36.0	27.5
Queue Length 95th (m)	44.0	75.7	30.7
Internal Link Dist (m)			
Turn Bay Length (m)			
Base Capacity (vph)	3225	2698	311
Station Cap Reductn	242	0	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.38	0.25	0.33
<b>Intersection Summary</b>			

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Movement											
Lane Configurations		4TB		4TB				4TB			
Traffic Volume (vph)	10	1080	20	20	550	85	20	25	55	0	0
Future Volume (vph)	10	1080	20	20	550	85	20	25	55	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5
Total Lost time (s)		9.2		9.2				9.4			
Lane Util. Factor		0.95		0.95				1.00			
Frbp. ped/bikes		0.99		0.96				0.95			
Frbp. ped/bikes		1.00		1.00				0.97			
Frt		1.00		0.98				0.93			
Frt Protected		1.00		1.00				0.99			
Sat'd. Flow (prot)		3466		3239				1593			
Flt Permitted		0.95		0.89				0.99			
Sat'd. Flow (perm)		3285		2886				1593			
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	10	1113	21	21	567	88	21	26	57	0	0
RTOR Reduction (vph)	0	1	0	0	9	0	0	29	0	0	0
Lane Group Flow (vph)	0	1143	0	0	667	0	0	75	0	0	0
Confl. Peds. (#/hr)	95	95	95	95	95	80	35	35	35	80	80
Confl. Bikes (#/hr)		75		35				5			
Heavy Vehicles (%)	0%	2%	0%	0%	4%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	2.9		6.10		8						
Permitted Phases	2.9		6.10		8						
Actuated Green, C (s)	68.5		68.5		15.0						
Effective Green, g (s)	69.5		69.5		16.0						
Actuated g/C Ratio	0.72		0.72		0.17						
Clearance Time (s)					10.4						
Vehicle Extension (s)					3.0						
Lane Grp Cap (vph)	2380		2091		265						
v/s Ratio Prot											
v/s Ratio Perm	0.35		0.23		0.05						
v/c Ratio	0.48		0.32		0.28						
Uniform Delay, d1	5.6		4.7		34.9						
Progression Factor	1.00		1.00		1.00						
Incremental Delay, d2	0.2		0.1		0.6						
Delay (s)	5.7		4.8		35.5						
Level of Service	A		A		D						
Approach Delay (s)	5.7		4.8		35.5						0.0
Approach LOS	A		A		D						A
<b>Intersection Summary</b>											
HCM 2000 Control Delay	7.0 HCM 2000 Level of Service										
HCM 2000 Volume to Capacity ratio	0.49										
Actuated Cycle Length (s)	95.9 Sum of lost time (s)										
Intersection Capacity Utilization	67.3% ICU Level of Service										
Analysis Period (min)	15										
c Critical Lane Group											

7: Gerrard Street E & Sword Street

10-25-2022

10-25-2022

2032 Future Background PM Model (ExRN)

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔↔	↔↔	↔↔	↔↔	↔↔	↔↔
Traffic Volume (veh/h)	0	1135	645	0	15	10
Future Volume (Veh/h)	0	1135	645	0	15	10
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	1261	717	0	17	11
Pedestrians					65	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					5	
Right turn flare (veh)						
Median type		None				
Median storage (veh)						
Upstream signal (m)		100	91			
pX platoon unblocked		0.93		0.90	0.93	
VC conflicting volume		782		1412	424	
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
IC single (s)		618		887	233	
IC 2 stage (s)		4.1		6.8	7.1	
p0 queue free %		2.2		3.5	3.4	
IC (s)		100		93	98	
CM capacity (veh/h)		860		245	665	
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	630	630	358	358	28	28
Volume Left	0	0	0	0	17	11
Volume Right	0	0	0	0	0	0
cSH	1700	1700	1700	1700	326	326
Volumes to Capacity	0.37	0.37	0.21	0.21	0.09	0.09
Queue Length 95th (m)	0.0	0.0	0.0	0.0	2.1	2.1
Control Delay (s)	0.0	0.0	0.0	0.0	17.1	17.1
Lane LOS					C	C
Approach Delay (s)	0.0	0.0	0.0	0.0	17.1	17.1
Approach LOS					C	C
Intersection Summary						
Average Delay	0.2					
Intersection Capacity Utilization	41.4%					
Analysis Period (min)	15					
ICU Level of Service	A					



8: River Street & Gerrard Street E

10-25-2022

10-25-2022

2032 Future Background PM Model (ExRN)

10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Lane Configurations	↔↔	↔↔	↔↔	↔↔	↔↔	↔↔	↔↔	↔↔	↔↔			
Traffic Volume (vph)	225	855	80	385	70	340	165	450	190			
Future Volume (vph)	225	855	80	385	70	340	165	450	190			
Turn Type	custom	NA	Perm	NA	Perm	NA	Perm	NA	Perm			
Protected Phases	5	52.9	6.10	8.12	8.12	4.11	4.11	4.11	4.11			
Detector Phase	5	5.2	6	6	8	8	4	4	4			
Switch Phase												
Minimum Initial (s)		6.0								23.0	19.0	23.0
Minimum Split (s)		10.0								33.8	30.0	33.8
Total Split (s)		12.0								48.0	43.0	36.0
Total Split (%)		10.1%								40%	36%	30%
Yellow Time (s)		3.0								3.0	3.0	3.0
All-Red Time (s)		1.0								7.8	3.0	7.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag		Lead										
Lead-Lag Optimize?		Yes										
Recall Mode		None										
Act Effct Green (s)		55.6		36.5	39.0	39.0	39.0	39.0	39.0			
Actuated g/C Ratio		0.54		0.36	0.38	0.38	0.38	0.38	0.38			
W/C Ratio		0.90		0.91	0.36	0.77	1.00	0.67	0.30			
Control Delay		29.9		50.0	31.3	36.8	104.7	33.0	4.6			
Queue Delay		17.6		0.0	0.0	0.0	0.0	0.0	0.0			
Total Delay		47.5		50.0	31.3	36.8	104.7	33.0	4.6			
LOS		D		D	C	D	F	C	A			
Approach Delay		47.5		50.0	36.1	41.0						
Approach LOS		D		D	D	D						
Intersection Summary												
Cycle Length	119											
Actuated Cycle Length	102.7											
Natural Cycle	105											
Control Type	Actuated-Uncoordinated											
Maximum W/C Ratio	1.00											
Intersection Signal Delay	44.2											
Intersection Capacity Utilization	116.3%											
Analysis Period (min)	15											



Timings 2032 Future Background PM Model (ExRN) 10-25-2022

Queues 2032 Future Background PM Model (ExRN) 10-25-2022

8: River Street & Gerrard Street E

	08	09	010	011	012
Lane Group					
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Turn Type					
Protected Phases					
Permitted Phases					
Detector Phase					
Switch Phase					
Minimum Initial (s)	19.0	5.0	5.0	5.0	5.0
Minimum Split (s)	30.0	18.0	18.0	10.0	10.0
Total Split (s)	43.0	18.0	18.0	10.0	10.0
Total Split (%)	36%	15%	15%	8%	8%
Yellow Time (s)	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)	3.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	Min	None	None	None	None
Act Effct Green (s)					
Actuated g/C Ratio					
v/c Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Intersection Summary					

8: River Street & Gerrard Street E

	EBT	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group							
Lane Group Flow (vph)	1198	593	73	521	172	469	198
v/c Ratio	0.90	0.91	0.36	0.77	1.00	0.67	0.30
Control Delay	29.9	50.0	31.3	36.8	104.7	33.0	4.6
Queue Delay	17.6	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.5	50.0	31.3	36.8	104.7	33.0	4.6
Queue Length 50th (m)	81.6	55.3	11.6	96.9	-41.0	86.1	0.0
Queue Length 95th (m)	#140.0	#98.4	24.8	138.5	#82.6	121.3	14.0
Internal Link Dist (m)	67.0	81.6		126.5		61.7	
Turn Bay Length (m)			30.0				
Base Capacity (vph)	1335	650	211	697	178	730	678
Starvation Cap Reductn	165	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	1.02	0.91	0.35	0.75	0.97	0.64	0.29
Intersection Summary							
-	Volume exceeds capacity, queue is theoretically infinite.						
-	Queue shown is maximum after two cycles.						
#	95th percentile volume exceeds capacity, queue may be longer.						
-	Queue shown is maximum after two cycles.						

8: River Street & Gerrard Street E

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN) 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	225	855	70	80	385	105	70	340	160	165	450	190
Future Volume (vph)	225	855	70	80	385	105	70	340	160	165	450	190
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	3.0	9.8	5.0	5.0	9.8	5.0	5.0	9.8	5.0	5.0	9.8	5.0
Lane Util. Factor	0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frbp. ped/bikes	0.99	0.98	1.00	0.99	0.98	1.00	0.98	1.00	0.99	0.98	1.00	0.96
Frbp. ped/bikes	1.00	1.00	1.00	0.99	1.00	1.00	0.98	1.00	0.98	1.00	1.00	1.00
Frt	0.99	0.99	1.00	0.97	1.00	0.95	1.00	0.95	1.00	1.00	0.85	1.00
Frt Protected	0.99	0.99	1.00	0.99	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00
Sat'd. Flow (prot)	3383	3271	1625	1742	1643	1860	1424					
FIL Permitted	0.67	0.54	0.31	1.00	0.26	1.00	1.00					
Sat'd. Flow (perm)	2281	1788	536	1742	456	1860	1424					
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	234	891	73	83	401	109	73	354	167	172	469	198
RTOR Reduction (vph)	0	4	0	0	15	0	0	14	0	0	0	114
Lane Group Flow (vph)	0	1194	0	0	578	0	73	507	0	172	469	84
Confl. Peds. (#/hr)	40	80	80	40	15	40	40	15	40	40	15	40
Confl. Bikes (#/hr)			35			15		15		15		10
Heavy Vehicles (%)	3%	2%	3%	0%	4%	0%	3%	0%	1%	1%	1%	2%
Turn Type	custom	MA	Perm	NA	Perm	NA	Perm	NA	Perm	MA	Perm	NA
Protected Phases	5	5.29	6.10	6.10	8.12	8.12	4.11			4.11		4.11
Permitted Phases	2.9		6.10		8.12	43.6	43.6	43.6	43.6	43.6	43.6	43.6
Effective Green, G (s)	57.4	48.6	46.3	44.6	44.6	44.6	44.6	44.6	44.6	44.6	44.6	44.6
Actuated Green, g (s)	48.6	46.3	44.6	44.6	44.6	44.6	44.6	44.6	44.6	44.6	44.6	44.6
Actuated g/C Ratio	0.46	0.44	0.44	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1151	788	227	739	193	790	604					
v/s Ratio Prot	c0.09			0.29		0.25						
v/s Ratio Perm	c0.39			0.14		0.38						
v/c Ratio	1.04	0.73	0.32	0.69	0.89	0.59	0.14					
Uniform Delay, d1	28.2	24.3	20.1	24.5	28.0	23.2	18.5					
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00					
Incremental Delay, d2	36.6	3.6	0.8	2.7	36.2	1.2	0.1					
Delay (s)	64.8	27.8	20.9	27.2	64.1	24.4	18.6					
Level of Service	E	C	C	C	E	C	B					
Approach Delay (s)	64.8	27.8	26.4		31.2							
Approach LOS	E	C	C		C							
Intersection Summary												
HCM 2000 Control Delay	42.2 HCM 2000 Level of Service D											
HCM 2000 Volume to Capacity ratio	1.02											
Actuated Cycle Length (s)	106.0 Sum of lost time (s) 19.8											
Intersection Capacity Utilization	116.3% ICU Level of Service H											
Analysis Period (min)	15											
c Critical Lane Group												

9: Sackville Street & Site Driveway

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN) 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	10	0	0	5	0	5	0	20	0	5	85	0
Future Volume (Veh/h)	10	0	0	5	0	5	0	20	0	5	85	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	12	0	0	6	0	6	0	24	0	6	100	0
Pedestrians	60			35				5			15	
Lane Width (m)	3.5			3.5				3.5			3.5	
Walking Speed (m/s)	1.1			1.1				1.1			1.1	
Percent Blockage	5			3				0			1	
Right turn flare (veh)												
Median type	None											
Median storage (veh)												
Upstream signal (m)												
PK. Platoon unblocked												
v/c, conflicting volume	217	231	165	176	231	74	160				59	
v/c1, stage 1 conf vol												
v/c2, stage 2 conf vol												
v/cU, unblocked vol	217	231	165	176	231	74	160				59	
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	98	100	100	99	100	99	100				100	
dM capacity (veh/h)	645	615	834	713	615	950	1356				1510	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	12	12	24	106								
Volume Left	12	6	0	6								
Volume Right	0	6	0	0								
CSH	645	814	1356	1510								
Volumes to Capacity	0.02	0.01	0.00	0.00								
Queue Length 95th (m)	0.4	0.3	0.0	0.1								
Control Delay (s)	10.7	9.5	0.0	0.4								
Lane LOS	B	A	A	A								
Approach Delay (s)	10.7	9.5	0.0	0.4								
Approach LOS	B	A	A	A								
Intersection Summary												
Average Delay	1.9											
Intersection Capacity Utilization	26.6%											
ICU Level of Service	A											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
10-25-2022  
10: Parliament Street & Oak Street

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		W			W
Traffic Volume (veh/h)	35	50	495	25	25	370
Future Volume (Veh/h)	35	50	495	25	25	370
Sign Control	Stop		Free		Free	Free
Grade	0%		0%		0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	41	59	582	29	29	435
Pedestrians	200		5			55
Lane Width (m)	3.0	3.5	3.5			3.5
Walking Speed (m/s)	1.1	1.1	1.1			1.1
Percent Blockage			0			5
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			151			143
PX platoon unblocked	0.93	0.93			0.93	
VC conflicting volume	1097	580			831	
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol	956	401			670	
IC single (s)	6.8	6.9			4.3	
IC 2 stage (s)						
IF (s)	3.5	3.3			2.3	
p0 queue free %	79	87			96	
CM capacity (veh/h)	192	446			661	
Direction Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volumes Total	100	388	223	174	290	
Volume Left	41	0	0	29	0	
Volume Right	59	0	29	0	0	
CSH	289	1700	1700	661	1700	
Volumes to Capacity	0.35	0.23	0.13	0.04	0.17	
Queue Length 95th (m)	11.4	0.0	0.0	1.0	0.0	
Control Delay (s)	23.9	0.0	0.0	2.2	0.0	
Lane LOS	C			A		
Approach Delay (s)	23.9	0.0		0.8		
Approach LOS	C					
Intersection Summary						
Average Delay	2.4					
Intersection Capacity Utilization	48.2%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
10-25-2022  
11: Oak Street & Dreamers Way

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			W		W	W
Traffic Volume (veh/h)	0	55	35	0	15	45
Future Volume (Veh/h)	0	55	35	0	15	45
Sign Control		Free	Free	Stop	Stop	Stop
Grade		0%	0%	0%	0%	0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	59	38	0	16	48
Pedestrians		25	10		40	
Lane Width (m)		3.5	3.5		3.0	
Walking Speed (m/s)		1.1	1.1		1.1	
Percent Blockage		2	1		3	
Right turn flare (veh)						
Median type		None	None		None	
Median storage (veh)						
Upstream signal (m)						
PX platoon unblocked					147	103
VC conflicting volume	78					
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol	78				147	103
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)						
IF (s)	2.2				3.5	3.3
p0 queue free %	100				98	95
CM capacity (veh/h)	1487				817	908
Direction Lane #	EB 1	WB 1	SB 1			
Volumes Total	59	38	64			
Volume Left	0	0	16			
Volume Right	0	0	48			
CSH	1700	1700	883			
Volumes to Capacity	0.03	0.02	0.07			
Queue Length 95th (m)	0.0	0.0	1.8			
Control Delay (s)	0.0	0.0	9.4			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	9.4			
Approach LOS			A			
Intersection Summary						
Average Delay	3.7					
Intersection Capacity Utilization	27.2%					
ICU Level of Service	A					
Analysis Period (min)	15					



HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
10-25-2022  
12: Regent Street & Oak Street

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑	↑	↑
Traffic Volume (veh/h)	70	0	0	20	15	45
Future Volume (Veh/h)	70	0	0	20	15	45
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	84	0	0	24	18	54
Pedestrians	15			15	40	
Lane Width (m)	3.5			3.5	3.0	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	1			1	3	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume		124			163	139
VC1 stage 1 conf vol						
VC2 stage 2 conf vol		124			163	139
VCU unblocked vol		4.1			6.4	6.2
IC single (s)						
IC 2 stage (s)		2.2			3.5	3.3
p0 queue free %		100			98	94
ICM capacity (veh/h)		1431			7%	875
Direction_Lane #	EB 1	WB 1	NB 1			
Volumes Total	84	24	72			
Volume Left	0	0	18			
Volume Right	0	0	54			
cSH	1700	1700	854			
Volumes to Capacity	0.05	0.01	0.08			
Queue Length 95th (m)	0.0	0.0	2.1			
Control Delay (s)	0.0	0.0	9.6			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.6			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			3.8			
Intersection Capacity Utilization			27.2%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
10-25-2022  
13: Oak Street & Site Driveway

Movement	EBL	EBT	WBT	WBL	SBL	SBR
Lane Configurations		↑	↑	↑	↑	↑
Traffic Volume (veh/h)	10	105	20	0	5	0
Future Volume (Veh/h)	10	105	20	0	5	0
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	12	127	24	0	6	0
Pedestrians					35	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					1.1	
Right turn flare (veh)					3	
Median type	None				None	
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume		59			275	59
VC1 stage 1 conf vol						
VC2 stage 2 conf vol		59			275	59
VCU unblocked vol		4.1			6.4	6.2
IC single (s)						
IC 2 stage (s)		2.2			3.5	3.3
p0 queue free %		99			99	100
ICM capacity (veh/h)		1516			654	986
Direction_Lane #	EB 1	WB 1	SB 1			
Volumes Total	139	24	6			
Volume Left	12	0	6			
Volume Right	0	0	0			
cSH	1516	1700	654			
Volumes to Capacity	0.01	0.01	0.01			
Queue Length 95th (m)	0.2	0.0	0.2			
Control Delay (s)	0.7	0.0	10.6			
Lane LOS	A	A	B			
Approach Delay (s)	0.7	0.0	10.6			
Approach LOS	B	B	B			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			22.7%		ICU Level of Service	A
Analysis Period (min)			15			

14: Sackville Street & Oak Street

15: Sumach Street & Oak Street

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop		Stop		Stop		Stop		Stop		Stop	
Traffic Volume (vph)	20	75	20	5	5	0	0	0	0	0	10	70
Future Volume (vph)	20	75	20	5	5	0	0	0	0	0	10	70
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	22	83	22	6	6	0	0	0	0	0	11	78
Direction, Lane #	EB 1	WB 1	SB 1									
Volume Total (vph)	127	12	95									
Volume Left (vph)	22	6	11									
Volume Right (vph)	22	0	6									
Head (s)	-0.05	0.27	0.03									
Departure Headway (s)	4.1	4.5	4.2									
Degree Utilization, x	0.14	0.02	0.11									
Capacity (veh/h)	859	773	818									
Control Delay (s)	7.8	7.6	7.8									
Approach Delay (s)	7.8	7.6	7.8									
Approach LOS	A	A	A									
Intersection Summary												
Delay	7.8											
Level of Service	A											
Intersection Capacity Utilization	32.4%		ICU Level of Service		A							
Analysis Period (min)	15											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop		Stop		Stop		Stop		Stop		Stop	
Traffic Volume (vph)	20	55	10	0	0	0	5	80	75	15	35	5
Future Volume (vph)	20	55	10	0	0	0	5	80	75	15	35	5
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	22	60	11	0	0	0	5	88	82	16	38	5
Direction, Lane #	EB 1	NB 1	SB 1									
Volume Total (vph)	93	175	59									
Volume Left (vph)	22	5	16									
Volume Right (vph)	11	82	5									
Head (s)	0.05	-0.20	0.13									
Departure Headway (s)	4.4	4.0	4.4									
Degree Utilization, x	0.11	0.19	0.07									
Capacity (veh/h)	768	875	789									
Control Delay (s)	8.0	7.9	7.8									
Approach Delay (s)	8.0	7.9	7.8									
Approach LOS	A	A	A									
Intersection Summary												
Delay	7.9											
Level of Service	A											
Intersection Capacity Utilization	32.4%		ICU Level of Service		A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
10-25-2022  
16: Tubman Avenue & Oak Street



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	T					
Traffic Volume (veh/h)	135	10	0	0	0	0
Future Volume (Veh/h)	135	10	0	0	0	0
Sign Control	Free					
Grade	0%					
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	145	11	0	0	0	0
Pedestrians	20					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.1					
Percent Blockage	2					
Right turn flare (veh)	None					
Median type	None					
Median storage (veh)	None					
Upstream signal (m)	None					
PX platoon unblocked	186					
VC conflicting volume	200					
VC1 stage 1 conf vol	186					
VC2 stage 2 conf vol	186					
VCU unblocked vol	4.1					
IC single (s)	2.2					
IC 2 stage (s)	3.5					
p0 queue free %	100					
CM capacity (veh/h)	1401					
Direction_Lane #	EB 1					
Volume Total	156					
Volume Left	0					
Volume Right	11					
GSH	1700					
Volumes to Capacity	0.09					
Queue Length 95th (m)	0.0					
Control Delay (s)	0.0					
Lane LOS	A					
Approach Delay (s)	0.0					
Approach LOS	A					
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	22.9%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
10-25-2022  
17: Oak Street & Site Driveway



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	T					
Traffic Volume (veh/h)	5	130	0	0	10	0
Future Volume (Veh/h)	5	130	0	0	10	0
Sign Control	Free					
Grade	0%					
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	6	148	0	0	11	0
Pedestrians	5					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.1					
Percent Blockage	0					
Right turn flare (veh)	2					
Median type	None					
Median storage (veh)	None					
Upstream signal (m)	None					
PX platoon unblocked	25					
VC conflicting volume	190					
VC1 stage 1 conf vol	25					
VC2 stage 2 conf vol	25					
VCU unblocked vol	4.1					
IC single (s)	2.2					
IC 2 stage (s)	3.5					
p0 queue free %	100					
CM capacity (veh/h)	1572					
Direction_Lane #	EB 1 SB 1					
Volume Total	154					
Volume Left	6					
Volume Right	0					
GSH	1572					
Volumes to Capacity	0.00					
Queue Length 95th (m)	0.1					
Control Delay (s)	0.3					
Lane LOS	A					
Approach Delay (s)	0.3					
Approach LOS	A					
Intersection Summary						
Average Delay	0.9					
Intersection Capacity Utilization	22.4%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
18: River Street & Oak Street

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations											
Traffic Volume (veh/h)	85	5	50	20	0	50	0	435	40	50	550
Future Volume (Veh/h)	85	5	50	20	0	50	0	435	40	50	550
Sign Control	Stop	0%	Stop	0%	0%	Free	0%	Free	0%	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	92	5	54	22	0	54	0	473	43	54	598
Pedestrians	75			45			50				5
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	7			4			4				0
Right turn flare (veh)											
Median type							None				None
Median storage (veh)											
Upstream signal (m)							14.3				151
pX platoon unblocked	0.85	0.85	0.80	0.85	0.85	0.90	0.80	0.80	0.80	0.90	0.90
VC, conflicting volume	1334	1342	723	1352	1320	544	673				561
VC1, stage 1 conf vol											
VC2, stage 2 conf vol											
VCU, unblocked vol	1047	1056	527	1068	1031	442	464				460
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1
IC, 2 stage (s)											
p0 queue free %	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2
IF (s)	29	97	86	81	100	90	100				94
p0 capacity (veh/h)	130	163	389	116	169	535	826				964
Direction, Lane #	EB 1	WB 1	NB 1	SB 1							
Volumes Total	151	76	516	652							
Volume Left	92	22	0	54							
Volume Right	54	54	43	0							
cSH	172	262	1700	964							
Volumes to Capacity	0.88	0.29	0.30	0.06							
Queue Length 95th (m)	48.1	8.9	0.0	1.4							
Control Delay (s)	93.5	24.3	0.0	1.5							
Lane LOS	F	C	A	A							
Approach Delay (s)	93.5	24.3	0.0	1.5							
Approach LOS	F	C	C	C							
Intersection Summary											
Average Delay			12.1								
Intersection Capacity Utilization			83.5%								E
Analysis Period (min)			15								

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
19: Parliament Street & Cole Street

10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	0	520	20	25	380
Future Volume (Veh/h)	0	0	520	20	25	380
Sign Control	Stop	0%	Free	0%	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	0	0	619	24	30	452
Pedestrians	255		25			15
Lane Width (m)	0.0	0.0	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	0		2			1
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			80			214
pX platoon unblocked	0.89	0.89	0.89	0.89	0.89	0.89
VC, conflicting volume	1197	592				898
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	983	306				649
IC, single (s)	6.8	6.9				4.1
IC, 2 stage (s)						
p0 queue free %	3.5	3.3				2.2
IF (s)	100	100				96
p0 capacity (veh/h)	210	614				846
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	413	230	181	301		
Volume Left	0	0	30	0		
Volume Right	0	24	0	0		
cSH	1700	1700	846	1700		
Volumes to Capacity	0.24	0.14	0.04	0.18		
Queue Length 95th (m)	0.0	0.0	0.8	0.0		
Control Delay (s)	0.0	0.0	1.9	0.0		
Lane LOS	A	A	A	A		
Approach Delay (s)	0.0	0.0	0.7	0.0		
Approach LOS						
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			43.3%			A
Analysis Period (min)			15			

Timings 2032 Future Background PM Model (ExRN) 10-25-2022

Queues 2032 Future Background PM Model (ExRN) 10-25-2022

20: Parliament Street & Dundas Street E

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4TB	4TB	4TB	4TB	4TB	4TB	4TB	4TB
Traffic Volume (vph)	65	490	45	195	45	405	40	320
Future Volume (vph)	65	490	45	195	45	405	40	320
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2		6		8		8	4
Permitted Phases	2	2	6	6	8	8	4	4
Detector Phase								
Switch Phase								
Minimum Initial (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Minimum Split (s)	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	47.0	47.0	47.0	47.0	47.0	47.0	47.0	47.0
Total Split (%)	61.8%	61.8%	61.8%	61.8%	38.2%	38.2%	38.2%	38.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0

20: Parliament Street & Dundas Street E

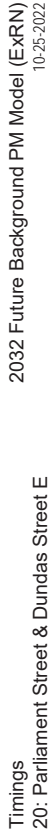
	EBT	WBT	NBT	SBT
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	691	349	551	427
v/c Ratio	0.45	0.26	0.59	0.48
Control Delay	11.4	12.6	23.7	22.1
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	11.4	12.6	23.7	22.1
Queue Length 50th (m)	27.3	10.3	34.0	25.3
Queue Length 95th (m)	43.2	28.5	45.3	35.0
Internal Link Dist (m)	85.0	103.5	45.0	56.1
Turn Bay Length (m)				
Base Capacity (vph)	1557	1354	937	892
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.44	0.26	0.59	0.48

20: Parliament Street & Dundas Street E

	C-Min	C-Min	C-Min	Min	Min
Lead/Lag Optimize?					
Recall Mode					
Act Effct Green (s)	41.3	41.3	24.7	24.7	
Actuated g/C Ratio	0.54	0.54	0.32	0.32	
v/c Ratio	0.45	0.26	0.59	0.48	
Control Delay	11.4	12.6	23.7	22.1	
Queue Delay	0.0	0.0	0.0	0.0	
Total Delay	11.4	12.6	23.7	22.1	
LOS	B	B	C	C	
Approach Delay	11.4	12.6	23.7	22.1	
Approach LOS	B	B	C	C	

Intersection Summary

Cycle Length: 76
Actuated Cycle Length: 76
Offset: 9 (12%) Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.59
Intersection Signal Delay: 17.2
Intersection Capacity Utilization 93.3%
Analysis Period (min) 15



Intersection Summary

Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	691	349	551	427
v/c Ratio	0.45	0.26	0.59	0.48
Control Delay	11.4	12.6	23.7	22.1
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	11.4	12.6	23.7	22.1
Queue Length 50th (m)	27.3	10.3	34.0	25.3
Queue Length 95th (m)	43.2	28.5	45.3	35.0
Internal Link Dist (m)	85.0	103.5	45.0	56.1
Turn Bay Length (m)				
Base Capacity (vph)	1557	1354	937	892
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.44	0.26	0.59	0.48

Intersection Summary

Cycle Length: 76
Actuated Cycle Length: 76
Offset: 9 (12%) Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.59
Intersection Signal Delay: 17.2
Intersection Capacity Utilization 93.3%
Analysis Period (min) 15



HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 20: Parliament Street & Dundas Street E

10-25-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	65	490	60	45	195	70	45	405	40	40	320	20
Future Volume (vph)	65	490	60	45	195	70	45	405	40	40	320	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0			5.0			5.0			5.0		5.0
Lane Util. Factor	0.95			0.95			0.95			0.95		0.95
Frbp. ped/bikes	0.98			0.94			0.98			0.99		0.99
Fibp. ped/bikes	0.98			0.99			0.99			0.99		0.99
Frt	0.99			0.97			1.00			0.99		0.99
FIL Protected												
Sat'd. Flow (prot)	3205			3013			3272			3214		3214
FIL Permitted	0.87			0.80			0.87			0.85		0.85
Sat'd. Flow (perm)	2799			2434			2858			2732		2732
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	73	551	67	51	219	79	51	455	45	45	360	22
RTOR Reduction (vph)	0	11	0	0	9	0	0	9	0	0	5	0
Lane Group Flow (vph)	0	680	0	0	340	0	0	542	0	0	422	0
Confl. Peds. (#/hr)	260			155			260			225		225
Confl. Bikes (#/hr)	25			25			20			5		10
Heavy Vehicles (%)	3%	6%	5%	5%	6%	6%	3%	5%	0%	3%	8%	10%
Parking (#/hr)	0			0			0			0		0
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA
Permitted Phases	2			6			6			8		4
Permitted Phases	2			6			8			4		4
Actuated Green, G (s)	40.3			40.3			23.7			23.7		23.7
Effective Green, g (s)	41.3			41.3			24.7			24.7		24.7
Actuated g/C Ratio	0.54			0.54			0.32			0.32		0.32
Clearance Time (s)	6.0			6.0			6.0			6.0		6.0
Vehicle Extension (s)	3.0			3.0			3.0			3.0		3.0
Lane Grp Cap (vph)	1521			1322			928			887		887
v/s Ratio Prot												
v/s Ratio Perm	0.24			0.14			0.19			0.15		0.15
v/C Ratio	0.45			0.26			0.58			0.48		0.48
Uniform Delay, d1	10.5			9.2			21.4			20.5		20.5
Progression Factor	1.00			1.34			1.00			1.00		1.00
Incremental Delay, d2	1.0			0.5			0.9			0.4		0.4
Delay (s)	11.4			12.8			22.3			20.9		20.9
Level of Service	B			B			C			C		C
Approach Delay (s)	11.4			12.8			22.3			20.9		20.9
Approach LOS	B			B			C			C		C
Intersection Summary												
HCM 2000 Control Delay	16.6			HCM 2000 Level of Service		B						
HCM 2000 Volume to Capacity ratio	0.50											
Actuated Cycle Length (s)	76.0	Sum of lost time (s)										
Intersection Capacity Utilization	93.3%	ICU Level of Service										
Analysis Period (min)	15	F										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 21: Regent Street & Dundas Street E

10-25-2022

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	40	500	15	5	300	25	5	10	5	5	5	25
Future Volume (Veh/h)	40	500	15	5	300	25	5	10	5	5	5	25
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%			0%			0%			0%		0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	43	538	16	5	323	27	5	11	5	5	5	27
Pedestrians	30			10			125			135		135
Lane Width (m)	3.5			3.5			3.5			3.5		3.5
Walking Speed (m/s)	1.1			1.1			1.1			1.1		1.1
Percent Blockage	3			1			11			12		12
Right turn flare (veh)												
Median type	None			None			None			None		None
Median storage (veh)												
Upstream signal (m)	127			124								
pX platoon unblocked				0.95			0.95			0.95		0.95
VC, conflicting volume	485			679			988			1252		412
VC1, stage 1 conf vol												340
VC2, stage 2 conf vol												
VCU, unblocked vol	485			556			881			1159		274
IC, single (s)	4.1			4.4			7.5			6.5		7.5
IC, 2 stage (s)												
IF (s)	2.2			2.4			3.5			4.0		3.8
p0 queue free %	96			99			97			92		99
d0 capacity (veh/h)	958			777			149			139		500
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1					
Volume Total	312	285	166	188	21	37						
Volume Left	43	0	5	0	5	5						
Volume Right	0	16	0	27	5	27						
CSH	958	1700	777	1700	172	338						
Volumes to Capacity	0.04	0.17	0.01	0.11	0.12	0.11						
Queue Length 95th (m)	1.1	0.0	0.1	0.0	3.1	2.8						
Control Delay (s)	1.6	0.0	0.4	0.0	28.9	17.0						
Lane LOS	A	A	A	A	D	C						
Approach Delay (s)	0.9	0.2	0.2	28.9	17.0							
Approach LOS	D	D	D	C								
Intersection Summary												
Average Delay												1.8
Intersection Capacity Utilization												49.0%
Analysis Period (min)												15
ICU Level of Service												A

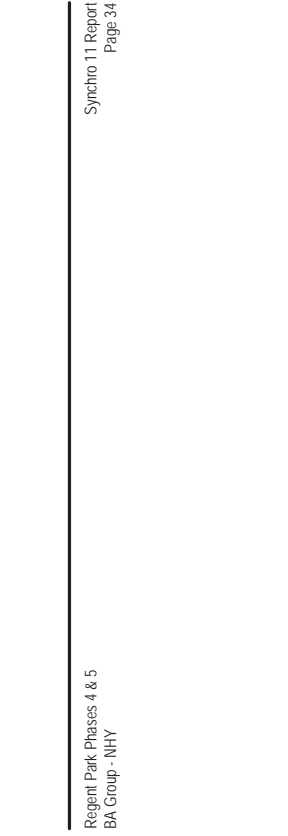
Timings 2032 Future Background PM Model (ExRN) 10-25-2022

Queues 2032 Future Background PM Model (ExRN) 10-25-2022

Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	↑↑	↑	↓	↓	↓	↓	↓
Traffic Volume (vph)	490	15	255	40	0	30	35
Future Volume (vph)	490	15	255	40	0	30	35
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	6	6	8	8	4	4
Permitted Phases	2	6	6	8	8	4	4
Detector Phase							
Switch Phase							
Minimum Initial (s)	17.0	17.0	17.0	7.0	7.0	7.0	7.0
Minimum Split (s)	24.0	24.0	24.0	26.0	26.0	26.0	26.0
Total Split (s)	50.0	50.0	50.0	26.0	26.0	26.0	26.0
Total Split (%)	65.8%	65.8%	65.8%	34.2%	34.2%	34.2%	34.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0

Lead-Lag Optimize?	C-Min	C-Min	C-Min	None	None	None
Recall Mode						
Act Effct Green (s)	47.6	47.6	18.4	18.4		
Actuated g/C Ratio	0.63	0.63	0.24	0.24		
v/c Ratio	0.27	0.27	0.23	0.28		
Control Delay	5.1	7.0	13.2	17.1		
Queue Delay	0.0	0.3	0.0	0.0		
Total Delay	5.1	7.2	13.2	17.1		
LOS	A	A	B	B		
Approach Delay	5.1	7.2	13.2	17.1		
Approach LOS	A	A	B	B		

Intersection Summary
Cycle Length: 76
Actuated Cycle Length: 76
Offset: 25 (33%), Referenced to phase 2EBT and 6WBT, Start of 1st Green
Natural Cycle: 50
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.28
Intersection Signal Delay: 7.6
Intersection Capacity Utilization 49.8%
Analysis Period (min) 15



Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	559	290	75	108
v/c Ratio	0.27	0.27	0.23	0.28
Control Delay	5.1	7.0	13.2	17.1
Queue Delay	0.0	0.3	0.0	0.0
Total Delay	5.1	7.2	13.2	17.1
Queue Length 50th (m)	12.4	18.5	3.4	7.8
Queue Length 95th (m)	16.4	21.7	12.9	19.6
Internal Link Dist (m)	99.8	79.6	36.2	126.8
Turn Bay Length (m)				
Base Capacity (vph)	2058	1063	369	434
Starvation Cap Reductn	0	313	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.27	0.39	0.20	0.25

Intersection Summary
Cycle Length: 76
Actuated Cycle Length: 76
Offset: 25 (33%), Referenced to phase 2EBT and 6WBT, Start of 1st Green
Natural Cycle: 50
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.28
Intersection Signal Delay: 7.6
Intersection Capacity Utilization 49.8%
Analysis Period (min) 15

2032 Future Background PM Model (ExRN)  
 10-25-2022  
 HCM Signalized Intersection Capacity Analysis  
 22: Sackville Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4	4	4	4	4	4	4	4	4	4	4	4
Traffic Volume (vph)	0	490	30	15	255	0	40	0	30	30	30	35
Future Volume (vph)	0	490	30	15	255	0	40	0	30	30	35	35
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.5	3.0	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.98	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.97	0.97	0.97
Frbp. ped/bikes	1.00	0.99	0.99	1.00	0.99	0.96	0.96	0.96	0.96	0.97	0.97	0.97
Frt	1.00	0.99	1.00	1.00	0.94	0.94	0.97	0.97	0.97	0.99	0.99	0.99
Flt Protected	3289	1756	1756	1494	1494	1494	1618	1618	1618	1618	1618	1618
Flt Permitted	1.00	0.96	0.96	0.80	0.80	0.80	0.90	0.90	0.90	0.90	0.90	0.90
Satd. Flow (perm)	3289	1697	1697	1226	1226	1226	1478	1478	1478	1478	1478	1478
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	0	527	32	16	274	0	43	0	32	32	38	38
RTOR Reduction (vph)	0	0	0	0	0	0	33	0	0	0	27	0
Lane Group Flow (vph)	0	559	0	0	290	0	42	0	0	0	81	0
Conf. Peds. (#/hr)	105	175	175	105	70	20	55	55	55	55	55	55
Conf. Bikes (#/hr)	35	35	35	20	20	20	55	55	55	55	55	55
Heavy Vehicles (%)	0%	5%	6%	0%	6%	0%	0%	0%	12%	0%	2%	5%
Turn Type	MA	MA	NA	NA	NA	NA	MA	MA	MA	MA	MA	MA
Protected Phases	2	6	6	6	6	6	8	8	8	8	8	8
Permitted Phases	4	4	4	4	4	4	4	4	4	4	4	4
Actuated Green, G (s)	46.6	46.6	46.6	17.4	17.4	17.4	17.4	17.4	17.4	17.4	17.4	17.4
Effective Green, g (s)	47.6	47.6	47.6	18.4	18.4	18.4	18.4	18.4	18.4	18.4	18.4	18.4
Actuated g/C Ratio	0.63	0.63	0.63	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	2059	1062	296	296	296	296	357	357	357	357	357	357
v/s Ratio Prot	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17	0.17
v/s Ratio Perm	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27
v/c Ratio	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27	0.27
Uniform Delay, d1	6.4	6.4	6.4	22.6	22.6	22.6	23.1	23.1	23.1	23.1	23.1	23.1
Progression Factor	0.67	0.67	0.67	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.3	0.3	0.3	0.6	0.6	0.6	0.3	0.3	0.3	0.3	0.3	0.3
Delay (s)	4.6	4.6	4.6	6.1	6.1	6.1	23.4	23.4	23.4	23.4	23.4	23.4
Level of Service	A	A	A	C	C	C	C	C	C	C	C	C
Approach Delay (s)	4.6	4.6	6.1	22.8	22.8	22.8	23.4	23.4	23.4	23.4	23.4	23.4
Approach LOS	A	A	A	C	C	C	C	C	C	C	C	C
Intersection Summary												
HCM 2000 Control Delay	8.3 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.26											
Actuated Cycle Length (s)	76.0 Sum of lost time (s)											
Intersection Capacity Utilization	49.8% ICU Level of Service A											
Analysis Period (min)	15											
c Critical Lane Group												

2032 Future Background PM Model (ExRN)  
 10-25-2022  
 Timings  
 23: Regent Park Boulevard & Dundas Street E

Lane Group	EBT	WBL	WBT	Ø4
Lane Configurations	4	4	4	4
Traffic Volume (vph)	520	15	270	270
Future Volume (vph)	520	15	270	270
Turn Type	NA	Perm	NA	NA
Protected Phases	2	6	6	4
Permitted Phases	2	6	6	4
Detector Phase	2	6	6	4
Switch Phase	18.0	18.0	18.0	20.0
Minimum Initial (s)	23.9	23.9	23.9	23.0
Minimum Split (s)	53.0	53.0	53.0	23.0
Total Split (%)	69.7%	69.7%	69.7%	30%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	0.0
Total Lost Time (s)	4.9	4.9	4.9	4.9
LeadLag	4.9	4.9	4.9	4.9
Lead-Lag Optimize?	4.9	4.9	4.9	4.9
Recall Mode	C-Min	C-Min	C-Min	None
Ad Effect Green (s)	64.8	64.8	64.8	64.8
Actuated g/C Ratio	0.85	0.85	0.85	0.85
v/c Ratio	0.42	0.22	0.22	0.22
Control Delay	3.1	4.1	4.1	4.1
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	3.2	4.1	4.1	4.1
LOS	A	A	A	A
Approach Delay	3.2	4.1	4.1	4.1
Approach LOS	A	A	A	A
Intersection Summary				
Cycle Length: 76				
Actuated Cycle Length: 76				
Offset: 22 (29%), Referenced to phase 2EBT and 6.WBTL Start of 1st Green				
Natural Cycle: 60				
Control Type: Actuated-Coordinated				
Maximum v/c Ratio: 0.42				
Intersection Signal Delay: 3.5				
Intersection Capacity Utilization 49.2%				
Analysis Period (min) 15				
Spills and Phases: 23: Regent Park Boulevard & Dundas Street E				
Ø2 (R)	53 s	23 s	23 s	23 s
Ø6 (R)	53 s	23 s	23 s	23 s



23: Regent Park Boulevard & Dundas Street E

	EBT	WBT
Lane Group	618	320
Lane Group Flow (vph)	0.42	0.22
v/c Ratio	3.1	4.1
Control Delay	0.0	0.0
Queue Delay	3.2	4.1
Total Delay	1.6	0.0
Queue Length 50th (m)	50.5	29.6
Queue Length 95th (m)	79.6	86.0
Internal Link Dist (m)	1461	1452
Turn Bay Length (m)	36	133
Base Capacity (vph)	0	0
Starvation Cap Reductn	0	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0.43	0.24
Reduced v/c Ratio		
Intersection Summary		

23: Regent Park Boulevard & Dundas Street E

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	EB	EB	WB	WB	NB	NB
Traffic Volume (vph)	520	30	15	270	0	0
Future Volume (vph)	520	30	15	270	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.0	3.0	3.5	3.0	3.0
Total Lost time (s)	4.9			4.9		
Lane Util. Factor	1.00			1.00		
Frbp. ped/bikes	0.97			1.00		
Frbp. psd/bikes	1.00			0.99		
Frt	0.99			1.00		
Flt Protected	1.00			1.00		
Satd. Flow (prot)	1710			1755		
Flt Permitted	1.00			0.96		
Satd. Flow (perm)	1770			1698		
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	584	34	17	303	0	0
RTOR Reduction (vph)	2	0	0	0	0	0
Lane Group Flow (vph)	616	0	0	320	0	0
Conf. Peds. (#/hr)	235	235	235	60	65	65
Heavy Vehicles (%)	6%	6%	0%	6%	0%	0%
Turn Type	NA	Perm	NA	NA		
Protected Phases	2		6			
Permitted Phases			6			
Actuated Green, G (s)	59.1		59.1			
Effective Green, g (s)	60.1		60.1			
Actuated g/C Ratio	0.79		0.79			
Clearance Time (s)	5.9		5.9			
Vehicle Extension (s)	3.0		3.0			
Lane Grp Cap (vph)	1352		1342			
v/s Ratio Prot	c0.36					
v/s Ratio Perm		0.19				
v/c Ratio	0.46		0.24			
Uniform Delay, d1	2.6		2.0			
Progression Factor	0.48		1.00			
Incremental Delay, d2	1.1		0.4			
Delay (s)	2.3		2.5			
Level of Service	A		A			
Approach Delay (s)	2.3		2.5	0.0		
Approach LOS	A		A	A		
Intersection Summary						
HCM 2000 Control Delay		2.4			HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio		0.40				
Actuated Cycle Length (s)		76.0			Sum of lost time (s)	7.9
Intersection Capacity Utilization		49.2%			ICU Level of Service	A
Analysis Period (min)		15				
c. Critical Lane Group						

Timings 2032 Future Background PM Model (ExRN) 10-25-2022

Queues 2032 Future Background PM Model (ExRN) 10-25-2022

24: Sumach Street & Dundas Street E

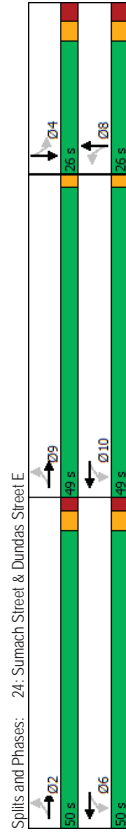
	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø6	Ø9	Ø10
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Lane Configurations	45	460	25	245	25	90	25	20				
Traffic Volume (vph)	45	460	25	245	25	90	25	20				
Future Volume (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA				
Turn Type	2.9	6.10	8	8	4	4	4	4				
Protected Phases	2	2	6	6	8	8	4	4				
Detector Phase												
Switch Phase												
Minimum Initial (s)	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0	16.0	16.0	5.0	5.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0	21.0	21.0	49.0	49.0
Total Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0	50.0	50.0	49.0	49.0
Total Split (%)	20.8%	20.8%	20.8%	20.8%	20.8%	20.8%	20.8%	20.8%	40%	40%	39%	39%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0	0.0	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0				
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Lead/Lag												
Lead-Lag Optimize?	None	None	None	None	None	None	None	None	Min	Min	None	None
Recall Mode												
Act Effct Green (s)	61.0	61.0	61.0	61.0	23.1	23.1	23.1	23.1				
Actuated g/C Ratio	0.65	0.65	0.65	0.65	0.25	0.25	0.25	0.25				
v/c Ratio	0.54	0.36	0.36	0.46	0.19	0.19	0.19	0.19				
Control Delay	9.3	6.7	6.7	40.2	33.4	33.4	33.4	33.4				
Queue Delay	1.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Total Delay	11.1	6.7	6.7	40.2	33.4	33.4	33.4	33.4				
LOS	B	A	A	D	C	C	C	C				
Approach Delay	11.1	6.7	6.7	40.2	33.4	33.4	33.4	33.4				
Approach LOS	B	A	A	D	C	C	C	C				
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 94.1												
Natural Cycle: 100												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.54												
Intersection Signal Delay: 15.4												
Intersection Capacity Utilization 61.6%												
Analysis Period (min) 15												

24: Sumach Street & Dundas Street E

	EBT	WBT	NBT	SBT
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	559	349	172	65
v/c Ratio	0.54	0.36	0.46	0.19
Control Delay	9.3	6.7	40.2	33.4
Queue Delay	1.8	0.0	0.0	0.0
Total Delay	11.1	6.7	40.2	33.4
Queue Length 50th (m)	43.1	21.2	28.5	9.0
Queue Length 95th (m)	62.7	32.8	55.5	22.9
Internal Link Dist (m)	86.0	75.5	76.2	121.5
Turn Bay Length (m)				
Base Capacity (vph)	1220	1126	374	335
Starvation Cap Reductn	475	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.75	0.31	0.46	0.19
Intersection Summary				

24: Sumach Street & Dundas Street E

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	45	460	25	245	25	90	25	20
Traffic Volume (vph)	45	460	25	245	25	90	25	20
Future Volume (vph)	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Turn Type	2.9	6.10	8	8	4	4	4	4
Protected Phases	2	2	6	6	8	8	4	4
Detector Phase								
Switch Phase								
Minimum Initial (s)	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
Minimum Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0
Total Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	26.0	26.0
Total Split (%)	20.8%	20.8%	20.8%	20.8%	20.8%	20.8%	20.8%	20.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag								
Lead-Lag Optimize?	None	None	None	None	None	None	None	None
Recall Mode								
Act Effct Green (s)	61.0	61.0	61.0	61.0	23.1	23.1	23.1	23.1
Actuated g/C Ratio	0.65	0.65	0.65	0.65	0.25	0.25	0.25	0.25
v/c Ratio	0.54	0.36	0.36	0.46	0.19	0.19	0.19	0.19
Control Delay	9.3	6.7	6.7	40.2	33.4	33.4	33.4	33.4
Queue Delay	1.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.1	6.7	6.7	40.2	33.4	33.4	33.4	33.4
LOS	B	A	A	D	C	C	C	C
Approach Delay	11.1	6.7	6.7	40.2	33.4	33.4	33.4	33.4
Approach LOS	B	A	A	D	C	C	C	C
Intersection Summary								
Cycle Length: 125								
Actuated Cycle Length: 94.1								
Natural Cycle: 100								
Control Type: Actuated-Uncoordinated								
Maximum v/c Ratio: 0.54								
Intersection Signal Delay: 15.4								
Intersection Capacity Utilization 61.6%								
Analysis Period (min) 15								



HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 24: Sumach Street & Dundas Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	45	460	15	25	245	55	25	90	45	25	20	15
Future Volume (vph)	45	460	15	25	245	55	25	90	45	25	20	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Frbp. ped/bikes	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Frt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Sat'd. Flow (prot)	1699	1586	1586	1586	1586	1586	1586	1586	1586	1586	1586	1586
Flt Permitted	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Sat'd. Flow (perm)	1614	1502	1502	1516	1516	1516	1516	1516	1516	1516	1516	1516
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	48	495	16	27	263	59	27	97	48	27	22	16
RTOR Reduction (vph)	0	1	0	0	7	0	0	10	0	0	0	8
Lane Group Flow (vph)	0	558	0	0	342	0	0	162	0	0	0	57
Conf. Bikes (#/hr)	100	185	185	100	105	50	50	50	50	50	50	105
Heavy Vehicles (%)	12%	6%	8%	0%	9%	5%	4%	5%	6%	0%	9%	0%
Turn Type	Perm	MA	Perm	NA	Perm	NA	Perm	NA	Perm	MA	Perm	MA
Protected Phases												
Permitted Phases	2,9	6,10	6,10	8	8	4	8	4	4	4	4	4
Effective Green, G (s)	63.5	64.5	63.5	22.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Actuated Cycle Length (s)	69	69	69	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Actuated g/C Ratio	0.69	0.69	0.69	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1113	1036	1036	372	372	372	372	372	372	372	372	372
v/s Ratio Prot												
v/s Ratio Perm	60.35	0.23	0.23	60.11	60.11	60.11	60.11	60.11	60.11	60.11	60.11	60.11
v/c Ratio	0.50	0.33	0.33	0.44	0.44	0.44	0.44	0.44	0.44	0.44	0.44	0.44
Uniform Delay, d1	6.9	5.8	5.8	29.8	29.8	29.8	29.8	29.8	29.8	29.8	29.8	29.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.4	0.2	0.2	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8
Delay (s)	7.2	6.0	6.0	30.6	30.6	30.6	30.6	30.6	30.6	30.6	30.6	30.6
Level of Service	A	A	A	C	C	C	C	C	C	C	C	C
Approach Delay (s)	7.2	6.0	6.0	30.6	30.6	30.6	30.6	30.6	30.6	30.6	30.6	30.6
Approach LOS	A	A	A	C	C	C	C	C	C	C	C	C
Intersection Summary												
HCM 2000 Control Delay	11.5 HCM 2000 Level of Service B											
HCM 2000 Volume to Capacity ratio	0.50											
Actuated Cycle Length (s)	93.5 Sum of lost time (s) 10.0											
Intersection Capacity Utilization	61.6% ICU Level of Service B											
Analysis Period (min)	15											
c Critical Lane Group												

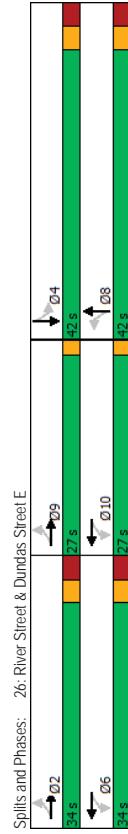
HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 25: Tubman Avenue & Dundas Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	520	10	10	320	0	0	0	0	5	0	5
Future Volume (Veh/h)	0	520	10	10	320	0	0	0	0	5	0	5
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	547	11	11	337	0	0	0	0	5	0	5
Pedestrians	10	10	10	10	10	10	10	10	10	10	10	10
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	1	1	1	1	1	1	1	1	1	1	1	1
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None	None	None
Median type	None	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)	99	99	99	99	99	99	99	99	99	99	99	99
Upstream signal (m)	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
px. platoon unblocked	387	688	688	1056	1092	692	972	1097	397			
v/c, conflicting volume												
v/c1, stage 1 conf vol												
v/c2, stage 2 conf vol												
v/cu, unblocked vol	387	562	562	988	1028	567	889	1034	397			
IC, single (s)	4.1	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	7.1	6.5	6.2
IC, 2 stage (s)												
IF (s)	2.2	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100	99	99	100	100	100	98	100	99	98	100	99
dM capacity (veh/h)	1130	882	882	186	193	452	209	191	622			
Direction, Lane #	EB 1	WB 1	SB 1									
Volumes Total	558	348	10									
Volume Left	0	11	5									
Volume Right	11	0	5									
ESH	1130	882	313									
Volumes to Capacity	0.00	0.01	0.03									
Queue Length 95th (m)	0.0	0.3	0.8									
Control Delay (s)	0.0	0.4	16.9									
Lane LOS	A	A	C									
Approach Delay (s)	0.0	0.4	16.9									
Approach LOS	C	C	C									
Intersection Summary												
Average Delay	0.3											
Intersection Capacity Utilization	42.6% ICU Level of Service A											
Analysis Period (min)	15											

Timings 26: River Street & Dundas Street E 2032 Future Background PM Model (ExRN) 10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	EB	EB	WB	WB	NB	NB	SB	SB				
Traffic Volume (vph)	25	440	95	210	45	365	105	445				
Future Volume (vph)	25	440	95	210	45	365	105	445				
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA				
Protected Phases	2.9		6.10		8		4		2	6	9	10
Permitted Phases	2	2	6	6	8	8	4	4				
Detector Phase												
Switch Phase												
Minimum Initial (s)					21.0	21.0	21.0	21.0	21.0	21.0	5.0	5.0
Minimum Split (s)					27.0	27.0	27.0	27.0	27.0	27.0	27.0	27.0
Total Split (s)					42.0	42.0	42.0	42.0	34.0	34.0	27.0	27.0
Total Split (%)					40.8%	40.8%	40.8%	40.8%	33%	33%	26%	26%
Yellow Time (s)					3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
All-Red Time (s)					3.0	3.0	3.0	3.0	3.0	3.0	0.0	0.0
Lost Time Adjust (s)					-1.0	-1.0	-1.0	-1.0				
Total Lost Time (s)					5.0	5.0	5.0	5.0				
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode					Min	Min	Min	Min	Min	Min	None	None
Act Effct Green (s)		31.8		31.8	37.7	37.7	37.7	37.7				
Actuated g/C Ratio		0.40		0.40	0.47	0.47	0.47	0.47				
v/c Ratio		0.82		0.85	0.20	0.79	0.88	0.71				
Control Delay		31.5		37.3	19.5	28.2	34.8	25.7				
Queue Delay		0.0		0.0	0.0	0.0	0.0	0.0				
Total Delay		31.5		37.3	19.5	28.2	34.8	25.7				
LOS		C		D	B	C	C	C				
Approach Delay		31.5		37.3	27.6	27.6	27.2	27.2				
Approach LOS		C		D	C	C	C	C				
Intersection Summary												
Cycle Length: 103												
Actuated Cycle Length: 79.7												
Natural Cycle: 95												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.85												
Intersection Signal Delay: 30.2												
Intersection Capacity Utilization: 120.3%												
Analysis Period (min): 15												



Queues 26: River Street & Dundas Street E 2032 Future Background PM Model (ExRN) 10-25-2022

Lane Group	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	552	421	47	610	111	547
v/c Ratio	0.82	0.85	0.20	0.79	0.58	0.71
Control Delay	31.5	37.3	19.5	28.2	34.8	25.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.5	37.3	19.5	28.2	34.8	25.7
Queue Length 50th (m)	70.0	52.4	3.6	64.4	10.6	56.6
Queue Length 95th (m)	103.3	86.0	16.6	192.0	49.6	168.5
Internal Link Dist (m)	73.4	80.5		131.0		118.9
Turn Bay Length (m)			20.0		25.0	
Base Capacity (vph)	711	523	230	777	193	769
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.78	0.80	0.20	0.79	0.58	0.71
Intersection Summary						
# 95th percentile volume exceeds capacity, queue may be longer.						
Queue shown is maximum after two cycles.						

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 26: River Street & Dundas Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	25	440	60	95	210	95	45	365	215	105	445	75
Future Volume (vph)	25	440	60	95	210	95	45	365	215	105	445	75
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Frbp. ped/bikes	1.00	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Frt	1.00	0.98	0.98	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Flt Protected	1.00	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Sat'd. Flow (prot)	1724	1637	1637	1532	1633	1633	1591	1627	1591	1627	1627	1627
Flt Permitted	0.97	0.73	0.73	0.29	1.00	1.00	0.23	1.00	0.23	1.00	1.00	1.00
Sat'd. Flow (perm)	1676	1214	1214	466	1633	1633	393	1627	393	1627	1627	1627
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	26	463	63	100	221	100	47	384	226	111	468	79
RTOR Reduction (vph)	0	5	0	0	13	0	0	17	0	0	5	0
Lane Group Flow (vph)	0	547	0	0	408	0	47	593	0	111	542	0
Conf. Bikes (#/hr)	55	75	75	75	55	75	75	75	75	75	75	75
Heavy Vehicles (%)	9%	5%	0%	1%	9%	0%	5%	3%	0%	2%	12%	0%
Turn Type	Permi	MA	Permi	NA	Permi	NA	Permi	NA	Permi	MA	Permi	MA
Protected Phases	2.9		6.10		6.10		8		8	4		4
Permitted Phases	2.9		6.10		6.10		8		8	4		4
Actuated Green, G (s)	36.5		36.5		36.5		36.7		36.7	36.7		36.7
Effective Green, g (s)	37.5		37.5		37.7		37.7		37.7	37.7		37.7
Actuated g/C Ratio	0.46		0.46		0.46		0.46		0.46	0.46		0.46
Clearance Time (s)							6.0		6.0	6.0		6.0
Vehicle Extension (s)							3.0		3.0	3.0		3.0
Lane Grp Cap (vph)	774		560		560		216		758	182		755
v/s Ratio Prot							c0.36					0.33
v/s Ratio Perm	0.33		c0.34		c0.34		0.10		0.28	0.28		0.28
v/c Ratio	0.71		0.73		0.73		0.22		0.78	0.61		0.72
Uniform Delay, d1	17.5		17.7		17.7		13.0		18.3	16.3		17.5
Progression Factor	1.00		1.00		1.00		1.00		1.00	1.00		1.00
Incremental Delay, d2	3.0		4.7		4.7		0.5		5.3	5.7		3.3
Delay (s)	20.4		22.4		22.4		13.5		23.6	21.9		20.8
Level of Service	C		C		C		B		C	C		C
Approach Delay (s)	20.4		22.4		22.4		22.8		22.8	21.0		21.0
Approach LOS	C		C		C		C		C	C		C
Intersection Summary												
HCM 2000 Control Delay	21.6 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.79											
Actuated Cycle Length (s)	81.2 Sum of lost time (s) 11.0											
Intersection Capacity Utilization	120.3% ICU Level of Service H											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 27: Dreamer's Way & Gerrard Street E

10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	835	55	10	430	0	0
Future Volume (Veh/h)	835	55	10	430	0	0
Sign Control	Free	Free	Stop	Stop	0%	0%
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	879	58	11	453	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None					
Median type	None					
Median storage (veh)						
Upstream signal (m)	90			162		
pK platoon unblocked	0.87			0.87		0.87
vC, conflicting volume	937			1156		468
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	623			876		83
IC, single (s)	4.1			6.8		6.9
IC, 2 stage (s)						
p0 queue free %	99			100		100
IF (s)	2.2			3.5		3.3
dM capacity (veh/h)	840			250		839
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volumes Total	586	351	162	302		
Volume Left	0	0	11	0		
Volume Right	0	58	0	0		
CSH	1700	1700	840	1700		
Volumes to Capacity	0.34	0.21	0.01	0.18		
Queue Length 95th (m)	0.0	0.0	0.3	0.0		
Control Delay (s)	0.0	0.0	0.8	0.0		
Lane LOS			A			
Approach Delay (s)	0.0		0.3			
Approach LOS			A			
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	28.2%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (ExRN)  
 29: Sumach Street & Site Driveway

10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		T			T
Traffic Volume (veh/h)	10	5	95	0	0	40
Future Volume (Veh/h)	10	5	95	0	0	40
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	5	100	0	0	42
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)			None			None
Median type			None			None
Median storage (veh)						
Upstream signal (m)						55
pX platoon unblocked						
VC conflicting volume	142	100			100	
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol	142	100			100	
IC single (s)	6.4	6.2			4.1	
IC 2 stage (s)						
IF (s)	3.5	3.3			2.2	
p0 queue free %	99	99			100	
CM capacity (veh/h)	856	961			1505	
Direction_Lane #	WB 1	NB 1	SB 1			
Volumes Total	16	100	42			
Volume Left	11	0	0			
Volume Right	5	0	0			
CSH	886	1700	1505			
Volumes to Capacity	0.02	0.06	0.00			
Queue Length 95th (m)	0.4	0.0	0.0			
Control Delay (s)	9.1	0.0	0.0			
Lane LOS	A					
Approach Delay (s)	9.1	0.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization		15.0%			ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 1: Parliament Street & Gerrard Street E (North Section)

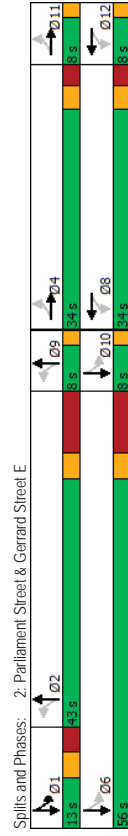
10-25-2022

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations			T	T	T	T
Traffic Volume (veh/h)	0	0	10	515	585	75
Future Volume (Veh/h)	0	0	10	515	585	75
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	0	0	11	579	657	84
Pedestrians	150				5	10
Lane Width (m)	0.0				3.5	3.5
Walking Speed (m/s)	1.1				1.1	1.1
Percent Blockage					0	1
Right turn flare (veh)					0	1
Median type			None		None	None
Median storage (veh)						
Upstream signal (m)					39	
pX platoon unblocked						
VC conflicting volume	1170	526	891			
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol	1066	526	891			
IC single (s)	6.8	6.9	4.3			
IC 2 stage (s)						
IF (s)	3.5	3.3	2.3			
p0 queue free %	100	100	98			
CM capacity (veh/h)	204	500	702			
Direction_Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	204	386	438	303		
Volume Left	11	0	0	0		
Volume Right	0	0	0	84		
CSH	702	1700	1700	1700		
Volumes to Capacity	0.02	0.23	0.26	0.18		
Queue Length 95th (m)	0.4	0.0	0.0	0.0		
Control Delay (s)	0.7	0.0	0.0	0.0		
Lane LOS	A					
Approach Delay (s)	0.2		0.0			
Approach LOS						
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization		32.9%			ICU Level of Service	A
Analysis Period (min)			15			

Timings  
2: Parliament Street & Gerrard Street E

2032 Future Background PM Model (FutRN)  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø2	Ø4	Ø6	Ø8
Lane Configurations												
Traffic Volume (vph)	30	540	45	250	75	360	230	285	41%	41%		
Future Volume (vph)	30	540	45	250	75	360	230	285				
Turn Type	Perm	NA	Perm	NA	Perm	NA	custom	NA				
Protected Phases	4 11	8 12	8 12	2 9	1 16	10	2 4	6 8				
Permitted Phases	4	4	8	8	2	2	1	1 6				
Detector Phase												
Switch Phase												
Minimum Initial (s)									22.0	21.0	22.0	21.0
Minimum Split (s)									12.5	33.2	31.8	33.2
Total Split (s)									13.0	43.0	34.0	56.0
Total Split (%)									12.3%	41%	32%	53%
Yellow Time (s)									3.3	3.0	3.3	3.0
All-Red Time (s)									3.2	7.9	2.8	7.9
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag												
Lead-Lag Optimize?									Yes	Yes	Yes	Yes
Recall Mode									None	None	None	None
Act Effct Green (s)	31.4	31.4	31.4	28.9	31.4	28.9	53.1	53.1				
Actuated g/C Ratio	0.33	0.33	0.33	0.30	0.33	0.30	0.56	0.56				
v/C Ratio	0.64	0.50	0.50	0.77	0.51	0.51						
Control Delay	30.0	30.0	30.0	37.3	30.0	37.3	13.4	13.4				
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0						
Total Delay	30.0	23.4	23.4	37.3	30.0	37.3	13.4	13.4				
LOS	C	C	C	D	C	D	B	B				
Approach Delay	30.0	23.4	23.4	37.3	30.0	37.3	13.4	13.4				
Approach LOS	C	C	C	D	C	D	B	B				
Intersection Summary												
Cycle Length: 106												
Actuated Cycle Length: 95												
Natural Cycle: 95												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.77												
Intersection Signal Delay: 26.1												
Intersection Capacity Utilization: 100.2%												
Analysis Period (min): 15												



Timings  
2: Parliament Street & Gerrard Street E

2032 Future Background PM Model (FutRN)  
10-25-2022

Lane Group	Ø9	Ø10	Ø11	Ø12
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Turn Type				
Protected Phases	9	10	11	12
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0	5.0	5.0	5.0
Minimum Split (s)	8.0	8.0	8.0	8.0
Total Split (s)	8.0	8.0	8.0	8.0
Total Split (%)	8%	8%	8%	8%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	None	None
Actuated g/C Ratio				
v/C Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Intersection Summary				

Queues 2032 Future Background PM Model (FutRN) 10-25-2022  
 2: Parliament Street & Gerrard Street E

	EBT	WBT	NBT	SBT
Lane Group	665	443	567	603
Lane Group Flow (vph)	0.64	0.50	0.77	0.51
v/c Ratio	30.0	23.4	37.3	13.4
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	30.0	23.4	37.3	13.4
Total Delay	54.9	28.9	50.5	30.9
Queue Length 50th (m)	76.3	45.1	73.1	43.6
Queue Length 95th (m)	33.0	65.6	119.2	15.0
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1097	902	848	1234
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.61	0.49	0.67	0.49
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN) 10-25-2022  
 2: Parliament Street & Gerrard Street E

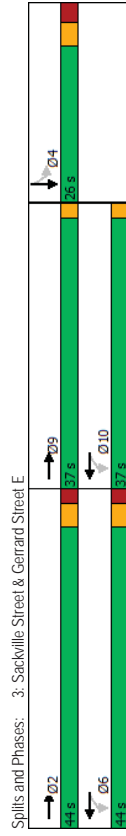
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4TB			4TB			4TB					
Traffic Volume (vph)	30	540	75	45	250	135	75	360	115	230	285	70	
Future Volume (vph)	30	540	75	45	250	135	75	360	115	230	285	70	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	4.8			4.8				10.2				5.5	
Lane Util. Factor	0.95			0.95				0.95				0.95	
Frbp. ped/bikes	0.98			0.95				0.93				0.99	
Frbp. psd/bikes	1.00			1.00				0.99				0.97	
Frt	0.98			0.95				0.97				0.98	
Flt Protected	1.00			0.99				0.99				0.98	
Sat'd. Flow (prot)	3390			3105				3109				3172	
Flt Permitted	0.91			0.81				0.75				0.60	
Sat'd. Flow (perm)	3108			2522				2353				1946	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	31	557	77	46	258	139	77	371	119	237	294	72	
RTOR Reduction (vph)	0	9	0	0	48	0	0	18	0	0	0	11	
Lane Group Flow (vph)	0	656	0	0	395	0	0	549	0	0	592	0	
Confl. Peds. (#/hr)	130	120	120	130	125	125	235	235	125	235	235	125	
Confl. Bikes (#/hr)			25			10			15			5	
Heavy Vehicles (%)	10%	0%	5%	0%	0%	10%	4%	2%	2%	6%	4%	0%	
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	4 11			8 12			2 9			1 6 10			
Permitted Phases													
Actuated Green, G (s)	35.1			35.1			43.8			56.9			
Effective Green, g (s)	36.1			36.1			44.8			47.7			
Actuated g/C Ratio	0.38			0.38			0.47			0.50			
Clearance Time (s)													
Vehicle Extension (s)													
Lane Grp Cap (vph)	1168			948			1098			1063			
v/s Ratio Prot										60.04			
v/s Ratio Perm	60.21			0.16			60.23			60.23			
v/c Ratio	0.56			0.42			0.50			0.56			
Uniform Delay, d1	23.7			22.2			17.8			16.8			
Progression Factor	1.00			1.00			1.00			1.00			
Incremental Delay, d2	0.6			0.3			0.4			0.6			
Delay (s)	24.3			22.5			18.2			17.4			
Level of Service	C			C			B			B			
Approach Delay (s)	24.3			22.5			18.2			17.4			
Approach LOS	C			C			B			B			
<b>Intersection Summary</b>													
HCM 2000 Control Delay	20.6											HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.64												
Actuated Cycle Length (s)	96.0											Sum of lost time (s)	22.5
Intersection Capacity Utilization	100.2%											ICU Level of Service	G
Analysis Period (min)	15												
c Critical Lane Group													



Timings 2032 Future Background PM Model (FutRN) 10-25-2022

3. Sackville Street & Gerrard Street E

Lane Group	EBT	WBL	WBT	SBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	↑↑		↑↑	↔				
Traffic Volume (vph)	930	50	540	40				
Future Volume (vph)	930	50	540	40				
Turn Type	NA	Perm	NA	NA				
Protected Phases	2.9		6.10	4	2	6	9	10
Permitted Phases	6.10							
Detector Phase	2	6	6	4				
Switch Phase								
Minimum Initial (s)				20.0	16.0	16.0	5.0	5.0
Minimum Split (s)				25.7	21.0	21.0	37.0	37.0
Total Split (s)				26.0	44.0	44.0	37.0	37.0
Total Split (%)				24.3%	41%	41%	35%	35%
Yellow Time (s)				3.0	3.0	3.0	2.0	2.0
All-Red Time (s)				2.7	2.0	2.0	0.0	0.0
Lost Time Adjust (s)				-1.0				
Total Lost Time (s)				4.7				
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode				Min	Min	Min	None	None
Act Effct Green (s)	37.3		37.3	21.6				
Actuated g/C Ratio	0.55		0.55	0.32				
v/c Ratio	0.53		0.41	0.40				
Control Delay	10.1		9.1	23.6				
Queue Delay	0.0		0.0	0.0				
Total Delay	10.1		9.1	23.6				
LOS	B		A	C				
Approach Delay	10.1		9.1	23.6				
Approach LOS	B		A	C				
<b>Intersection Summary</b>								
Cycle Length:	107							
Actuated Cycle Length:	67.8							
Natural Cycle:	85							
Control Type:	Actuated-Uncoordinated							
Maximum v/c Ratio:	0.53							
Intersection Signal Delay:	11.3							
Intersection Capacity Utilization:	72.1%							
Analysis Period (min):	15							



Queues 2032 Future Background PM Model (FutRN) 10-25-2022

3. Sackville Street & Gerrard Street E

Lane Group	EBT	WBT	SBT
Lane Group Flow (vph)	995	615	209
v/c Ratio	0.53	0.41	0.40
Control Delay	10.1	9.1	23.6
Queue Delay	0.0	0.0	0.0
Total Delay	10.1	9.1	23.6
Queue Length 50th (m)	36.6	20.8	21.2
Queue Length 95th (m)	49.0	29.7	44.9
Internal Link Dist (m)	56.5	47.8	15.3
Turn Bay Length (m)			
Base Capacity (vph)	2208	1772	529
Starvation Cap Reductn	0	0	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.45	0.35	0.40
<b>Intersection Summary</b>			

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 3: Sackville Street & Gerrard Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4↑	4↑								4↓	4↓
Traffic Volume (vph)	0	930	25	50	540	0	0	0	0	135	40	25
Future Volume (vph)	0	930	25	50	540	0	0	0	0	135	40	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)		4.0	4.0								4.7	
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.98	0.98
Frt	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.97	0.97
Sat'd. Flow (prot)	3437	3437	3437	3437	3437	3437	3437	3437	3437	3437	1679	1679
FIL Permitted	1.00	0.82	0.82								0.97	0.97
Sat'd. Flow (perm)	3437	2760	2760								1679	1679
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	969	26	52	562	0	0	0	0	141	42	26
RTOR Reduction (vph)	0	2	0	0	0	0	0	0	0	0	0	4
Lane Group Flow (vph)	0	993	0	0	615	0	0	0	0	0	205	0
Conf. Peds. (#/hr)	85	70	70	85	90	85	90	60	60	60	60	90
Conf. Bikes (#/hr)		60		15				5			10	
Heavy Vehicles (%)	0%	3%	0%	0%	6%	0%	0%	0%	5%	1%	0%	0%
Turn Type	MA	Perm	MA	Perm	MA	Perm	MA	Perm	MA	Perm	MA	Perm
Protected Phases	2.9			6.10								4
Permitted Phases		6.10									4	
Actuated Green, G (s)	39.8			39.8							20.6	
Effective Green, g (s)	40.8			40.8							21.6	
Actuated g/C Ratio	0.60			0.60							0.32	
Clearance Time (s)											5.7	
Vehicle Extension (s)											3.0	
Lane Grp Cap (vph)	2059			1653							532	
v/s Ratio Prot	c0.29											
v/s Ratio Perm		0.48		0.22							0.12	
v/c Ratio		0.37		0.37							0.39	
Uniform Delay, d1	7.7			7.0							18.1	
Progression Factor	1.00			1.00							1.00	
Incremental Delay, d2	0.2			0.1							0.5	
Delay (s)	7.9			7.2							18.6	
Level of Service	A			A							B	
Approach Delay (s)	7.9			7.2			0.0				18.6	
Approach LOS	A			A			A				B	
Intersection Summary												
HCM 2000 Control Delay	8.9 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.47											
Actuated Cycle Length (s)	68.1 Sum of lost time (s) 9.7											
Intersection Capacity Utilization	72.1% ICU Level of Service C											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 4: Gerrard Street E & Gifford Street

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4↑	4↑		W	
Traffic Volume (veh/h)	10	1055	585	5	15	5
Future Volume (Veh/h)	10	1055	585	5	15	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	12	1241	688	6	18	6
Pedestrians		5			65	
Lane Width (m)		3.5			3.0	
Walking Speed (m/s)		1.1			1.1	
Percent Blockage		0			5	
Right turn flare (veh)		None		None		
Median type		None		None		
Median storage (veh)						
Upstream signal (m)		72	141			
pX platoon unblocked	0.97			0.86	0.97	
vC, conflicting volume	759			1400	417	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	684			960	331	
iC, single (s)	4.1			6.8	6.9	
iC, 2 stage (s)						
IF (s)	2.2			3.5	3.3	
p0 queue free %	99			91	99	
dM capacity (veh/h)	845			207	614	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	426	827	459	235	24	
Volume Left	12	0	0	0	18	
Volume Right	0	0	0	6	6	
ESH	845	1700	1700	1700	248	
Volumes to Capacity	0.01	0.49	0.27	0.14	0.10	
Queue Length 95th (m)	0.3	0.0	0.0	0.0	2.4	
Control Delay (s)	0.4	0.0	0.0	0.0	21.1	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		21.1	
Approach LOS					C	
Intersection Summary						
Average Delay	0.4					
Intersection Capacity Utilization	47.7%					
ICU Level of Service	A					
Analysis Period (min)	15					

5. Gerrard Street E & Nasmith Avenue

10-25-2022

2032 Future Background PM Model (FuTRN)

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	4A	4A	4B	4B	W	W
Traffic Volume (veh/h)	5	1065	585	5	5	5
Future Volume (Veh/h)	5	1065	585	5	5	5
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	6	1224	672	6	6	6
Pedestrians					60	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					5	
Right turn flare (veh)						
Median type		None		None		
Median storage (veh)						
Upstream signal (m)	0.94	145	68		0.88	0.94
pX platoon unblocked					1359	399
VC, conflicting volume						
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	601				844	241
IC, single (s)	4.1				6.8	6.9
IC, 2 stage (s)	2.2				3.5	3.3
p0 queue free %	99				98	99
CM capacity (veh/h)	888				256	689
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 1
Volumes Total	414	816	448	230	12	12
Volume Left	6	0	0	0	6	6
Volume Right	0	0	0	0	6	6
cSH	888	1700	1700	1700	373	373
Volumes to Capacity	0.01	0.48	0.26	0.14	0.03	0.03
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.8	0.8
Control Delay (s)	0.2	0.0	0.0	0.0	15.0	15.0
Lane LOS	A				B	B
Approach Delay (s)	0.1		0.0		15.0	
Approach LOS					B	
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	42.9%					
ICU Level of Service	A					
Analysis Period (min)	15					



6. Summach Street & Gerrard Street E

10-25-2022

2032 Future Background PM Model (FuTRN)

10-25-2022

Lane Group	EBL	EBT	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	4A	4A	4B	4B				
Traffic Volume (vph)	10	1060	565	25				
Future Volume (vph)	10	1060	565	25				
Turn Type	Perm	NA	NA	NA	2	6	9	10
Protected Phases		2.9	6.10	8				
Permitted Phases	2.9							
Detector Phase	2.9	2.9	6.10	8				
Switch Phase								
Minimum Initial (s)		15.0	11.0	11.0	5.0	5.0	5.0	5.0
Minimum Split (s)		25.4	26.2	26.2	72.0	72.0	72.0	72.0
Total Split (s)		27.0	43.0	43.0	72.0	72.0	72.0	72.0
Total Split (%)		19.0%	30%	30%	30%	51%	51%	51%
Yellow Time (s)		3.0	3.0	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)		7.4	7.4	7.2	7.2	0.0	0.0	0.0
Lost Time Adjust (s)		-1.0						
Total Lost Time (s)		9.4						
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode					None	Min	None	None
Act Effct Green (s)		58.1	58.1	16.4				
Actuated g/C Ratio		0.62	0.62	0.18				
v/C Ratio		0.54	0.34	0.47				
Control Delay		10.9	8.3	29.2				
Queue Delay		0.0	0.0	0.0				
Total Delay		10.9	8.3	29.2				
LOS		B	A	C				
Approach Delay		10.9	8.3	29.2				
Approach LOS		B	A	C				
Intersection Summary								
Cycle Length	142							
Actuated Cycle Length	93.3							
Natural Cycle	725							
Control Type	Actuated-Uncoordinated							
Maximum v/C Ratio	0.54							
Intersection Signal Delay	11.4							
Intersection Capacity Utilization	68.7%							
Analysis Period (min)	15							



Queues  
 6: Summach Street & Gerrard Street E  
 2032 Future Background PM Model (FutRN)  
 10-25-2022

	EBT	WBT	NBT
Lane Group	1103	670	145
Lane Group Flow (vph)	0.54	0.34	0.47
v/c Ratio	10.9	8.3	29.2
Control Delay	0.0	0.0	0.0
Queue Delay	10.9	8.3	29.2
Total Delay	52.5	25.3	15.8
Queue Length 50th (m)	70.4	35.9	35.5
Queue Length 95th (m)	44.0	75.7	30.7
Internal Link Dist (m)			
Turn Bay Length (m)	3242	3028	329
Base Capacity (vph)	245	0	0
Station Cap Reductn	0	0	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.37	0.22	0.44
<b>Intersection Summary</b>			

HCM Signalized Intersection Capacity Analysis  
 6: Summach Street & Gerrard Street E  
 2032 Future Background PM Model (FutRN)  
 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations		4						4			
Traffic Volume (vph)	10	1060	0	0	565	85	25	25	90	0	0
Future Volume (vph)	10	1060	0	0	565	85	25	25	90	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5
Total Lost time (s)		9.2			9.2			9.4			
Lane Util. Factor		0.95			0.95			1.00			
Frbp. ped/bikes		1.00			0.96			0.95			
Frbp. psd/bikes		1.00			1.00			0.97			
Frt		1.00			0.98			0.91			
Flt Protected		3495			3247			1569			
Sat'd. Flow (prot)		0.95			1.00			0.99			
Flt Permitted		3312			3247			1569			
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	10	1093	0	0	582	88	26	26	93	0	0
RTOR Reduction (vph)	0	0	0	0	9	0	0	43	0	0	0
Lane Group Flow (vph)	0	1103	0	0	661	0	0	102	0	0	0
Confl. Peds. (#/hr)	95	95	95	95	95	80	35	35	35	35	80
Confl. Bikes (#/hr)		75			35			5			
Heavy Vehicles (%)	0%	2%	0%	0%	4%	0%	0%	0%	0%	0%	0%
Turn Type	Perm	NA	NA	NA	NA	Perm	NA	NA	NA	NA	NA
Protected Phases		2.9			6.10			8			
Permitted Phases		2.9			6.10			8			
Actuated Green, G (s)		65.3			65.3			15.4			
Effective Green, g (s)		66.3			66.3			16.4			
Actuated g/C Ratio		0.71			0.71			0.18			
Clearance Time (s)								10.4			
Vehicle Extension (s)								3.0			
Lane Grp Cap (vph)		2358			2312			276			
v/s Ratio Prot					0.20						
v/s Ratio Perm		0.33			0.29			0.07			
v/c Ratio		0.47			0.29			0.37			
Uniform Delay, d1		5.8			4.8			33.8			
Progression Factor		1.00			1.00			1.00			
Incremental Delay, d2		0.1			0.1			0.8			
Delay (s)		5.9			4.9			34.6			
Level of Service		A			A			C			
Approach Delay (s)		5.9			4.9			34.6			0.0
Approach LOS		A			A			C			A
<b>Intersection Summary</b>											
HCM 2000 Control Delay	7.7 HCM 2000 Level of Service										
HCM 2000 Volume to Capacity ratio	0.49										
Actuated Cycle Length (s)	93.1										
Intersection Capacity Utilization	68.7%										
Analysis Period (min)	15										
c Critical Lane Group											

7. Site Laneway/Sword Street & Gerrard Street E

10-25-2022

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔		↔↔							↔↔	
Traffic Volume (veh/h)	0	1140	10	0	640	0	0	0	0	15	0	10
Future Volume (Veh/h)	0	1140	10	0	640	0	0	0	0	15	0	10
Sign Control		Free		Free			Stop		Stop		Stop	
Grade		0%		0%			0%		0%		0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	1267	11	0	711	0	0	0	0	17	0	11
Pedestrians											65	
Lane Width (m)											3.0	
Walking Speed (m/s)											1.1	
Percent Blockage											5	
Right turn flare (veh)												
Median type		None		None								
Median storage (veh)												
Upstream signal (m)		100		0.87			0.90	0.90	0.87	0.90	0.90	0.93
pX platoon unblocked	0.93			1278			1639	2048	639	1410	2054	420
VC, conflicting volume												
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	621			1009			1157	1613	271	902	1620	241
IC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	7.1
IC, 2 stage (s)												
p0 queue free %	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.4
p0 queue free (s)	100			100			100	100	100	91	100	98
CM capacity (veh/h)	862			601			131	90	634	194	89	660
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1							
Volumes Total	845	433	356	356	28							
Volume Left	0	0	0	0	17							
Volume Right	0	11	0	0	11							
cSH	1700	1700	1700	1700	268							
Volumes to Capacity	0.50	0.25	0.21	0.21	0.10							
Queue Length 95th (m)	0.0	0.0	0.0	0.0	2.6							
Control Delay (s)	0.0	0.0	0.0	0.0	20.0							
Lane LOS					C							
Approach Delay (s)	0.0	0.0	0.0	20.0								
Approach LOS				C								
<b>Intersection Summary</b>												
Average Delay	0.3											
Intersection Capacity Utilization	41.8%											
Analysis Period (min)	15											
	ICU Level of Service A											



Splits and Phases: 8: River Street & Gerrard Street E

Regent Park Phases 4 & 5  
BA Group - NHY

Synchro 11 Report  
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8: River Street & Gerrard Street E

10-25-2022

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Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Lane Configurations		↔↔		↔↔		↔↔		↔↔				
Traffic Volume (vph)	225	855	85	380	70	340	165	450	190			
Future Volume (vph)	225	855	85	380	70	340	165	450	190			
Turn Type	custom	NA	Perm	NA	Perm	NA	Perm	NA	Perm	2	4	6
Protected Phases		5	5	2	6	6	8	8	8	4	4	4
Permitted Phases	2	9	6	10	8	12	4	11	4	4	4	4
Detector Phase												
Switch Phase												
Minimum Initial (s)		6.0								23.0	19.0	23.0
Minimum Split (s)		10.0								33.8	30.0	33.8
Total Split (s)		12.0								48.0	43.0	36.0
Total Split (%)		10.1%								40%	36%	30%
Yellow Time (s)		3.0								3.0	3.0	3.0
All-Red Time (s)		1.0								7.8	3.0	7.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag		Lead										
Lead-Lag Optimize?		Yes										
Recall Mode		None										
Ad Effct Green (s)		55.6								39.0	39.0	39.0
Actuated g/C Ratio		0.54								0.38	0.38	0.38
v/c Ratio		0.90								0.77	1.00	0.67
Control Delay		30.4								33.0	33.0	4.6
Queue Delay		18.5								0.0	0.0	0.0
Total Delay		48.9								33.0	33.0	4.6
LOS		D								D	F	C
Approach Delay		48.9								36.1	41.0	
Approach LOS		D								D	D	
<b>Intersection Summary</b>												
Cycle Length	119											
Actuated Cycle Length	102.7											
Natural Cycle	105											
Control Type	Actuated-Uncoordinated											
Maximum v/c Ratio	1.00											
Intersection Signal Delay	45.0											
Intersection Capacity Utilization	116.5%											
Analysis Period (min)	15											



Splits and Phases: 8: River Street & Gerrard Street E

Regent Park Phases 4 & 5  
BA Group - NHY

Synchro 11 Report  
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Timings  
8: River Street & Gerrard Street E

Queues  
8: River Street & Gerrard Street E

2032 Future Background PM Model (FutRN)  
10-25-2022

2032 Future Background PM Model (FutRN)  
10-25-2022

Lane Group	Ø8	Ø9	Ø10	Ø11	Ø12
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Turn Type					
Protected Phases	8	9	10	11	12
Permitted Phases					
Detector Phase					
Switch Phase					
Minimum Initial (s)	19.0	5.0	5.0	5.0	5.0
Minimum Split (s)	30.0	18.0	18.0	10.0	10.0
Total Split (s)	43.0	18.0	18.0	10.0	10.0
Total Split (%)	36%	15%	15%	8%	8%
Yellow Time (s)	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)	3.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	Min	None	None	None	None
Act Effct Green (s)					
Actuated g/C Ratio					
v/c Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Intersection Summary					

Lane Group	EBT	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	1203	594	73	521	172	469	198
v/c Ratio	0.90	0.92	0.36	0.77	1.00	0.67	0.30
Control Delay	30.4	51.8	31.3	36.8	104.7	33.0	4.6
Queue Delay	18.5	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.9	51.8	31.3	36.8	104.7	33.0	4.6
Queue Length 50th (m)	82.2	55.7	11.6	96.9	-41.0	86.1	0.0
Queue Length 95th (m)	#142.2	#99.8	24.8	138.5	#82.6	121.3	14.0
Internal Link Dist (m)	67.0	81.6		126.5		61.7	
Turn Bay Length (m)			30.0		30.0		
Base Capacity (vph)	1334	644	211	697	178	730	678
Starvation Cap Reductn	163	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	1.03	0.92	0.35	0.75	0.97	0.64	0.29
Intersection Summary							
-	Volume exceeds capacity, queue is theoretically infinite.						
-	Queue shown is maximum after two cycles.						
#	95th percentile volume exceeds capacity, queue may be longer.						
-	Queue shown is maximum after two cycles.						

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 8: River Street & Gerrard Street E

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	225	855	75	85	380	105	70	340	160	165	450	190
Future Volume (vph)	225	855	75	85	380	105	70	340	160	165	450	190
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	3.0	9.8	3.0	9.8	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.99	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.99	0.99	0.97	0.97	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.85
Frt Protected	0.99	0.99	0.99	0.99	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Sat'd. Flow (prot)	3380	3271	3271	1625	1742	1643	1643	1860	1860	1860	1860	1860
FIL Permitted	0.67	0.54	0.54	0.31	1.00	0.26	1.00	1.00	1.00	1.00	1.00	1.00
Sat'd. Flow (perm)	2278	1770	1770	536	1742	456	1860	1860	1860	1860	1860	1860
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	234	891	78	89	396	109	73	354	167	172	469	198
RTOR Reduction (vph)	0	5	0	0	15	0	0	14	0	0	0	114
Lane Group Flow (vph)	0	1198	0	0	579	0	73	507	0	172	469	84
Conf. Peds. (#/hr)	40	80	80	40	15	40	15	40	40	40	15	10
Heavy Vehicles (%)	3%	2%	3%	0%	4%	0%	3%	0%	1%	1%	1%	2%
Turn Type	custom	MA	Perm	NA	Perm	NA	Perm	NA	Perm	MA	Perm	MA
Protected Phases	5	5.29	6.10	6.10	8.12	8.12	4.11	4.11	4.11	4.11	4.11	4.11
Permitted Phases	2.9	57.4	45.3	43.6	43.6	43.6	43.6	43.6	43.6	43.6	43.6	43.6
Effective Green, G (s)	48.6	46.3	44.6	44.6	44.6	44.6	44.6	44.6	44.6	44.6	44.6	44.6
Actuated g/C Ratio	0.46	0.44	0.44	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1149	780	780	227	739	193	790	604	604	604	604	604
v/s Ratio Prot	c0.09				0.29							0.25
v/s Ratio Perm	c0.39				0.14							0.06
v/c Ratio	1.04	0.74	0.74	0.32	0.69	0.89	0.59	0.14	0.14	0.14	0.14	0.14
Uniform Delay, d1	28.2	24.4	20.1	24.5	28.0	23.2	18.5	18.5	18.5	18.5	18.5	18.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	38.4	3.8	3.8	0.8	2.7	36.2	1.2	0.1	0.1	0.1	0.1	0.1
Delay (s)	66.6	28.2	28.2	20.9	27.2	64.1	24.4	18.6	18.6	18.6	18.6	18.6
Level of Service	E	C	C	C	C	E	C	C	C	E	C	B
Approach Delay (s)	66.6	28.2	28.2	26.4	26.4	31.2	31.2	31.2	31.2	31.2	31.2	31.2
Approach LOS	E	C	C	C	C	E	C	C	C	E	C	C
Intersection Summary												
HCM 2000 Control Delay	43.0 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	1.02											
Actuated Cycle Length (s)	106.0 Sum of lost time (s)											
Intersection Capacity Utilization	116.5% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 9: Sackville Street & Site Driveway

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	10	10	0	0	0	0	0	5	110	0
Future Volume (Veh/h)	0	0	10	10	0	0	0	0	0	5	110	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	0	0	12	12	0	0	0	0	0	6	129	0
Pedestrians	60	35	35	35	35	5	5	5	5	15	15	15
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	0.0	0.0	0.0	0.0	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	5	5	5	5	5	0	0	0	0	0	0	0
Right turn flare (veh)						3						
Median type	None											
Median storage (veh)	None											
Upstream signal (m)	58											
px. platoon unblocked	35											
v/c, conflicting volume	216	236	194	193	236	50	189	189	189	35	35	35
vc1, stage 1 conf vol	216	236	194	193	236	50	189	189	189	35	35	35
vc2, stage 2 conf vol	7.1	6.5	6.2	7.1	6.5	6.2	4.1	4.1	4.1	4.1	4.1	4.1
IC, single (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2	2.2	2.2	2.2	2.2	2.2
IF (s)	100	100	99	98	100	100	100	100	100	100	100	100
p0 capacity (veh/h)	650	611	807	688	611	979	1323	1323	1323	1540	1540	1540
Direction_Lane #	EB 1	WB 1	SB 1									
Volumes Total	12	12	135									
Volume Left	0	12	6									
Volume Right	12	0	0									
cSH	807	688	1540									
Volumes to Capacity	0.01	0.02	0.00									
Queue Length 95th (m)	0.3	0.4	0.1									
Control Delay (s)	9.5	10.3	0.4									
Lane LOS	A	B	A									
Approach Delay (s)	9.5	10.3	0.4									
Approach LOS	A	B	A									
Intersection Summary												
Average Delay	1.8											
Intersection Capacity Utilization	29.2%											
ICU Level of Service	A											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 10: Parliament Street & Oak Street

10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		W			W
Traffic Volume (veh/h)	35	55	495	25	25	380
Future Volume (Veh/h)	35	55	495	25	25	380
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	41	65	582	29	29	447
Pedestrians	200		5			55
Lane Width (m)	3.0		3.5			3.5
Walking Speed (m/s)	1.1		1.1			1.1
Percent Blockage			0			5
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			151			143
PX platoon unblocked	0.93	0.93			0.93	
VC conflicting volume	1103	580			831	
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol	962	401			670	
IC single (s)	6.8	6.9			4.3	
IC 2 stage (s)						
IF (s)	3.5	3.3			2.3	
p0 queue free %	78	85			96	
CM capacity (veh/h)	190	446			661	
Direction Lane #	WB 1	NB 2	SB 1	SB 2		
Volumes Total	106	388	223	178	298	
Volume Left	41	0	0	29	0	
Volume Right	65	0	29	0	0	
CSH	293	1700	1700	661	1700	
Volumes to Capacity	0.36	0.23	0.13	0.04	0.18	
Queue Length 95th (m)	12.1	0.0	0.0	1.0	0.0	
Control Delay (s)	24.1	0.0	0.0	2.2	0.0	
Lane LOS	C			A		
Approach Delay (s)	24.1	0.0		0.8		
Approach LOS	C			A		
Intersection Summary						
Average Delay			2.5			
Intersection Capacity Utilization			48.5%			
Analysis Period (min)			15			
				ICU Level of Service		A

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 11: Oak Street & Dreamers Way

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			W		W	
Traffic Volume (veh/h)	0	55	40	0	15	45
Future Volume (Veh/h)	0	55	40	0	15	45
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	59	43	0	16	48
Pedestrians		25	10		40	
Lane Width (m)		3.5	3.5		3.0	
Walking Speed (m/s)		1.1	1.1		1.1	
Percent Blockage		2	1		3	
Right turn flare (veh)						
Median type		None	None		None	
Median storage (veh)						
Upstream signal (m)						
PX platoon unblocked					152	108
VC conflicting volume	83					
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol	83				152	108
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)						
IF (s)	2.2				3.5	3.3
p0 queue free %	100				98	95
CM capacity (veh/h)	1481				872	902
Direction Lane #	EB 1	WB 1	SB 1			
Volumes Total	59	43	64			
Volume Left	0	0	16			
Volume Right	0	0	48			
CSH	1700	1700	878			
Volumes to Capacity	0.03	0.03	0.07			
Queue Length 95th (m)	0.0	0.0	1.8			
Control Delay (s)	0.0	0.0	9.4			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	9.4			
Approach LOS			A			
Intersection Summary						
Average Delay			3.6			
Intersection Capacity Utilization			27.2%			
Analysis Period (min)			15			
				ICU Level of Service		A



12: Regent Street & Oak Street  
 HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	EBT	EBR	WBL	WBT	NBL	NBR
Traffic Volume (veh/h)	70	0	0	25	15	45
Future Volume (Veh/h)	70	0	0	25	15	45
Sign Control	Free	Stop	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	84	0	0	30	18	54
Pedestrians	15			15	40	
Lane Width (m)	3.5			3.5	3.0	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	1			1	3	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume		124		169		139
VC1, stage 1 conf vol						
VC2, stage 2 conf vol		124		169		139
IC, single (s)		4.1		6.4		6.2
IC, 2 stage (s)		2.2		3.5		3.3
p0 queue free %		100		98		94
CM capacity (veh/h)		1431		790		875
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	84	30	72			
Volume Left	0	0	18			
Volume Right	0	0	54			
cSH	1700	1700	852			
Volume to Capacity	0.05	0.02	0.08			
Queue Length 95th (m)	0.0	0.0	2.1			
Control Delay (s)	0.0	0.0	9.6			
Lane LOS	A	A	A			
Approach Delay (s)	0.0	0.0	9.6			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utilization			27.2%		ICU Level of Service	A
Analysis Period (min)			15			

14: Sackville Street & Oak Street  
 HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Volume (vph)	0	85	20	0	5	0	0	0	0	0	35	85
Future Volume (vph)	0	85	20	0	5	0	0	0	0	0	35	85
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	94	22	0	6	0	0	0	0	0	39	94
Direction, Lane #	EB 1	WB 1	SB 1									
Volume Total (vph)	116	6	139									
Volume Left (vph)	0	0	39									
Volume Right (vph)	22	0	6									
Head (s)	-0.09	0.00	0.10									
Departure Headway (s)	4.1	4.3	4.3									
Degree Utilization, x	0.13	0.01	0.16									
Capacity (veh/h)	840	785	816									
Control Delay (s)	7.8	7.4	8.1									
Approach Delay (s)	7.8	7.4	8.1									
Approach LOS	A	A	A									
Intersection Summary												
Delay			7.9									
Level of Service			A									
Intersection Capacity Utilization			32.7%				ICU Level of Service					A
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 15: Sumach Street & Oak Street

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop		Stop		Stop		Stop		Stop		Stop	
Traffic Volume (vph)	45	60	15	0	0	0	5	80	75	0	0	0
Future Volume (vph)	45	60	15	0	0	0	5	80	75	0	0	0
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	49	66	16	0	0	0	5	88	82	0	0	0
Direction, Lane #												
Volume Total (vph)	EB 1		NB 1									
Volume Left (vph)	131		175									
Volume Right (vph)	49		82									
Hadj (s)	0.07		-0.20									
Departure Headway (s)	4.3		4.0									
Degree Utilization, x	0.16		0.19									
Capacity (veh/h)	797		868									
Control Delay (s)	8.2		8.0									
Approach Delay (s)	8.2		8.0									
Approach LOS	A		A									
Intersection Summary												
Delay	8.0											
Level of Service	A											
Intersection Capacity Utilization	34.6%		ICU Level of Service								A	
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 16: Tubman Avenue/Site Laneway & Oak Street

10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	125	10	0	0	0	0	0	0	0	5	5
Future Volume (Veh/h)	0	125	10	0	0	0	0	0	0	0	5	5
Sign Control	Free		Free		Free		Stop		Stop		Stop	
Grade	0%		0%		0%		0%		0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	134	11	0	0	0	0	0	0	0	5	5
Pedestrians	20		5		5		30		30		0	
Lane Width (m)	3.5		0.0		0.0		0.0		0.0		0.0	
Walking Speed (m/s)	1.1		1.1		1.1		1.1		1.1		1.1	
Percent Blockage	2		0		0		0		0		0	
Right turn flare (veh)	None		None		None		None		None		None	
Median type	None		None		None		None		None		None	
Median storage (veh)	None		None		None		None		None		None	
Upstream signal (m)	None		None		None		None		None		None	
dx, platoon unblocked	None		None		None		None		None		None	
VC, conflicting volume	0	175	175	192	170	174	144	175	20	20	175	20
VC1, stage 1 conf vol	0		175		175		192		170		174	
VC2, stage 2 conf vol	0		175		175		192		170		174	
VCu, unblocked vol	4.1	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	7.1	6.5	6.2
IC, single (s)	2.2	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	3.5	4.0	3.3
IC, 2 stage (s)	100	100	100	100	100	100	99	99	100	99	99	100
p0 queue free %	1636	1414	1414	754	727	874	829	722	1045	1045	722	1045
dM capacity (veh/h)	EB 1		SB 1									
Direction, Lane #	EB 1		SB 1									
Volume Total	145		10									
Volume Left	0		5									
Volume Right	11		0									
cSH	1700		772									
Volume to Capacity	0.09		0.01									
Queue Length 95th (m)	0.0		0.3									
Control Delay (s)	0.0		9.7									
Lane LOS	A		A									
Approach Delay (s)	0.0		9.7									
Approach LOS	A		A									
Intersection Summary												
Average Delay	0.6											
Intersection Capacity Utilization	26.7%		ICU Level of Service								A	
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
17: Oak Street & Site Driveway

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	125	0	0	10	0
Future Volume (Veh/h)	5	125	0	0	10	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	6	142	0	0	11	0
Pedestrians	5	5	5	5	25	5
Lane Width (m)	3.5	0.0	3.0	3.0	1.1	1.1
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	0	0	0	0	2	2
Right turn flare (veh)	None	None	None	None	None	None
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked					184	30
VC, conflicting volume	25					
VC1, stage 1 conf vol						
VCu, unblocked vol	25				184	30
IC, single (s)	4.1				6.4	6.2
IC, 2 stage (s)						
IF (s)	2.2				3.5	3.3
p0 queue free %	100				99	100
CM capacity (veh/h)	1572				792	1026
Direction, Lane #	EB 1	SB 1				
Volumes Total	148	11				
Volume Left	6	11				
Volume Right	0	0				
cSH	1572	792				
Volumes to Capacity	0.00	0.01				
Queue Length 95th (m)	0.1	0.3				
Control Delay (s)	0.3	9.6				
Lane LOS	A	A				
Approach Delay (s)	0.3	9.6				
Approach LOS	A	A				
Intersection Summary						
Average Delay		1.0				
Intersection Capacity Utilization		22.2%			ICU Level of Service	A
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
18: River Street & Oak Street

10-25-2022

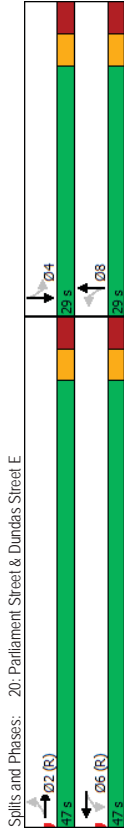
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations											
Traffic Volume (veh/h)	85	5	45	20	0	50	0	435	40	50	560
Future Volume (Veh/h)	85	5	45	20	0	50	0	435	40	50	560
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	92	5	49	22	0	54	0	473	43	54	609
Pedestrians	75	75	45	45	45	50	50	3.5	3.5	3.5	5
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	1.1	1.1	1.1	1.1	1.1	1.1
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	7	7	4	4	4	4	4	4	4	4	0
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None	None
Median type	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)											
Upstream signal (m)								143			151
pX platoon unblocked	0.85	0.85	0.80	0.85	0.85	0.90	0.80			0.90	
VC, conflicting volume	1346	1353	734	1358	1332	544	684			561	
VC1, stage 1 conf vol											
VC2, stage 2 conf vol											
VCu, unblocked vol	1060	1069	541	1075	1044	442	478			460	
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1	
IC, 2 stage (s)											
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2	
p0 queue free %	28	97	87	81	100	90	100			94	
CM capacity (veh/h)	127	160	382	116	166	535	816			964	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1							
Volumes Total	146	76	516	663							
Volume Left	92	22	0	54							
Volume Right	49	54	43	0							
cSH	165	262	1700	964							
Volumes to Capacity	0.88	0.29	0.30	0.06							
Queue Length 95th (m)	47.8	8.9	0.0	1.4							
Control Delay (s)	97.1	24.3	0.0	1.4							
Lane LOS	F	C	A	A							
Approach Delay (s)	97.1	24.3	0.0	1.4							
Approach LOS	F	C	A	A							
Intersection Summary											
Average Delay			12.1								
Intersection Capacity Utilization			83.6%			ICU Level of Service					E
Analysis Period (min)			15								

19: Parliament Street & Cole Street  
 HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			←	←	←	←
Traffic Volume (veh/h)	0	0	520	20	25	390
Future Volume (Veh/h)	0	0	520	20	25	390
Sign Control	Stop	Stop	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	0	0	619	24	30	464
Pedestrians	255	0	25	0	15	0
Lane Width (m)	0.0	0.0	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	0	0	2	2	1	1
Right turn flare (veh)						
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)			80	80	214	214
pX platoon unblocked	0.89	0.89	592	592	898	898
VC, conflicting volume						
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	990	306	649	649	4.1	4.1
IC, single (s)	6.8	6.9	3.3	2.2	2.2	2.2
IC, 2 stage (s)	3.5	3.3	100	100	96	96
p0 queue free %	208	614	846	846		
CM capacity (veh/h)						
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	413	230	185	309		
Volume Left	0	0	30	0		
Volume Right	0	24	0	0		
cSH	1700	1700	846	1700		
Volumes to Capacity	0.24	0.14	0.04	0.18		
Queue Length 95th (m)	0.0	0.0	0.8	0.0		
Control Delay (s)	0.0	0.0	1.8	0.0		
Lane LOS			A			
Approach Delay (s)	0.0	0.0	0.7			
Approach LOS						
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			43.5%			
ICU Level of Service			A			
Analysis Period (min)			15			

20: Parliament Street & Dundas Street E  
 2032 Future Background PM Model (FutRN)  
 10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations			←	←	←	←	←	←
Traffic Volume (vph)	65	490	45	190	45	405	45	320
Future Volume (vph)	65	490	45	190	45	405	45	320
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	2	6	6	8	8	4	4
Permitted Phases	2	2	6	6	8	8	4	4
Detector Phase								
Switch Phase								
Minimum Initial (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Minimum Split (s)	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
47.0 Split (s)	47.0	47.0	47.0	47.0	47.0	47.0	29.0	29.0
Total Split (%)	61.8%	61.8%	61.8%	61.8%	38.2%	38.2%	38.2%	38.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode								
Act Effct Green (s)	41.3	41.3	C-Min	C-Min	Min	Min	Min	Min
Actuated g/C Ratio	0.54	0.54	0.54	0.54	0.32	0.32	0.32	0.32
v/c Ratio	0.45	0.45	0.26	0.26	0.59	0.59	0.51	0.51
Control Delay	11.5	12.1	12.1	23.7	22.5	22.5	22.5	22.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.5	12.1	12.1	23.7	22.5	22.5	22.5	22.5
LOS	B	B	B	C	C	C	C	C
Approach Delay	11.5	12.1	12.1	23.7	22.5	22.5	22.5	22.5
Approach LOS	B	B	B	C	C	C	C	C
Intersection Summary								
Cycle Length: 76								
Actuated Cycle Length: 76								
Offset: 9 (12%) Referenced to phase 2:EBTL and 6:WBTL Start of 1st Green								
Natural Cycle: 60								
Control Type: Actuated-Coordinated								
Maximum v/c Ratio: 0.59								
Intersection Signal Delay: 17.3								
Intersection Capacity Utilization 93.3%								
Analysis Period (min) 15								



Queues 2032 Future Background PM Model (FutRN) 10-25-2022

20: Parliament Street & Dundas Street E

	EBT	WBT	NBT	SBT
Lane Group	691	343	551	439
Lane Group Flow (vph)	0.45	0.26	0.69	0.51
v/c Ratio	11.5	12.1	23.7	22.5
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	11.5	12.1	23.7	22.5
Total Delay	27.3	9.8	34.0	26.2
Queue Length 50th (m)	43.3	27.8	45.3	36.1
Queue Length 95th (m)	85.0	103.5	45.0	56.1
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1558	1347	935	867
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.44	0.25	0.69	0.51
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN) 10-25-2022

20: Parliament Street & Dundas Street E

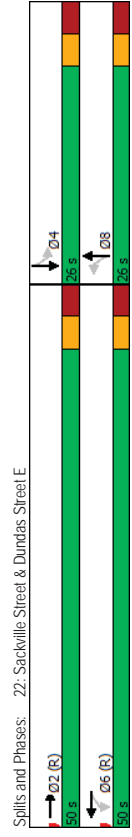
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4TB			4TB			4TB					
Traffic Volume (vph)	65	490	60	45	190	70	45	405	40	45	320	25	
Future Volume (vph)	65	490	60	45	190	70	45	405	40	45	320	25	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	0.0	5.0	5.0	0.0	5.0	0.0	0.0	5.0	0.0	0.0	5.0	0.0	
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Frbp. ped/bikes	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	
Frbp. psd/bikes	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	
Frt	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Flt Protected	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Satd. Flow (prot)	3205	3007	3007	3272	3272	3272	3272	3272	3272	3272	3197	3197	
Flt Permitted	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	
Satd. Flow (perm)	2802	2802	2802	2425	2425	2425	2853	2853	2853	2853	2649	2649	
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	
Adj. Flow (vph)	73	551	67	51	213	79	51	465	45	51	360	28	
RTOR Reduction (vph)	0	11	0	0	9	0	0	9	0	0	0	0	
Lane Group Flow (vph)	0	680	0	0	334	0	0	542	0	0	432	0	
Confl. Peds. (#/hr)	260	155	155	260	165	225	225	225	225	225	165	165	
Confl. Bikes (#/hr)	25	25	25	20	20	20	20	20	20	20	20	20	
Heavy Vehicles (%)	3%	6%	5%	5%	6%	6%	3%	5%	0%	3%	8%	10%	
Parking (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0	
Turn Type	Perm	NA	NA	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Permitted Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Actuated Green, G (s)	40.3	40.3	40.3	40.3	40.3	40.3	23.7	23.7	23.7	23.7	23.7	23.7	
Effective Green, g (s)	41.3	41.3	41.3	41.3	41.3	41.3	24.7	24.7	24.7	24.7	24.7	24.7	
Actuated g/C Ratio	0.54	0.54	0.54	0.54	0.54	0.54	0.32	0.32	0.32	0.32	0.32	0.32	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1522	1522	1522	1317	1317	1317	927	927	927	927	860	860	
v/s Ratio Prot													
v/s Ratio Perm	0.24	0.14	0.14	0.14	0.14	0.14	0.19	0.19	0.16	0.16	0.16	0.16	
v/c Ratio	0.45	0.25	0.25	0.25	0.25	0.25	0.58	0.58	0.50	0.50	0.50	0.50	
Uniform Delay, d1	10.5	9.2	9.2	9.2	9.2	9.2	21.4	21.4	20.7	20.7	20.7	20.7	
Progression Factor	1.00	1.00	1.00	1.29	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	1.0	0.5	0.5	0.5	0.5	0.5	0.9	0.9	0.5	0.5	0.5	0.5	
Delay (s)	11.4	12.3	12.3	12.3	12.3	12.3	22.3	22.3	21.2	21.2	21.2	21.2	
Level of Service	B	B	B	B	B	B	C	C	C	C	C	C	
Approach Delay (s)	11.4	12.3	12.3	12.3	12.3	12.3	22.3	22.3	21.2	21.2	21.2	21.2	
Approach LOS	B	B	B	B	B	B	C	C	C	C	C	C	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	16.7											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.50												
Actuated Cycle Length (s)	76.0											Sum of lost time (s)	10.0
Intersection Capacity Utilization	93.3%											ICU Level of Service	F
Analysis Period (min)	15												
c Critical Lane Group													

21: Regent Street & Dundas Street E  
 HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	40	505	15	5	295	25	5	10	5	5	5	25
Traffic Volume (veh/h)	40	505	15	5	295	25	5	10	5	5	5	25
Future Volume (Veh/h)	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Sign Control	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Grade	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Peak Hour Factor	43	543	16	5	317	27	5	11	5	5	5	27
Hourly flow rate (vph)	30	30	30	10	10	10	125	125	125	135	135	135
Pedestrians	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Lane Width (m)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Walking Speed (m/s)	3	3	3	3	3	3	11	11	11	12	12	12
Percent Blockage	None	None	None	None	None	None	None	None	None	None	None	None
Right turn flare (veh)	127	127	127	124	124	124	124	124	124	124	124	124
Median storage (veh)	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Upstream signal (m)	684	684	684	684	684	684	990	1251	414	854	1246	337
pX platoon unblocked	479	479	479	479	479	479	479	479	479	479	479	479
VC, conflicting volume	479	479	479	479	479	479	479	479	479	479	479	479
VC1, stage 1 conf vol	479	479	479	479	479	479	479	479	479	479	479	479
VC2, stage 2 conf vol	479	479	479	479	479	479	479	479	479	479	479	479
VCU, unblocked vol	4.1	4.1	4.1	4.4	4.4	4.4	7.5	6.5	7.9	7.5	6.5	6.9
IC, single (s)	2.2	2.2	2.2	2.4	2.4	2.4	3.5	4.0	3.8	3.5	4.0	3.3
IC, 2 stage (s)	96	96	96	99	99	99	97	92	99	97	96	95
p0 queue free %	963	963	963	773	773	773	149	140	498	192	141	570
CM capacity (veh/h)	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2	SB 1	SB 2				
Direction, Lane #	314	288	164	186	21	37						
Volumes Total	43	0	5	0	5	5						
Volume Left	0	16	0	27	5	27						
Volume Right	963	1700	773	1700	171	339						
cSH	0.04	0.17	0.01	0.11	0.12	0.11						
Volumes to Capacity	1.1	0.0	0.1	0.0	3.1	2.8						
Queue Length 95th (m)	1.6	0.0	0.4	0.0	28.9	16.9						
Control Delay (s)	A	A	A	D	C	C						
Lane LOS	0.9	0.2	0.2	28.9	16.9	16.9						
Approach Delay (s)	D	D	C									
Approach LOS												
Intersection Summary												
Average Delay	1.8											
Intersection Capacity Utilization	49.1%											A
Analysis Period (min)	15											

22: Sackville Street & Dundas Street E  
 2032 Future Background PM Model (FutRN)  
 10-25-2022

Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4	4	4	4	4	4	4
Traffic Volume (vph)	495	15	245	40	0	35	35
Future Volume (vph)	495	15	245	40	0	35	35
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	6	6	8	8	4	4
Permitted Phases	2	6	6	8	8	4	4
Detector Phase							
Switch Phase							
Minimum Initial (s)	17.0	17.0	17.0	7.0	7.0	7.0	7.0
Minimum Split (s)	24.0	24.0	24.0	26.0	26.0	26.0	26.0
Total Split (s)	50.0	50.0	50.0	26.0	26.0	26.0	26.0
Total Split (%)	65.8%	65.8%	65.8%	34.2%	34.2%	34.2%	34.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag							
Lead-Lag Optimize?							
Recall Mode	C-Min	C-Min	C-Min	None	None	None	None
Ad Effct Green (s)	47.6	47.6	47.6	18.4	18.4	18.4	18.4
Actuated g/C Ratio	0.63	0.63	0.63	0.24	0.24	0.24	0.24
v/C Ratio	0.27	0.26	0.26	0.23	0.23	0.23	0.23
Control Delay	5.1	7.0	7.0	13.2	17.8	17.8	17.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	5.1	7.0	7.0	13.2	17.8	17.8	17.8
LOS	A	A	A	B	B	B	B
Approach Delay	5.1	7.0	7.0	13.2	17.8	17.8	17.8
Approach LOS	A	A	A	B	B	B	B
Intersection Summary							
Cycle Length: 76							
Actuated Cycle Length: 76							
Offset: 25 (33%), Referenced to phase 2EBT and 6WBTL Start of 1st Green							
Natural Cycle: 50							
Control Type: Actuated-Coordinated							
Maximum v/C Ratio: 0.31							
Intersection Signal Delay: 7.7							
Intersection Capacity Utilization 49.4%							
Analysis Period (min) 15							



Queues 2032 Future Background PM Model (FutRN) 10-25-2022  
 22: Sackville Street & Dundas Street E

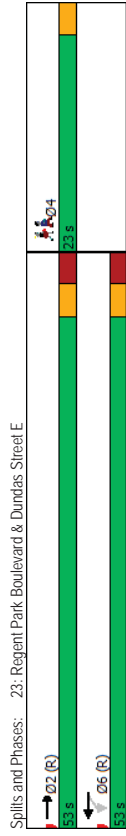
	EBT	WBT	NBT	SBT
Lane Group	564	279	75	119
Lane Group Flow (vph)	0.27	0.26	0.23	0.31
v/c Ratio	5.1	7.0	13.2	17.8
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	5.1	7.0	13.2	17.8
Total Delay	12.7	17.6	3.4	9.0
Queue Length 50th (m)	16.5	21.8	12.9	21.6
Queue Length 95th (m)	99.8	79.6	36.2	126.8
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2059	1061	367	427
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.27	0.26	0.20	0.28
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN) 10-25-2022  
 22: Sackville Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	0	495	30	15	245	0	40	0	30	35	35	40
Future Volume (vph)	0	495	30	15	245	0	40	0	30	35	35	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.0	3.5
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.98	1.00	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.97	0.97	0.97
Frbp. psd/bikes	1.00	0.99	1.00	0.99	1.00	0.96	0.96	0.96	0.96	0.97	0.97	0.97
Frt	1.00	0.99	1.00	1.00	1.00	0.94	0.94	0.94	0.94	0.95	0.95	0.95
Flt Protected	1.00	1.00	1.00	1.00	1.00	0.97	0.97	0.97	0.97	0.98	0.98	0.98
Satd. Flow (prot)	3290	1755	1755	1755	1495	1495	1495	1495	1495	1609	1609	1609
Flt Permitted	1.00	0.96	1.00	0.96	1.00	0.79	0.79	0.79	0.79	0.89	0.89	0.89
Satd. Flow (perm)	3290	1694	1694	1694	1220	1220	1220	1220	1220	1452	1452	1452
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	0	532	32	16	263	0	43	0	32	38	38	43
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	564	0	0	279	0	42	0	42	0	0	91
Conf. Peds. (#/hr)	105	175	175	175	105	70	55	55	55	55	55	70
Conf. Bikes (#/hr)	35	35	35	35	20	20	20	20	20	20	20	20
Heavy Vehicles (%)	0%	5%	6%	0%	6%	0%	0%	0%	12%	0%	2%	5%
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	2	6	6	6	6	8	8	8	8	8	8	8
Permitted Phases												
Actuated Green, G (s)	46.6	46.6	46.6	46.6	46.6	17.4	17.4	17.4	17.4	17.4	17.4	17.4
Effective Green, g (s)	47.6	47.6	47.6	47.6	47.6	18.4	18.4	18.4	18.4	18.4	18.4	18.4
Actuated g/C Ratio	0.63	0.63	0.63	0.63	0.63	0.24	0.24	0.24	0.24	0.24	0.24	0.24
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	2060	1060	1060	1060	295	295	295	295	295	351	351	351
v/s Ratio Prot	c0.17											
v/s Ratio Perm	0.27	0.26	0.26	0.26	0.14	0.14	0.14	0.14	0.14	0.26	0.26	0.26
v/c Ratio	6.4	6.4	6.4	6.4	22.6	22.6	22.6	22.6	22.6	23.3	23.3	23.3
Uniform Delay, d1	0.67	0.87	0.87	0.87	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	0.3	0.6	0.6	0.6	0.2	0.2	0.2	0.2	0.2	0.4	0.4	0.4
Incremental Delay, d2	4.6	6.1	6.1	6.1	22.8	22.8	22.8	22.8	22.8	23.7	23.7	23.7
Delay (s)	A	A	A	A	C	C	C	C	C	C	C	C
Level of Service	A	A	A	A	A	A	A	A	A	A	A	A
Approach Delay (s)	4.6	6.1	6.1	6.1	22.8	22.8	22.8	22.8	22.8	23.7	23.7	23.7
Approach LOS	A	A	A	A	A	A	A	A	A	A	A	A
<b>Intersection Summary</b>												
HCM 2000 Control Delay	8.5 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.27											
Actuated Cycle Length (s)	76.0 Sum of lost time (s) 10.0											
Intersection Capacity Utilization	49.4% ICU Level of Service A											
Analysis Period (min)	15											
c Critical Lane Group												

Timings 2032 Future Background PM Model (FutRN) 10-25-2022  
 23: Regent Park Boulevard & Dundas Street E

Lane Group	EBT	WBL	WBT	Ø4
Lane Configurations	1	1	1	4
Traffic Volume (vph)	530	15	260	
Future Volume (vph)	530	15	260	
Turn Type	NA	Perm	NA	4
Protected Phases	2	6	6	4
Permitted Phases	2	6	6	
Detector Phase				
Switch Phase				
Minimum Initial (s)	18.0	18.0	18.0	20.0
Minimum Split (s)	23.9	23.9	23.9	23.0
Total Split (s)	53.0	53.0	53.0	23.0
Total Split (%)	69.7%	69.7%	69.7%	30%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.9	4.9	4.9	
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	C-Min	C-Min	C-Min	None
Act Effct Green (s)	64.8	64.8	64.8	
Actuated g/C Ratio	0.85	0.85	0.85	
v/c Ratio	0.43	0.21	0.21	
Control Delay	3.2	4.0	4.0	
Queue Delay	0.0	0.0	0.0	
Total Delay	3.3	4.0	4.0	
LOS	A	A	A	
Approach Delay	3.3	4.0	4.0	
Approach LOS	A	A	A	
<b>Intersection Summary</b>				
Cycle Length: 76				
Actuated Cycle Length: 76				
Offset: 22 (29%), Referenced to phase 2EBT and 6WBT, Start of 1st Green				
Natural Cycle: 60				
Control Type: Actuated-Coordinated				
Maximum v/c Ratio: 0.43				
Intersection Signal Delay: 3.5				
Intersection Capacity Utilization 49.8%				
Analysis Period (min) 15				



Queues 2032 Future Background PM Model (FutRN) 10-25-2022  
 23: Regent Park Boulevard & Dundas Street E

Lane Group	EBT	WBT
Lane Group Flow (vph)	630	309
v/c Ratio	0.43	0.21
Control Delay	3.2	4.0
Queue Delay	0.0	0.0
Total Delay	3.3	4.0
Queue Length 50th (m)	1.7	0.0
Queue Length 95th (m)	51.7	28.6
Internal Link Dist (m)	79.6	86.0
Turn Bay Length (m)		
Base Capacity (vph)	1462	1448
Starvation Cap Reductn	41	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.44	0.21
<b>Intersection Summary</b>		



23: Regent Park Boulevard & Dundas Street E

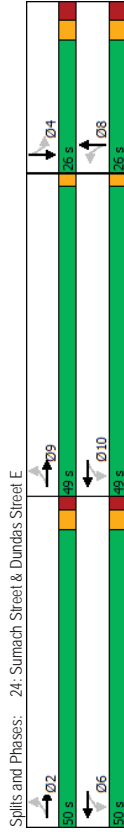
2032 Future Background PM Model (FuTRN)  
10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4	4	4	4	4	4
Traffic Volume (vph)	530	30	15	260	0	0
Future Volume (vph)	530	30	15	260	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.0	3.0	3.5	3.0	3.0
Total Lost time (s)	4.9		4.9			
Lane Util. Factor	1.00		1.00			
Frbp. ped/bikes	0.97		1.00			
Frbp. ped/bikes	1.00		0.99			
Frt	0.99		1.00			
Flt Protected	1.00		1.00			
Sat'd Flow (prot)	1712		1755			
Flt Permitted	1.00		0.96			
Sat'd Flow (perm)	1712		1695			
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	596	34	17	292	0	0
RTOR Reduction (vph)	1	0	0	0	0	0
Lane Group Flow (vph)	629	0	0	309	0	0
Conf. Peds. (#/hr)	235	235	60	65		
Heavy Vehicles (%)	6%	6%	0%	6%	0%	0%
Turn Types	NA	Perm	NA			
Protected Phases	2		6			
Permitted Phases			6			
Actuated Green, G (s)	59.1		59.1			
Effective Green, g (s)	60.1		60.1			
Actuated g/C Ratio	0.79		0.79			
Clearance Time (s)	5.9		5.9			
Vehicle Extension (s)	3.0		3.0			
Lane Grp Cap. (vph)	1353		1340			
v/s Ratio Prot	0.37		0.37			
v/s Ratio Perm	0.18		0.18			
w/s Ratio	0.46		0.23			
Uniform Delay, d1	2.6		2.0			
Progression Factor	0.50		1.00			
Incremental Delay, d2	1.1		0.4			
Delay (s)	2.4		2.4			
Level of Service	A		A			
Approach Delay (s)	2.4		2.4	0.0		
Approach LOS	A		A	A		
Intersection Summary						
HCM 2000 Control Delay		2.4				A
HCM 2000 Volume to Capacity ratio		0.41				
Actuated Cycle Length (s)		76.0				7.9
Intersection Capacity Utilization		49.8%				A
Analysis Period (min)		15				
c. Critical Lane Group						

24: Sumach Street & Dundas Street E

2032 Future Background PM Model (FuTRN)  
10-25-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4	4	4	4	4	4	4	4
Traffic Volume (vph)	45	465	30	245	25	90	15	10
Future Volume (vph)	45	465	30	245	25	90	15	10
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2, 9		6, 10		8		4	
Permitted Phases	2		6		8		4	
Switch Phase								
Minimum Initial (s)	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
Minimum Split (s)	26.0	26.0	26.0	26.0	21.0	21.0	49.0	49.0
Total Split (s)	26.0	26.0	26.0	26.0	26.0	26.0	50.0	49.0
Total Split (%)	20.8%	20.8%	20.8%	20.8%	20.8%	20.8%	40%	39%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
Lost Time Adjust (s)			-1.0		-1.0			
Total Lost Time (s)			5.0		5.0			
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode			None		None		Min	None
Act Effct Green (s)	62.8		62.8		22.8		22.8	
Actuated g/C Ratio	0.66		0.66		0.24		0.24	
w/C Ratio	0.54		0.37		0.47		0.10	
Control Delay	9.3		6.7		40.8		33.2	
Queue Delay	2.2		0.0		0.0		0.0	
Total Delay	11.5		6.7		40.8		33.2	
LOS	B		A		D		C	
Approach Delay	11.5		6.7		40.8		33.2	
Approach LOS	B		A		D		C	
Intersection Summary								
Cycle Length	125							
Actuated Cycle Length	95.4							
Natural Cycle	100							
Control Type	Actuated-Uncoordinated							
Maximum w/C Ratio	0.54							
Intersection Signal Delay	15.1							
Intersection Capacity Utilization	60.9%							
Analysis Period (min)	15							



Queues 2032 Future Background PM Model (FutRN) 10-25-2022  
 24: Sumach Street & Dundas Street E

	EBT	WBT	NBT	SBT
Lane Group	570	354	172	32
Lane Group Flow (vph)	0.54	0.37	0.47	0.10
v/c Ratio	9.3	6.7	40.8	33.2
Control Delay	2.2	0.0	0.0	0.0
Queue Delay	11.5	6.7	40.8	33.2
Total Delay	44.5	21.8	28.7	4.4
Queue Length 50th (m)	64.7	33.5	55.9	13.8
Queue Length 95th (m)	86.0	75.5	76.2	121.5
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1194	1093	364	328
Station Cap Reductn	466	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.78	0.32	0.47	0.10
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN) 10-25-2022  
 24: Sumach Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4	4		4	4		4			4	4	
Traffic Volume (vph)	45	465	20	30	245	55	25	90	45	15	10	5	
Future Volume (vph)	45	465	20	30	245	55	25	90	45	15	10	5	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frbp. ped/bikes	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	
Frbp. psd/bikes	0.99	0.99	0.99	0.98	0.98	0.98	0.96	0.96	0.96	0.96	0.96	0.96	
Frt	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.98	0.98	0.98	0.98	
Flt Protected	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	
Satd. Flow (prot)	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	1.688	
Flt Permitted	0.95	0.95	0.95	0.93	0.93	0.93	0.95	0.95	0.95	0.95	0.95	0.95	
Satd. Flow (perm)	1604	1604	1604	1478	1478	1478	1519	1519	1519	1519	1386	1386	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Adj. Flow (vph)	48	500	22	32	263	59	27	97	48	16	11	5	
RTOR Reduction (vph)	0	2	0	0	7	0	0	10	0	0	4	0	
Lane Group Flow (vph)	0	568	0	0	347	0	0	162	0	0	28	0	
Confl. Peds. (#/hr)	100	185	185	100	105	100	105	50	50	50	105	105	
Confl. Bikes (#/hr)	25	25	25	20	20	20	20	20	20	20	20	20	
Heavy Vehicles (%)	12%	6%	8%	0%	9%	5%	4%	5%	6%	0%	9%	0%	
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	NA	
Protected Phases	2.9	6.10	6.10	8	8	8	8	8	8	4	4	4	
Permitted Phases	2.9	6.10	6.10	8	8	8	8	8	8	4	4	4	
Actuated Green, G (s)	65.4	65.4	65.4	65.4	65.4	65.4	65.4	65.4	65.4	21.7	21.7	21.7	
Effective Green, g (s)	66.4	66.4	66.4	66.4	66.4	66.4	66.4	66.4	66.4	22.7	22.7	22.7	
Actuated g/C Ratio	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.70	0.24	0.24	0.24	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1119	1119	1119	1031	1031	1031	362	362	362	330	330	330	
v/s Ratio Prot	60.35	60.35	60.35	0.23	0.23	0.23	60.11	60.11	60.11	0.02	0.02	0.02	
v/c Ratio Perm	0.51	0.51	0.51	0.34	0.34	0.34	0.45	0.45	0.45	0.09	0.09	0.09	
v/c Ratio	6.7	6.7	6.7	5.7	5.7	5.7	30.9	30.9	30.9	28.1	28.1	28.1	
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Progression Factor	0.4	0.4	0.4	0.2	0.2	0.2	0.9	0.9	0.9	0.1	0.1	0.1	
Incremental Delay, d2	7.1	7.1	7.1	5.9	5.9	5.9	31.7	31.7	31.7	28.2	28.2	28.2	
Delay (s)	A	A	A	A	A	A	C	C	C	C	C	C	
Level of Service	7.1	7.1	7.1	5.9	5.9	5.9	31.7	31.7	31.7	28.2	28.2	28.2	
Approach Delay (s)	A	A	A	A	A	A	C	C	C	C	C	C	
Approach LOS	A	A	A	A	A	A	C	C	C	C	C	C	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	11.1											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.51												
Actuated Cycle Length (s)	95.1											Sum of lost time (s)	10.0
Intersection Capacity Utilization	60.9%											ICU Level of Service	B
Analysis Period (min)	15												
c Critical Lane Group													

25: Tubman Avenue & Dundas Street E

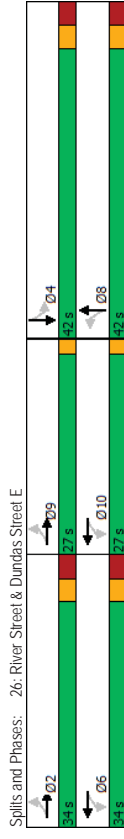
26: River Street & Dundas Street E

10-25-2022 HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FuTRN)

10-25-2022 2032 Future Background PM Model (FuTRN)

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	515	10	10	325	0	0	0	0	10	0	5
Future Volume (Veh/h)	0	515	10	10	325	0	0	0	0	10	0	5
Sign Control	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	542	11	11	342	0	0	0	0	11	0	5
Pedestrians	10						130					50
Lane Width (m)	3.5						0.0					3.5
Walking Speed (m/s)	1.1						1.1					1.1
Percent Blockage	1						0					4
Right turn flare (veh)												
Median type	None											
Median storage (veh)												
Upstream signal (m)	99						98					
pX platoon unblocked												
VC, conflicting volume	392						1056	1092	688	972	1097	402
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	392						987	1028	561	889	1034	402
IC, single (s)	4.1						7.1	6.5	6.2	7.1	6.5	6.2
IC, 2 stage (s)												
IF (s)	2.2						3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100						100	100	100	95	100	99
CM capacity (veh/h)	1125						186	193	455	209	191	618
Direction, Lane #	EB 1	WB 1	SB 1									
Volume Total	553	353	16									
Volume Left	0	11	11									
Volume Right	11	0	5									
cSH	1125	887	263									
Volume to Capacity	0.00	0.01	0.06									
Queue Length 95th (m)	0.0	0.3	1.5									
Control Delay (s)	0.0	0.4	19.6									
Lane LOS	A	A	C									
Approach Delay (s)	0.0	0.4	19.6									
Approach LOS	C	C	C									
Intersection Summary												
Average Delay			0.5									
Intersection Capacity Utilization			43.9%							A		
Analysis Period (min)			15									

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø6	Ø9	Ø10
Lane Configurations													
Traffic Volume (vph)	25	440	95	210	45	365	105	445					
Future Volume (vph)	25	440	95	210	45	365	105	445					
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA					
Protected Phases	2, 9		6, 10		8		4						
Permitted Phases	2	2	6	6	8	8	4	4					
Detector Phase													
Switch Phase													
Minimum Initial (s)					21.0	21.0	21.0	21.0	21.0	21.0	21.0	5.0	5.0
Minimum Split (s)					27.0	27.0	27.0	27.0	27.0	27.0	27.0	27.0	27.0
Total Split (s)					42.0	42.0	42.0	42.0	34.0	34.0	34.0	27.0	27.0
Total Split (%)					40.8%	40.8%	40.8%	40.8%	33%	33%	33%	26%	26%
Yellow Time (s)					3.0	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
All-Red Time (s)					3.0	3.0	3.0	3.0	3.0	3.0	3.0	0.0	0.0
Lost Time Adjust (s)					-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	0.0	0.0
Total Lost Time (s)					5.0	5.0	5.0	5.0					
Lead-Lag													
Lead-Lag Optimize?													
Recall Mode													
Act Effct Green (s)		31.8			37.7		37.7		37.7				
Actuated g/C Ratio		0.40			0.47		0.47		0.47				
v/C Ratio		0.82			0.85		0.79		0.58				
Control Delay		31.5			37.3		28.2		34.8				
Queue Delay		0.0			0.0		0.0		0.0				
Total Delay		31.5			37.3		19.6		28.2				
LOS		C			D		B		C				
Approach Delay		31.5			37.3		27.6		27.4				
Approach LOS		C			D		C		C				
Intersection Summary													
Cycle Length		103											
Actuated Cycle Length		79.7											
Natural Cycle		95											
Control Type		Actuated-Uncoordinated											
Maximum v/C Ratio		0.85											
Intersection Signal Delay		30.3											
Intersection Capacity Utilization		120.3%											
Analysis Period (min)		15											



Queues 2032 Future Background PM Model (FutRN) 10-25-2022

26: River Street & Dundas Street E

	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group	552	421	47	610	111	552
Lane Group Flow (vph)	0.82	0.85	0.21	0.79	0.58	0.72
v/c Ratio	31.5	37.3	19.6	28.2	34.8	26.0
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	31.5	37.3	19.6	28.2	34.8	26.0
Total Delay	70.0	52.4	3.7	64.4	10.6	57.3
Queue Length 50th (m)	103.3	86.0	16.6	#192.0	#49.6	#170.6
Queue Length 95th (m)	73.4	80.5		131.0		118.9
Internal Link Dist (m)			20.0		25.0	
Turn Bay Length (m)						
Base Capacity (vph)	711	523	227	777	193	769
Station Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.78	0.80	0.21	0.79	0.58	0.72

**Intersection Summary**  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN) 10-25-2022

26: River Street & Dundas Street E

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		+	+									
Traffic Volume (vph)	25	440	60	95	210	95	45	365	215	105	445	80
Future Volume (vph)	25	440	60	95	210	95	45	365	215	105	445	80
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0						5.0			5.0		5.0
Lane Util. Factor	1.00						1.00			1.00		1.00
Frbp. ped/bikes	0.98			0.97			1.00	0.94		1.00	0.97	
Frbp. psd/bikes	1.00			0.99			0.96	1.00		0.96	1.00	
Frt	1.00			0.97			1.00	0.94		1.00	0.98	
Flt Protected	1.00			0.99			0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1724			1637			1533	1633		1591	1624	
Flt Permitted	0.97			0.73			0.28	1.00		0.23	1.00	
Satd. Flow (perm)	1676			1214			460	1633		393	1624	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	26	463	63	100	221	100	47	384	226	111	468	84
RTOR Reduction (vph)	0	5	0	0	13	0	0	17	0	0	5	0
Lane Group Flow (vph)	0	547	0	0	408	0	47	593	0	111	547	0
Confl. Peds. (#/hr)	55	75	75	75	75	55	75	75	75	75	75	55
Confl. Bikes (#/hr)			25			20			10			5
Heavy Vehicles (%)	9%	5%	0%	1%	9%	0%	5%	3%	0%	2%	12%	0%
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	Perm
Protected Phases		2.9		6.10			8				4	
Permitted Phases		2.9		6.10			8				4	
Actuated Green, G (s)		36.5		36.5			36.7		36.7		36.7	
Effective Green, g (s)		37.5		37.5			37.7		37.7		37.7	
Actuated g/C Ratio		0.46		0.46			0.46		0.46		0.46	
Clearance Time (s)							6.0		6.0		6.0	
Vehicle Extension (s)							3.0		3.0		3.0	
Lane Grp Cap (vph)		774		560			213		758		182	
v/s Ratio Prot							0.36				0.34	
v/s Ratio Perm		0.33		0.34			0.10		0.28		0.28	
v/c Ratio		0.71		0.73			0.22		0.78		0.61	
Uniform Delay, d1		17.5		17.7			13.0		18.3		16.3	
Progression Factor		1.00		1.00			1.00		1.00		1.00	
Incremental Delay, d2		3.0		4.7			0.5		5.3		5.7	
Delay (s)		20.4		22.4			13.5		23.6		21.9	
Level of Service		C		C			B		C		C	
Approach Delay (s)		20.4		22.4			22.9		21.2		21.2	
Approach LOS		C		C			C		C		C	
Intersection Summary												
HCM 2000 Control Delay		21.7		HCM 2000 Level of Service			C					
HCM 2000 Volume to Capacity ratio		0.79										
Actuated Cycle Length (s)		81.2		Sum of lost time (s)			11.0					
Intersection Capacity Utilization		120.3%		ICU Level of Service			H					
Analysis Period (min)		15										
c Critical Lane Group												

Regent Park Phases 4 & 5  
 BA Group - NHTY  
 Synchro 11 Report  
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Queues 2032 Future Background PM Model (FutRN) 10-25-2022

26: River Street & Dundas Street E

	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group	552	421	47	610	111	552
Lane Group Flow (vph)	0.82	0.85	0.21	0.79	0.58	0.72
v/c Ratio	31.5	37.3	19.6	28.2	34.8	26.0
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	31.5	37.3	19.6	28.2	34.8	26.0
Total Delay	70.0	52.4	3.7	64.4	10.6	57.3
Queue Length 50th (m)	103.3	86.0	16.6	#192.0	#49.6	#170.6
Queue Length 95th (m)	73.4	80.5		131.0		118.9
Internal Link Dist (m)			20.0		25.0	
Turn Bay Length (m)						
Base Capacity (vph)	711	523	227	777	193	769
Station Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.78	0.80	0.21	0.79	0.58	0.72

**Intersection Summary**  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

Regent Park Phases 4 & 5  
 BA Group - NHTY  
 Synchro 11 Report  
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HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 27: Dreamer's Way & Gerrard Street E

10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	830	55	10	430	0	0
Future Volume (Veh/h)	830	55	10	430	0	0
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	874	58	11	453	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None	None	None	None	None	None
Median type						
Median storage (veh)	90			162		
Upstream signal (m)	0.87	0.87	0.87	0.87	0.87	0.87
pX platoon unblocked	932	1152	466			
VC, conflicting volume						
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	619	871	82			
IC, single (s)	4.1	6.8	6.9			
IC, 2 stage (s)	2.2	3.5	3.3			
p0 queue free %	99	100	100			
IF (s)	844	252	840			
CM capacity (veh/h)						
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volumes Total	583	349	162	302		
Volume Left	0	0	11	0		
Volume Right	0	58	0	0		
cSH	1700	1700	844	1700		
Volumes to Capacity	0.34	0.21	0.01	0.18		
Queue Length 95th (m)	0.0	0.0	0.3	0.0		
Control Delay (s)	0.0	0.0	0.8	0.0		
Lane LOS	A	A	A	A		
Approach Delay (s)	0.0	0.3				
Approach LOS						
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization		28.0%			ICU Level of Service	A
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 29: Sumach Street & Site Driveway

10-25-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	0	15	125	0	0	0
Future Volume (Veh/h)	0	15	125	0	0	0
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	16	132	0	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None	None	None	None	None	None
Median type						
Median storage (veh)						55
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	132	132			132	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	132	132			132	
IC, single (s)	6.4	6.2			4.1	
IC, 2 stage (s)	3.5	3.3			2.2	
p0 queue free %	100	98			100	
IF (s)	867	923			1466	
CM capacity (veh/h)						
Direction, Lane #	WB 1	NB 1				
Volumes Total	16	132				
Volume Left	0	0				
Volume Right	16	0				
cSH	923	1700				
Volumes to Capacity	0.02	0.08				
Queue Length 95th (m)	0.4	0.0				
Control Delay (s)	9.0	0.0				
Lane LOS	A	A				
Approach Delay (s)	9.0	0.0				
Approach LOS	A	A				
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization		16.6%		ICU Level of Service		A
Analysis Period (min)		15				

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 31: Site Laneway & Gerrard Street E

10-25-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔↔	↔	↔	↔↔	↔	↔
Traffic Volume (veh/h)	950	0	0	565	0	5
Future Volume (Veh/h)	950	0	0	565	0	5
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	1000	0	0	595	0	5
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)	171			80		
pX platoon unblocked		0.90		0.93		0.90
VC conflicting volume		1000		1298		500
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol		773		838		216
IC single (s)		4.1		6.8		6.9
IC 2 stage (s)		2.2		3.5		3.3
p0 queue free %		100		100		99
CM capacity (veh/h)		765		288		714
Direction_Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volumes Total	667	333	298	298	5	5
Volume Left	0	0	0	0	0	0
Volume Right	0	0	0	0	5	5
cSH	1700	1700	1700	1700	714	714
Volumes to Capacity	0.39	0.20	0.17	0.17	0.01	0.01
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.2	0.2
Control Delay (s)	0.0	0.0	0.0	0.0	10.1	10.1
Lane LOS	A	A	A	A	B	B
Approach Delay (s)	0.0	0.0	0.0	10.1		
Approach LOS	A	A	A	B		
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	36.3%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 2032 Future Background PM Model (FutRN)  
 32: Oak Street & Site Laneway

10-25-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔	↔	↔	↔
Traffic Volume (veh/h)	5	110	25	0	0	0
Future Volume (Veh/h)	5	110	25	0	0	0
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	5	116	26	0	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume		26			152	26
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCU unblocked vol		26			152	26
IC single (s)		4.1			6.4	6.2
IC 2 stage (s)		2.2			3.5	3.3
p0 queue free %		100			100	100
CM capacity (veh/h)		1601			842	1056
Direction_Lane #	EB 1	WB 1	SB 1			
Volumes Total	121	26	0			
Volume Left	5	0	0			
Volume Right	0	0	0			
cSH	1601	1700	1700			
Volumes to Capacity	0.00	0.02	0.00			
Queue Length 95th (m)	0.1	0.0	0.0			
Control Delay (s)	0.3	0.0	0.0			
Lane LOS	A	A	A			
Approach Delay (s)	0.3	0.0	0.0			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	0.3					
Intersection Capacity Utilization	13.2%					
ICU Level of Service	A					
Analysis Period (min)	15					

2032 Future Total AM Model  
11-07-2022

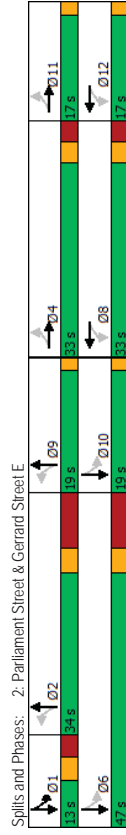
2: Parliament Street & Gerrard Street E

2032 Future Total AM Model  
11-07-2022

2: Parliament Street & Gerrard Street E

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				4TB	4TB	
Traffic Volume (veh/h)	0	0	5	575	495	60
Future Volume (Veh/h)	0	0	5	575	495	60
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	5	625	538	65
Pedestrians	80					5
Lane Width (m)	0.0					3.5
Walking Speed (m/s)	1.1					1.1
Percent Blockage	0					0
Right turn flare (veh)				None	None	
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						39
pX platoon unblocked	0.93					
VC, conflicting volume	978	382	683			
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	820	382	683			
IC, single (s)	6.8	6.9	4.5			
IC, 2 stage (s)						
IF (s)	3.5	3.3	2.4			
p0 queue free %	100	100	99			
CM capacity (veh/h)	291	622	795			
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volume Total	213	417	359	244		
Volume Left	5	0	0	0		
Volume Right	0	0	0	65		
cSH	795	1700	1700	1700		
Volumes to Capacity	0.01	0.25	0.21	0.14		
Queue Length 95th (m)	0.1	0.0	0.0	0.0		
Control Delay (s)	0.3	0.0	0.0	0.0		
Lane LOS	A					
Approach Delay (s)	0.1		0.0			
Approach LOS						
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	22.7%					
Analysis Period (min)	15					
ICU Level of Service	A					

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		4TB		4TB		4TB		4TB
Traffic Volume (vph)	25	245	70	545	110	345	85	340
Future Volume (vph)	25	245	70	545	110	345	85	340
Turn Type	Perm	NA	Perm	NA	Perm	NA	custom	NA
Protected Phases	4 11		8 12		2 9		1 16	2 4
Permitted Phases	4	4	8	8	2	2	1	16
Switch Phase								
Minimum Initial (s)							6.0	22.0
Minimum Split (s)							12.5	33.2
Total Split (s)							13.0	34.0
Total Split (%)							11.2%	29%
Yellow Time (s)							3.3	3.0
All-Red Time (s)							3.2	7.9
Lost Time Adjust (s)								2.8
Total Lost Time (s)								2.8
Lead/Lag							Lead	Lag
Recall Mode							Yes	Yes
Ad Effct Green (s)	33.5		33.5		30.4		48.8	
Actuated g/C Ratio	0.36		0.36		0.33		0.52	
v/C Ratio	0.35		0.84		0.70		0.38	
Control Delay	20.9		34.6		32.7		13.5	
Queue Delay	0.0		0.0		0.0		0.0	
Total Delay	20.9		34.6		32.7		13.5	
LOS	C		C		C		B	
Approach Delay	20.9		34.6		32.7		13.5	
Approach LOS	C		C		C		B	
Intersection Summary								
Cycle Length	116							
Actuated Cycle Length	93							
Natural Cycle	115							
Control Type	Actuated-Uncoordinated							
Maximum v/C Ratio	0.84							
Intersection Signal Delay	27.2							
Intersection Capacity Utilization	100.9%							
Analysis Period (min)	15							



Lane Group	Ø9	Ø10	Ø11	Ø12
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Turn Type	9	10	11	12
Protected Phases				
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0	5.0	5.0	5.0
Minimum Split (s)	19.0	19.0	17.0	17.0
Total Split (s)	19.0	19.0	17.0	17.0
Total Split (%)	16%	16%	15%	15%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	None	None
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Intersection Summary				



Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	363	869	526	521
v/c Ratio	0.35	0.84	0.70	0.38
Control Delay	20.9	34.6	32.7	13.5
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	20.9	34.6	32.7	13.5
Queue Length 50th (m)	18.7	60.2	37.2	21.2
Queue Length 95th (m)	38.5	110.8	68.4	43.5
Internal Link Dist (m)	33.0	63.9	119.2	15.0
Turn Bay Length (m)				
Base Capacity (vph)	1042	1065	763	1396
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.35	0.82	0.69	0.37
Intersection Summary				

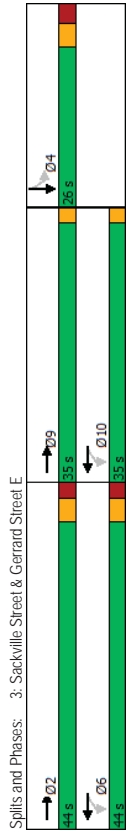


2. Parliament Street & Gerrard Street E  
 HCM Signalized Intersection Capacity Analysis  
 2032 Future Total AM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB			4TB		4TB		4TB	
Traffic Volume (vph)	25	245	75	70	545	210	110	345	45	85	340	70
Future Volume (vph)	25	245	75	70	545	210	110	345	45	85	340	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frb. ped/bikes	0.98	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.97	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Flt Protected	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Sat'd. Flow (prot)	3312	3197	3174	3174	3174	3174	3174	3174	3174	3199	3199	3199
Flt Permitted	0.86	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Sat'd. Flow (perm)	2854	2803	2298	2298	2298	2298	2298	2298	2298	2518	2518	2518
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	26	258	79	74	574	221	116	363	47	89	358	74
RTOR Reduction (vph)	0	21	0	0	28	0	0	6	0	0	13	0
Lane Group Flow (vph)	0	342	0	0	841	0	0	520	0	0	508	0
Conf. Peds. (#/hr)	80	85	85	80	85	80	85	120	120	120	85	85
Conf. Bikes (#/hr)			5		20				5		10	
Heavy Vehicles (%)	13%	0%	2%	1%	1%	11%	6%	8%	0%	11%	6%	2%
Turn Type	Perim	MA	Perm	NA	Perm	NA	Perm	NA	Perm	custom	MA	Perm
Protected Phases	4 11		8 12		2 9					1 6 10		
Permitted Phases				8 12		2 9				6 10		
Actuated Green, G (s)	38.0	39.0	38.0	39.0	39.0	40.6	41.6	41.6	44.6	44.6	53.8	53.8
Effective Green, g (s)	0.41	0.41	0.41	0.41	0.41	0.43	0.43	0.43	0.47	0.47	0.47	0.47
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1161		1141		997					1226		
v/s Ratio Prot			c0.30		60.23					c0.03		
v/s Ratio Perm			0.74		0.52					0.16		
v/c Ratio	0.29	0.74	0.29	0.29	0.74	0.29	0.29	0.29	0.41	0.41	0.41	0.41
Uniform Delay, d1	19.1	24.1	19.8	19.8	24.1	19.8	19.8	19.8	17.0	17.0	17.0	17.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	2.5	0.5	0.5	2.5	0.5	0.5	0.5	0.2	0.2	0.2	0.2
Delay (s)	19.3	26.6	20.3	20.3	26.6	20.3	20.3	20.3	17.2	17.2	17.2	17.2
Level of Service	B	C	C	C	C	C	C	C	B	B	B	B
Approach Delay (s)	19.3	26.6	20.3	20.3	26.6	20.3	20.3	20.3	17.2	17.2	17.2	17.2
Approach LOS	B	C	C	C	C	C	C	C	B	B	B	B
Intersection Summary												
HCM 2000 Control Delay	21.8 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.72											
Actuated Cycle Length (s)	95.8 Sum of lost time (s)											
Intersection Capacity Utilization	100.9% ICU Level of Service G											
Analysis Period (min)	15											
c Critical Lane Group												

3. Sackville Street & Gerrard Street E  
 Timings  
 2032 Future Total AM Model  
 11-07-2022

Lane Group	EBT	WBL	WBT	SBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	4TB		4TB					
Traffic Volume (vph)	335	140	875	35				
Future Volume (vph)	335	140	875	35				
Turn Type	NA	Perm	NA	NA				
Protected Phases	2 9		6 10	4 2	6	9	10	
Permitted Phases								
Detector Phase	2	6	6	4				
Switch Phase								
Minimum Initial (s)			20.0	16.0	16.0	5.0	5.0	
Minimum Split (s)			25.7	21.0	21.0	35.0	35.0	
Total Split (s)			26.0	44.0	44.0	35.0	35.0	
Total Spill (%)			24.8%	42%	49%	33%	33%	
Yellow Time (s)			3.0	3.0	3.0	2.0	2.0	
All-Red Time (s)			2.7	2.0	2.0	0.0	0.0	
Lost Time Adjust (s)			-1.0					
Total Lost Time (s)			4.7					
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode			Min	Min	Min	None	None	
Act Effct Green (s)	53.2	53.2	22.6					
Actuated g/C Ratio	0.62	0.62	0.62	0.27				
v/c Ratio	0.20	0.66	0.28					
Control Delay	5.7	10.9	33.3					
Queue Delay	0.0	0.0	0.0					
Total Delay	5.7	10.9	33.3					
LOS	A	B	C					
Approach Delay	5.7	10.9	33.3					
Approach LOS	A	B	C					
Intersection Summary								
Cycle Length: 105								
Actuated Cycle Length: 85.2								
Natural Cycle: 85								
Control Type: Actuated-Uncoordinated								
Maximum v/c Ratio: 0.66								
Intersection Signal Delay: 11.2								
Intersection Capacity Utilization: 68.8%								
Analysis Period (min): 15								
ICU Level of Service: C								



Queues  
3: Sackville Street & Gerrard Street E

2032 Future Total AM Model  
11-07-2022

	EBT	WBT	SBT
Lane Group	395	1116	114
Lane Group Flow (vph)	0.20	0.66	0.28
v/c Ratio	5.7	10.9	33.3
Control Delay	0.0	0.0	0.0
Queue Delay	5.7	10.9	33.3
Total Delay	11.3	49.8	17.5
Queue Length 50th (m)	16.0	65.0	35.3
Queue Length 95th (m)	58.8	47.8	15.3
Internal Link Dist (m)			
Turn Bay Length (m)	2231	1882	414
Base Capacity (vph)	0	0	0
Starvation Cap Reductn	0	0	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.18	0.59	0.28
<b>Intersection Summary</b>			

HCM Signalized Intersection Capacity Analysis  
3: Sackville Street & Gerrard Street E

2032 Future Total AM Model  
11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4+3			4+1							4+3
Traffic Volume (vph)	0	335	25	140	875	0	0	0	0	55	35	15
Future Volume (vph)	0	335	25	140	875	0	0	0	0	55	35	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)		4.0			4.0							4.7
Lane Util. Factor		0.95			0.95							1.00
Frbp. ped/bikes		0.99			1.00							0.99
Frbp. psd/bikes		1.00			0.99							0.91
Frt		0.99			1.00							0.98
Flt Protected		1.00			0.99							0.97
Sat'd. Flow (prot)		3191			3343							1561
Flt Permitted		1.00			0.81							0.97
Sat'd. Flow (perm)		3191			2730							1561
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	0	368	27	154	962	0	0	0	0	60	38	16
RTOR Reduction (vph)	0	6	0	0	0	0	0	0	0	0	0	5
Lane Group Flow (vph)	0	389	0	0	1116	0	0	0	0	0	0	109
Confl. Peds. (#/hr)	40	60	60	60	40	60	140	140	60	140	60	60
Confl. Bikes (#/hr)		15			75							5
Heavy Vehicles (%)	0%	10%	0%	9%	4%	0%	0%	0%	0%	0%	6%	6%
Turn Type	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA
Protected Phases	2.9			6.10						4		
Permitted Phases				6.10								4
Actuated Green, G (s)	56.4			56.4						21.6		
Effective Green, g (s)	57.4			57.4						22.6		
Actuated g/C Ratio	0.67			0.67						0.26		
Clearance Time (s)												5.7
Vehicle Extension (s)												3.0
Lane Grp Cap (vph)	2137			1828						411		
v/s Ratio Prot	0.12											
v/s Ratio Perm				0.41								
v/c Ratio	0.18			0.61						0.07		
Uniform Delay, d1	5.3			7.9						25.0		
Progression Factor	1.00			1.00						1.00		
Incremental Delay, d2	0.0			0.6						0.3		
Delay (s)	5.4			8.5						25.3		
Level of Service	A			A						C		
Approach Delay (s)	5.4			8.5			0.0			25.3		
Approach LOS	A			A			A			C		
<b>Intersection Summary</b>												
HCM 2000 Control Delay	8.9		HCM 2000 Level of Service		A							
HCM 2000 Volume to Capacity ratio	0.53											
Actuated Cycle Length (s)	85.7											
Sum of lost time (s)	9.7											
Intersection Capacity Utilization	68.8%											
ICU Level of Service	C											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 4: Gerrard Street E & Gifford Street

HCM Unsignalized Intersection Capacity Analysis  
 5: Gerrard Street E & Nasmith Avenue

11-07-2022

11-07-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4+1	1+1		W	
Traffic Volume (veh/h)	5	385	1010	20	5	5
Future Volume (Veh/h)	5	385	1010	20	5	5
Sign Control		Free	Free	Stop	Stop	
Grade		0%	0%	0%	0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	5	401	1052	21	5	5
Pedestrians		5	5		50	
Lane Width (m)		3.5	3.5	3.0		
Walking Speed (m/s)		1.1	1.1	1.1		
Percent Blockage		0	0		4	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)						
Upstream signal (m)		72	141			
pX platoon unblocked	0.86			0.87	0.86	
VC, conflicting volume	1123			1328	592	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	817			970	198	
IC, single (s)	4.1			7.5	7.3	
IC, 2 stage (s)						
p0 queue free %	2.2			3.8	3.5	
IF (s)	99			97	99	
CM capacity (veh/h)	678			167	623	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	139	267	701	372	10	
Volume Left	5	0	0	0	5	
Volume Right	0	0	0	21	5	
cSH	678	1700	1700	1700	263	
Volumes to Capacity	0.01	0.16	0.41	0.22	0.04	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.9	
Control Delay (s)	0.5	0.0	0.0	0.0	19.2	
Lane LOS	A				C	
Approach Delay (s)	0.2		0.0		19.2	
Approach LOS					C	
Intersection Summary						
Average Delay			0.2			
Intersection Capacity Utilization			40.2%		ICU Level of Service	
Analysis Period (min)			15		A	



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4+1	1+1		W	
Traffic Volume (veh/h)	5	385	1020	5	15	10
Future Volume (Veh/h)	5	385	1020	5	15	10
Sign Control		Free	Free	Stop	Stop	
Grade		0%	0%	0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	5	397	1052	5	15	10
Pedestrians					75	
Lane Width (m)					3.0	
Walking Speed (m/s)					1.1	
Percent Blockage					6	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)						
Upstream signal (m)		145	68			
pX platoon unblocked	0.84			0.84	0.84	0.84
VC, conflicting volume	1132			1338	604	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	773			1018	143	
IC, single (s)	4.1			6.9	7.1	
IC, 2 stage (s)						
p0 queue free %	99			92	99	
IF (s)	2.2			3.6	3.4	
CM capacity (veh/h)	674			177	676	
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volumes Total	137	265	701	356	25	
Volume Left	5	0	0	0	15	
Volume Right	0	0	0	5	10	
cSH	674	1700	1700	1700	251	
Volumes to Capacity	0.01	0.16	0.41	0.21	0.10	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	2.5	
Control Delay (s)	0.5	0.0	0.0	0.0	20.9	
Lane LOS	A				C	
Approach Delay (s)	0.2		0.0		20.9	
Approach LOS					C	
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization			38.4%		ICU Level of Service	
Analysis Period (min)			15		A	

Timings 2032 Future Total AM Model 11-07-2022

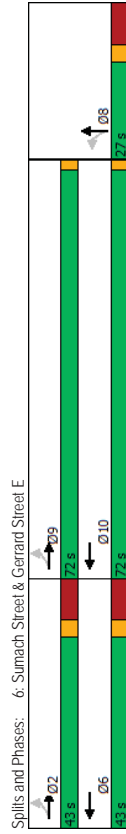
Queues 2032 Future Total AM Model 11-07-2022

6: Summach Street & Gerrard Street E

6: Summach Street & Gerrard Street E

Lane Group	EBL	EBT	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations		4↑	1↑	4↑				
Traffic Volume (vph)	5	395	920	10				
Future Volume (vph)	5	395	920	10				
Turn Type	Perm	NA	NA	NA				
Protected Phases		2, 9	6, 10	8	2	6	9	10
Permitted Phases	2, 9							
Detector Phase	2	2	6	8				
Switch Phase								
Minimum Initial (s)				15.0	11.0	5.0	5.0	
Minimum Split (s)				25.4	26.2	72.0	72.0	
Total Split (s)				27.0	43.0	72.0	72.0	
Total Split (%)				19.0%	30%	51%	51%	
Yellow Time (s)				3.0	3.0	2.0	2.0	
All-Red Time (s)				7.4	7.2	7.2	0.0	0.0
Lost Time Adjust (s)				-1.0				
Total Lost Time (s)				9.4				
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode				None	Min	None	None	
Act Effct Green (s)		56.4	56.4	20.0				
Actuated g/C Ratio		0.58	0.58	0.20				
v/c Ratio		0.25	0.64	0.82				
Control Delay		8.5	12.6	61.4				
Queue Delay		0.0	0.3	0.0				
Total Delay		8.5	12.9	61.4				
LOS		A	B	E				
Approach Delay		8.5	12.9	61.4				
Approach LOS		A	B	E				
Intersection Summary								
Cycle Length	142							
Actuated Cycle Length	97.6							
Natural Cycle	125							
Control Type	Actuated-Uncoordinated							
Maximum v/c Ratio	0.82							
Intersection Signal Delay	18.0							
Intersection Capacity Utilization	66.4%							
Analysis Period (min)	15							

Lane Group	EBT	WBT	NBT
Lane Group Flow (vph)	460	1201	241
v/c Ratio	0.25	0.64	0.82
Control Delay	8.5	12.6	61.4
Queue Delay	0.0	0.3	0.0
Total Delay	8.5	12.9	61.4
Queue Length 50th (m)	18.0	62.1	25.6
Queue Length 95th (m)	22.5	70.4	#127.9
Internal Link Dist (m)	44.0	75.7	33.2
Turn Bay Length (m)			
Base Capacity (vph)	1901	1950	294
Starvation Cap Reductn	0	253	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.24	0.71	0.82
Intersection Summary			
#	95th percentile volume exceeds capacity, queue may be longer.		
	Queue shown is maximum after two cycles.		



HCM Signalized Intersection Capacity Analysis  
 6: Sumach Street & Gerrard Street E  
 2032 Future Total AM Model  
 11-07-2022

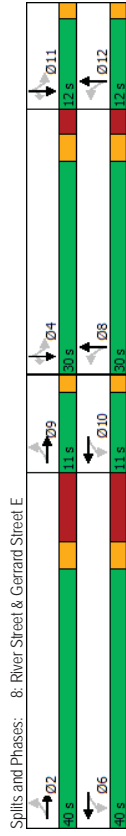
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4+1			1+1			4+1				
Traffic Volume (vph)	5	395	0	0	920	125	105	10	95	0	0	0
Future Volume (vph)	5	395	0	0	920	125	105	10	95	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	9.2			9.2			9.4					
Lane Util. Factor	0.95			0.95			1.00					
Frbp. ped/bikes	1.00			0.97			0.94					
Frbp. ped/bikes	1.00			1.00			0.90					
Frt	1.00			0.98			0.94					
Flt Protected	1.00			1.00			0.98					
Sat'd. Flow (prot)	3399			3279			1456					
Flt Permitted	0.94			1.00			0.98					
Sat'd. Flow (perm)	3185			3279			1456					
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	6	454	0	0	1057	144	121	11	109	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	19	0	0	0	0
Lane Group Flow (vph)	0	460	0	0	1192	0	0	222	0	0	0	0
Conf. Bikes (#/hr)	65	85	85	65	100		65	65	65			100
Heavy Vehicles (%)	0%	5%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perim	MA		NA		Perm	NA		NA			
Protected Phases		2.9		6.10			8					
Permitted Phases		2.9					8					
Effective Green, G (s)		65.2		65.2			18.9					
Actuated Green, g (s)		66.2		66.2			19.9					
Actuated g/C Ratio		0.69		0.69			0.21					
Clearance Time (s)							10.4					
Vehicle Extension (s)							3.0					
Lane Grp Cap (vph)		2184		2249			300					
v/s Ratio Prot				c0.36								
v/s Ratio Perm		0.14					0.15					
v/c Ratio		0.21		0.53			0.74					
Uniform Delay, d1		5.6		7.5			35.9					
Progression Factor		1.00		1.00			1.00					
Incremental Delay, d2		0.0		0.2			9.2					
Delay (s)		5.6		7.7			45.1					
Level of Service		A		A			D					0.0
Approach Delay (s)		5.6		7.7			45.1					0.0
Approach LOS		A		A			D					A
Intersection Summary												
HCM 2000 Control Delay				11.9			HCM 2000 Level of Service					B
HCM 2000 Volume to Capacity ratio				0.64								
Actuated Cycle Length (s)				96.5			Sum of lost time (s)					19.6
Intersection Capacity Utilization				66.4%			ICU Level of Service					C
Analysis Period (min)				15								
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 7: Site Laneway/Sword Street & Gerrard Street E  
 2032 Future Total AM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1+1			1+1							
Traffic Volume (veh/h)	0	480	10	0	1040	0	0	0	0	10	0	5
Future Volume (Veh/h)	0	480	10	0	1040	0	0	0	0	10	0	5
Sign Control		Free			Free			Stop				Stop
Grade		0%			0%			0%				0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	505	11	0	1095	0	0	0	0	11	0	5
Pedestrians												75
Lane Width (m)												3.0
Walking Speed (m/s)												1.1
Percent Blockage												6
Right turn flare (veh)												
Median type							None					
Median storage (veh)												
Upstream signal (m)		100			91							
pX platoon unblocked	0.79			0.97			0.81	0.81	0.97	0.81	0.81	0.81
vC, conflicting volume	1170			516			1063	1680	258	1422	1686	622
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	697			448			454	1218	183	899	1225	8
IC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	6.9
IC, 2 stage (s)												
IF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	100	100	94	100	99
dM capacity (veh/h)	681			1093			379	139	812	173	137	808
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1							
Volumes Total	337	179	548	548	16							
Volume Left	0	0	0	0	11							
Volume Right	0	11	0	0	5							
cSH	1700	1700	1700	1700	229							
Volumes to Capacity	0.20	0.11	0.32	0.32	0.07							
Queue Length 95th (m)	0.0	0.0	0.0	0.0	1.7							
Control Delay (s)	0.0	0.0	0.0	0.0	21.9							
Lane LOS					C							
Approach Delay (s)	0.0	0.0	0.0	0.0	21.9							
Approach LOS					C							
Intersection Summary												
Average Delay				0.2								
Intersection Capacity Utilization				38.7%								A
Analysis Period (min)				15								

Timings 2032 Future Total AM Model 11-07-2022  
8: River Street & Gerrard Street E

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Lane Configurations												
Traffic Volume (vph)	130	275	165	825	45	360	125	365	170			
Future Volume (vph)	130	275	165	825	45	360	125	365	170			
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm			
Protected Phases	2.9		6.10		8.12		4.11		4.11	2	4	6
Permitted Phases	2	2	6	6	8	8	4	4	4			
Detector Phase												
Switch Phase												
Minimum Initial (s)										23.0	19.0	23.0
Minimum Spilt (s)										33.8	30.0	33.8
Total Spilt (s)										40.0	30.0	40.0
Total Spilt (%)										43%	32%	43%
Yellow Time (s)										3.0	3.0	3.0
All-Red Time (s)										7.8	3.0	7.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode										Min	Min	Min
Act Effct Green (s)	36.8		36.8	29.9	29.9	29.9	29.9	29.9	29.9			
Actuated g/C Ratio	0.45		0.45	0.37	0.37	0.37	0.37	0.37	0.37			
v/C Ratio	0.858d		1.06	0.19	0.71	0.64	0.56	0.30				
Control Delay	23.1		66.9	20.8	28.8	38.2	24.6	8.9				
Queue Delay	0.0		0.0	0.0	0.0	0.0	0.0	0.0				
Total Delay	23.1		66.9	20.8	28.8	38.2	24.6	8.9				
LOS	C		E	C	C	D	C	A				
Approach Delay	23.1		66.9		28.0		23.1					
Approach LOS	C		E	C	C	C	C					
Intersection Summary												
Cycle Length: 93												
Actuated Cycle Length: 81.9												
Natural Cycle: 100												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 1.06												
Intersection Signal Delay: 42.3												
Intersection Capacity Utilization: 118.1%												
Analysis Period (min): 15												
di Defacto Left Lane. Recode with 1 though lane as a left lane.												



Timings 2032 Future Total AM Model 11-07-2022  
8: River Street & Gerrard Street E

Lane Group	Ø8	Ø9	Ø10	Ø11	Ø12
Lane Configurations					
Traffic Volume (vph)					
Future Volume (vph)					
Turn Type					
Protected Phases	8	9	10	11	12
Permitted Phases					
Detector Phase					
Switch Phase					
Minimum Initial (s)	19.0	5.0	5.0	5.0	5.0
Minimum Spilt (s)	30.0	11.0	11.0	12.0	12.0
Total Spilt (s)	30.0	11.0	11.0	12.0	12.0
Total Spilt (%)	32%	12%	12%	13%	13%
Yellow Time (s)	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)	3.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	Min	None	None	None	None
Actuated g/C Ratio					
v/C Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Intersection Summary					

	EBT	WBT	NBL	NBT	SBL	SBR
Lane Group	510	1219	47	453	130	380
Lane Group Flow (vph)	0.85dl	1.06	0.19	0.71	0.64	0.56
v/c Ratio	23.1	66.9	20.8	28.8	38.2	24.6
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	23.1	66.9	20.8	28.8	38.2	24.6
Total Delay	31.4	-109.9	5.4	62.7	17.9	50.4
Queue Length 50th (m)	54.2	#167.6	13.2	95.6	38.9	76.7
Queue Length 95th (m)	67.0	81.6		25.1		61.7
Internal Link Dist (m)			30.0		30.0	
Turn Bay Length (m)	750	1153	252	667	213	708
Base Capacity (vph)	0	0	0	0	0	0
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.68	1.06	0.19	0.68	0.61	0.54
0.54	0.29					
<b>Intersection Summary</b>						
-	Volume exceeds capacity, queue is theoretically infinite.					
-	Queue shown is maximum after two cycles.					
#	95th percentile volume exceeds capacity, queue may be longer.					
-	Queue shown is maximum after two cycles.					
dl	Defacto Left Lane. Record with 1 though lane as a left lane.					

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations		4TB		4TB			4TB			4TB	
Traffic Volume (vph)	130	275	85	165	825	180	45	360	75	125	365
Future Volume (vph)	130	275	85	165	825	180	45	360	75	125	365
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5
Total Lost time (s)	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8	9.8
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Frbp. psd/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Flt Protected	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Satd. Flow (prot)	3109	3266	1528	1734	1577	1860	1420				
Flt Permitted	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52	0.52
Satd. Flow (perm)	1631	2539	665	1734	562	1860	1420				
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	135	286	89	172	859	188	47	375	78	130	380
RTOR Reduction (vph)	0	15	0	0	13	0	0	8	0	0	0
Lane Group Flow (vph)	0	495	0	0	1206	0	47	445	0	130	380
Conf. Peds. (#/hr)	50	95	95	95	95	50	40	85	85	40	10
Conf. Bikes (#/hr)	10	10	10	10	10	10	10	10	10	10	10
Heavy Vehicles (%)	4%	8%	4%	0%	3%	1%	8%	4%	2%	3%	1%
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	2.9	2.9	2.9	2.9	2.9	2.9	2.9	2.9	2.9	2.9	2.9
Permitted Phases	6.10	6.10	6.10	6.10	6.10	6.10	6.10	6.10	6.10	6.10	6.10
Actuated Green, G (s)	45.5	45.5	45.5	45.5	45.5	45.5	45.5	45.5	45.5	45.5	45.5
Effective Green, g (s)	46.5	46.5	46.5	46.5	46.5	46.5	46.5	46.5	46.5	46.5	46.5
Actuated g/C Ratio	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56	0.56
Clearance Time (s)											
Vehicle Extension (s)											
Lane Grp Cap (vph)	910	1417	277	724	234	777	593				
v/s Ratio Prot											
v/s Ratio Perm	0.30	0.48	0.07	0.26	0.23	0.20					
v/c Ratio	0.85dl	0.85	0.17	0.62	0.56	0.49	0.19				
Uniform Delay, d1	11.7	15.5	19.0	18.4	17.7	15.3					
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00					
Incremental Delay, d2	0.7	5.1	0.3	1.6	2.8	0.5	0.2				
Delay (s)	12.3	20.6	19.3	20.0	20.5	15.8					
Level of Service	B	C	B	C	C	B					
Approach Delay (s)	12.3	20.6	20.1	20.1	18.1						
Approach LOS	B	C	C	C	B						
<b>Intersection Summary</b>											
HCM 2000 Control Delay	18.5 HCM 2000 Level of Service B										
HCM 2000 Volume to Capacity ratio	0.88										
Actuated Cycle Length (s)	83.3										
Intersection Capacity Utilization	118.1% Sum of lost time (s) 16.8										
Analysis Period (min)	15										
dl	Defacto Left Lane. Record with 1 though lane as a left lane.										
c	Critical Lane Group										

HCM Unsignalized Intersection Capacity Analysis  
 9: Sackville Street & Site Driveway  
 2032 Future Total AM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	80	55	0	0	0	0	0	0	20	155
Future Volume (Veh/h)	0	0	80	55	0	0	0	0	0	0	20	155
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Hourly flow rate (vph)	0	0	104	71	0	0	0	0	0	0	26	201
Pedestrians	70	70	70	70	70	70	15	15	15	15	10	10
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	0.0	0.0	0.0	0.0	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	6	6	6	6	6	6	0	0	0	0	1	1
Right turn flare (veh)												
Median type							None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)												58
pX platoon unblocked												
VC, conflicting volume	349	379	302	428	395	50	303				40	
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	349	379	302	428	395	50	303				40	
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	100	100	85	83	100	100	100				98	
CM capacity (veh/h)	519	495	696	406	485	979	1191				1527	
Direction, Lane #	EB 1	WB 1	SB 1									
Volumes Total	104	71	259									
Volume Left	0	71	26									
Volume Right	104	0	32									
cSH	696	406	1527									
Volumes to Capacity	0.15	0.17	0.02									
Queue Length 95th (m)	4.0	4.8	0.4									
Control Delay (s)	11.1	15.7	0.9									
Lane LOS	B	C	A									
Approach Delay (s)	11.1	15.7	0.9									
Approach LOS	B	C	C									
Intersection Summary												
Average Delay			5.7									
Intersection Capacity Utilization			35.5%									A
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis  
 10: Parliament Street & Oak Street  
 2032 Future Total AM Model  
 11-07-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	70	105	395	10	5	480
Future Volume (Veh/h)	70	105	395	10	5	480
Sign Control	Stop	Stop	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	74	111	416	11	5	505
Pedestrians	260	260	5	5	175	175
Lane Width (m)	3.0	3.0	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	20	20	0	0	15	15
Right turn flare (veh)						
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)			151			143
pX platoon unblocked	0.99	0.99			0.99	
VC, conflicting volume	949	648			687	
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	898	629			668	
IC, single (s)	6.8	7.0			4.1	
IC, 2 stage (s)						
IF (s)	3.5	3.3			2.2	
p0 queue free %	67	61			99	
CM capacity (veh/h)	223	281			742	
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volumes Total	185	277	150	173	337	
Volume Left	74	0	0	5	0	
Volume Right	111	0	11	0	0	
cSH	255	1700	1700	742	1700	
Volumes to Capacity	0.73	0.16	0.09	0.01	0.20	
Queue Length 95th (m)	38.3	0.0	0.0	0.2	0.0	
Control Delay (s)	49.3	0.0	0.0	0.4	0.0	
Lane LOS	E	C	A	A	A	
Approach Delay (s)	49.3	0.0	0.0	0.1	0.1	
Approach LOS	E	E	A	A	A	
Intersection Summary						
Average Delay			8.2			
Intersection Capacity Utilization			38.3%			A
Analysis Period (min)			15			



HCM Unsignalized Intersection Capacity Analysis  
 11: Oak Street & Dreamer's Way

HCM Unsignalized Intersection Capacity Analysis  
 12: Regent Street & Oak Street

2032 Future Total AM Model  
 11-07-2022

2032 Future Total AM Model  
 11-07-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Traffic Volume (veh/h)	0	15	125	0	10	55
Future Volume (Veh/h)	0	15	125	0	10	55
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82
Hourly flow rate (vph)	0	18	152	0	12	67
Pedestrians	15				40	
Lane Width (m)	3.5	3.5	3.0	3.0	3.0	3.0
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	1				3	
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked					210	207
VC conflicting volume	192					
VC1 stage 1 conf vol						
VC2 stage 2 conf vol	192				210	207
VCu unblocked vol	4.1				6.4	6.3
IC single (s)						
IC 2 stage (s)	2.2				3.5	3.4
p0 queue free %	100				98	91
CM capacity (veh/h)	1351				759	774
Direction_Lane #	EB 1	WB 1	SB 1			
Volumes Total	18	152	79			
Volume Left	0	0	12			
Volume Right	0	0	67			
cSH	1700	1700	772			
Volumes to Capacity	0.01	0.09	0.10			
Queue Length 95th (m)	0.0	0.0	2.6			
Control Delay (s)	0.0	0.0	10.2			
Lane LOS			B			
Approach Delay (s)	0.0	0.0	10.2			
Approach LOS			B			
Intersection Summary						
Average Delay			3.2			
Intersection Capacity Utilization			29.1%		ICU Level of Service	A
Analysis Period (min)			15			

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑		↑	↑	↑
Traffic Volume (veh/h)	25	0	0	95	30	30
Future Volume (Veh/h)	25	0	0	95	30	30
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.78	0.78	0.78	0.78	0.78	0.78
Hourly flow rate (vph)	32	0	0	122	38	38
Pedestrians	15			10	30	
Lane Width (m)	3.5	3.5	3.0	3.0	3.0	3.0
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	1			1	2	
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked				62	199	72
VC conflicting volume						
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol				62	199	72
IC single (s)				4.1	6.4	6.2
IC 2 stage (s)				2.2	3.5	3.3
p0 queue free %	100			100	95	96
CM capacity (veh/h)	1519			757	954	
Direction_Lane #	EB 1	WB 1	NB 1			
Volumes Total	32	122	76			
Volume Left	0	0	38			
Volume Right	0	0	38			
cSH	1700	1700	844			
Volumes to Capacity	0.02	0.07	0.09			
Queue Length 95th (m)	0.0	0.0	2.2			
Control Delay (s)	0.0	0.0	9.7			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	9.7			
Approach LOS			A			
Intersection Summary						
Average Delay			3.2			
Intersection Capacity Utilization			29.1%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis  
 14: Sackville Street & Oak Street

HCM Unsignalized Intersection Capacity Analysis  
 15: Sumach Street & Oak Street

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop			Stop			Stop			Stop		
Traffic Volume (vph)	0	40	15	0	20	0	0	0	0	55	165	70
Future Volume (vph)	0	40	15	0	20	0	0	0	0	55	165	70
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Hourly flow rate (vph)	0	49	19	0	25	0	0	0	0	68	204	86
Direction, Lane #	EB 1	WB 1	SB 1									
Volume Total (vph)	68	25	358									
Volume Left (vph)	0	0	68									
Volume Right (vph)	19	0	86									
Head (s)	-0.06	0.00	-0.01									
Departure Headway (s)	4.7	4.8	4.1									
Degree Utilization, x	0.09	0.03	0.41									
Capacity (veh/h)	709	687	855									
Control Delay (s)	8.1	8.0	10.0									
Approach Delay (s)	8.1	8.0	10.0									
Approach LOS	A	A	A									
Intersection Summary												
Delay	9.6											
Level of Service	A											
Intersection Capacity Utilization	41.4%			ICU Level of Service			A					
Analysis Period (min)	15											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop			Stop			Stop			Stop		
Traffic Volume (vph)	35	35	25	0	0	0	20	105	35	0	0	0
Future Volume (vph)	35	35	25	0	0	0	20	105	35	0	0	0
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Hourly flow rate (vph)	47	47	33	0	0	0	27	140	47	0	0	0
Direction, Lane #	EB 1	NB 1										
Volume Total (vph)	127	214										
Volume Left (vph)	47	27										
Volume Right (vph)	33	47										
Head (s)	0.00	-0.03										
Departure Headway (s)	4.4	4.2										
Degree Utilization, x	0.15	0.25										
Capacity (veh/h)	786	836										
Control Delay (s)	8.2	8.5										
Approach Delay (s)	8.2	8.5										
Approach LOS	A	A										
Intersection Summary												
Delay	8.4											
Level of Service	A											
Intersection Capacity Utilization	35.5%			ICU Level of Service			A					
Analysis Period (min)	15											

16: Tubman Avenue/Site Laneway & Oak Street  
 HCM Unsignalized Intersection Capacity Analysis  
 2032 Future Total AM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	70	0	0	0	0	0	0	0	0	0	10
Future Volume (Veh/h)	0	70	0	0	0	0	0	0	0	0	0	10
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75	0.75
Hourly flow rate (vph)	0	93	0	0	0	0	0	0	0	0	0	13
Pedestrians	40	40	0	0	0	0	0	0	0	0	0	0
Lane Width (m)	3.5	3.5	0.0	0.0	0.0	0.0	1.1	1.1	1.1	1.1	1.1	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	4	4	0	0	0	0	0	0	0	0	0	0
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None	None	None
Median type	None	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)	None	None	None	None	None	None	None	None	None	None	None	None
Upstream signal (m)	None	None	None	None	None	None	None	None	None	None	None	None
pX platoon unblocked	0	138	0	0	0	0	184	138	148	103	138	40
VC, conflicting volume	0	138	0	0	0	0	184	138	148	103	138	40
VC1, stage 1 conf vol	0	138	0	0	0	0	184	138	148	103	138	40
VC2, stage 2 conf vol	4.1	4.1	0	0	0	0	7.1	6.5	6.7	7.1	6.5	6.2
VCu, unblocked vol	0	138	0	0	0	0	184	138	148	103	138	40
IC, single (s)	2.2	2.2	0	0	0	0	3.5	4.0	3.8	3.5	4.0	3.3
IC, 2 stage (s)	100	100	0	0	0	0	100	100	100	100	98	100
p0 queue free %	1636	1458	0	0	0	0	743	757	786	882	757	1000
CM capacity (veh/h)	93	13	0	0	0	0	0	0	0	0	0	0
Direction, Lane #	EB 1	SB 1										
Volumes Total	93	13	0	0	0	0	0	0	0	0	0	0
Volume Left	0	0	0	0	0	0	0	0	0	0	0	0
Volume Right	0	0	0	0	0	0	0	0	0	0	0	0
ESH	1700	757	0	0	0	0	0	0	0	0	0	0
Volumes to Capacity	0.05	0.02	0	0	0	0	0	0	0	0	0	0
Queue Length 95th (m)	0.0	0.4	0	0	0	0	0	0	0	0	0	0
Control Delay (s)	0.0	9.8	0	0	0	0	0	0	0	0	0	0
Lane LOS	A	A										
Approach Delay (s)	0.0	9.8	0	0	0	0	0	0	0	0	0	0
Approach LOS	A	A										
Intersection Summary												
Average Delay	1.2											
Intersection Capacity Utilization	28.5%											
ICU Level of Service	A											
Analysis Period (min)	15											

18: River Street & Oak Street  
 HCM Unsignalized Intersection Capacity Analysis  
 2032 Future Total AM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	40	0	30	35	0	65	0	375	25	40	630	0
Future Volume (Veh/h)	40	0	30	35	0	65	0	375	25	40	630	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	44	0	33	38	0	71	0	412	27	44	692	0
Pedestrians	75	75	0	65	65	35	0	35	0	5	5	0
Lane Width (m)	3.5	3.5	0.0	3.5	3.5	3.5	1.1	1.1	1.1	1.1	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	7	7	0	6	6	3	0	0	0	0	0	0
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None	None	None
Median type	None	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)	None	None	None	None	None	None	None	None	None	None	None	None
Upstream signal (m)	None	None	None	None	None	None	None	None	None	None	None	None
pX platoon unblocked	0.86	0.86	0.85	0.86	0.86	0.79	0.85	0.85	0.79	0.79	0.79	151
VC, conflicting volume	1356	1359	802	1338	1346	496	767	767	504	504	504	504
VC1, stage 1 conf vol	951	954	682	930	938	228	641	641	239	239	239	239
VC2, stage 2 conf vol	7.2	6.5	6.2	7.1	7.0	6.2	4.1	4.1	4.1	4.1	4.1	4.1
VCu, unblocked vol	951	954	682	930	938	228	641	641	239	239	239	239
IC, single (s)	3.6	4.0	3.3	3.5	4.5	3.3	2.2	2.2	2.2	2.2	2.2	2.2
IC, 2 stage (s)	69	100	91	75	100	88	100	100	88	100	96	96
p0 queue free %	144	189	350	153	160	601	759	759	997	997	997	997
CM capacity (veh/h)	77	109	439	736	736	0	0	0	0	0	0	0
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volumes Total	77	109	439	736	736	0	0	0	0	0	0	0
Volume Left	44	38	0	44	44	0	0	0	0	0	0	0
Volume Right	33	71	27	0	0	0	0	0	0	0	0	0
ESH	193	298	1700	997	997	0	0	0	0	0	0	0
Volumes to Capacity	0.40	0.37	0.26	0.04	0.04	0	0	0	0	0	0	0
Queue Length 95th (m)	13.5	12.3	0.0	1.1	1.1	0	0	0	0	0	0	0
Control Delay (s)	35.6	23.9	0.0	1.1	1.1	0	0	0	0	0	0	0
Lane LOS	E	C	C	A	A							
Approach Delay (s)	35.6	23.9	0.0	1.1	1.1	0	0	0	0	0	0	0
Approach LOS	E	C	C	A	A							
Intersection Summary												
Average Delay	4.5											
Intersection Capacity Utilization	78.6%											
ICU Level of Service	D											
Analysis Period (min)	15											

19: Parliament Street & Cole Street  
 HCM Unsignalized Intersection Capacity Analysis  
 2032 Future Total AM Model  
 11-07-2022

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			←	←	←	←
Traffic Volume (veh/h)	0	0	405	20	25	525
Future Volume (Veh/h)	0	0	405	20	25	525
Sign Control	Stop	Stop	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	440	22	27	571
Pedestrians	250	10				15
Lane Width (m)	0.0	3.5	3.5			3.5
Walking Speed (m/s)	1.1	1.1	1.1			1.1
Percent Blockage	0	1				1
Right turn flare (veh)			None			None
Median storage (veh)						
Upstream signal (m)	0.94	0.94	80			214
pX platoon unblocked	1050	496				712
VC, conflicting volume						
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	924	334				564
IC, single (s)	6.8	6.9				4.2
IC, 2 stage (s)	3.5	3.3				2.2
p0 queue free %	100	100				97
cM capacity (veh/h)	246	619				930
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	293	169	217	381		
Volume Left	0	0	27	0		
Volume Right	0	22	0	0		
cSH	1700	1700	930	1700		
Volumes to Capacity	0.17	0.10	0.03	0.22		
Queue Length 95th (m)	0.0	0.0	0.7	0.0		
Control Delay (s)	0.0	0.0	1.4	0.0		
Lane LOS			A			
Approach Delay (s)	0.0		0.5			
Approach LOS						
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization			45.8%			
ICU Level of Service			A			
Analysis Period (min)			15			

20: Parliament Street & Dundas Street E  
 Timings  
 2032 Future Total AM Model  
 11-07-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	←	←	←	←	←	←	←	←
Traffic Volume (vph)	30	310	60	330	40	310	65	400
Future Volume (vph)	30	310	60	330	40	310	65	400
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2		6		8		8	4
Permitted Phases	2	2	6	6	8	8	4	4
Detector Phase								
Switch Phase								
Minimum Initial (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Minimum Split (s)	29.0	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	59.2%	59.2%	59.2%	59.2%	40.8%	40.8%	40.8%	40.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)								
LeadLag	5.0		5.0		5.0		5.0	5.0
Lead-Lag Optimize?								
Recall Mode								
Act Effct Green (s)	41.0	41.0	C-Min	C-Min	Min	Min	Min	Min
Actuated g/C Ratio	0.54	0.54	0.54	0.54	0.33	0.33	0.33	0.33
v/C Ratio	0.25	0.25	0.35	0.35	0.45	0.45	0.61	0.61
Control Delay	9.8	12.7	21.3	21.3	23.6	23.6	23.6	23.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.8	12.7	21.3	21.3	23.6	23.6	23.6	23.6
LOS	A	B	B	B	C	C	C	C
Approach Delay	9.8		12.7		21.3		23.6	
Approach LOS	A		B		C		C	
Intersection Summary								
Cycle Length: 76								
Actuated Cycle Length: 76								
Offset: 54 (7%), Referenced to phase 2EBTL and 6:WBT, Start of 1st Green								
Natural Cycle: 60								
Control Type: Actuated-Coordinated								
Maximum v/C Ratio: 0.61								
Intersection Signal Delay: 17.2								
Intersection Capacity Utilization 93.3%								
Analysis Period (min) 15								



20: Parliament Street & Dundas Street E



	EBT	WBT	NBT	SBT
Lane Group	389	505	389	559
Lane Group Flow (vph)	0.25	0.35	0.45	0.61
v/c Ratio	9.8	12.7	21.3	23.6
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	9.8	12.7	21.3	23.6
Total Delay	13.4	27.2	22.9	34.4
Queue Length 50th (m)	23.7	38.2	31.8	45.8
Queue Length 95th (m)	85.0	103.5	45.0	56.1
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1553	1454	922	970
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.25	0.35	0.42	0.58
<b>Intersection Summary</b>				



	EBT	WBT	NBT	SBT
Lane Configurations	4TB	4TB	4TB	4TB
Traffic Volume (vph)	30	310	25	330
Future Volume (vph)	30	310	25	330
Ideal Flow (vphpl)	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.5
Total Lost time (s)	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	0.95	0.95	0.95
Frbp. ped/bikes	0.99	0.97	0.99	0.98
Frbp. psd/bikes	0.99	0.99	0.99	0.99
Frt	0.99	0.97	0.99	0.98
Flt Protected	1.00	0.99	0.99	0.99
Satd. Flow (prot)	3173	3058	3113	3239
Flt Permitted	0.89	0.85	0.84	0.84
Satd. Flow (perm)	2836	2627	2638	2749
Peak-hour factor, PHF	0.94	0.94	0.94	0.94
Adj. Flow (vph)	32	330	27	351
RTOR Reduction (vph)	0	7	0	23
Lane Group Flow (vph)	0	382	0	482
Confl. Peds. (#/hr)	155	110	110	140
Confl. Bikes (#/hr)	10	25	25	160
Heavy Vehicles (%)	6%	10%	4%	22%
Parking (#/hr)	0	0	0	0
Turn Type	Perm	NA	NA	Perm
Protected Phases	2	6	6	8
Permitted Phases	2	6	6	8
Actuated Green, G (s)	40.0	40.0	24.0	24.0
Effective Green, g (s)	41.0	41.0	25.0	25.0
Actuated g/C Ratio	0.54	0.54	0.33	0.33
Clearance Time (s)	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1529	1417	867	904
v/s Ratio Prot				
v/s Ratio Perm	0.13	0.18	0.15	0.20
v/c Ratio	0.25	0.34	0.44	0.60
Uniform Delay, d1	9.3	9.9	20.0	21.4
Progression Factor	1.00	1.29	1.00	1.00
Incremental Delay, d2	0.4	0.6	0.4	1.1
Delay (s)	9.7	13.4	20.4	22.5
Level of Service	A	B	C	C
Approach Delay (s)	9.7	13.4	20.4	22.5
Approach LOS	A	B	C	C
<b>Intersection Summary</b>				
HCM 2000 Control Delay	16.9	HCM 2000 Level of Service B		
HCM 2000 Volume to Capacity ratio	0.44			
Actuated Cycle Length (s)	76.0	Sum of lost time (s) 10.0		
Intersection Capacity Utilization	93.3%	ICU Level of Service F		
Analysis Period (min)	15			
c Critical Lane Group				

21: Regent Street & Dundas Street E

2032 Future Total AM Model  
11-07-2022

22: Sackville Street & Dundas Street E

2032 Future Total AM Model  
11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	4T	4T	4T	4T	4T	4T	4T	4T	4T	4T	4T
Traffic Volume (veh/h)	15	380	10	10	450	35	15	10	5	20	10
Future Volume (Veh/h)	15	380	10	10	450	35	15	10	5	20	10
Sign Control	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	413	11	11	489	38	16	11	5	22	11
Pedestrians	20			5			70			80	
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	2			0			6			7	
Right turn flare (veh)											
Median type	None	None	None	None	None	None	None	None	None	None	None
Median storage (veh)											
Upstream signal (m)	127			1.00	1.24		1.00	1.00	1.00	1.00	1.00
pX platoon unblocked	607			494			834	1150	287	864	1136
VC, conflicting volume											
VC1, stage 1 conf vol											
VC2, stage 2 conf vol											
VCU, unblocked vol	607			484			826	1142	276	865	1128
IC, single (s)	4.1			4.1			7.6	6.7	6.9	7.6	6.5
IC, 2 stage (s)											
p0 queue free %	2.2			2.2			3.6	4.1	3.3	3.5	4.0
p0 queue free (s)	98			99			92	93	99	88	94
CM capacity (veh/h)	912			1018			190	157	677	189	174
Direction, Lane #	EB1	EB2	WB1	WB2	NB1	SB1					
Volume Total	222	218	256	282	32	55					
Volume Left	16	0	11	0	16	22					
Volume Right	0	11	0	38	5	22					
cSH	912	1700	1018	1700	198	251					
Volumes to Capacity	0.02	0.13	0.01	0.17	0.16	0.22					
Queue Length 95th (m)	0.4	0.0	0.2	0.0	4.3	6.2					
Control Delay (s)	0.8	0.0	0.5	0.0	26.7	23.3					
Lane LOS	A	A	A	D	C	C					
Approach Delay (s)	0.4		0.2	26.7	23.3						
Approach LOS				D	C						
Intersection Summary											
Average Delay	2.3										
Intersection Capacity Utilization	37.2%										
Analysis Period (min)	15										
	ICU Level of Service A										

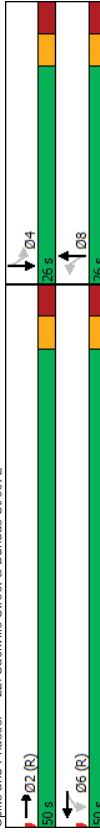
21: Regent Street & Dundas Street E

2032 Future Total AM Model  
11-07-2022

22: Sackville Street & Dundas Street E

2032 Future Total AM Model  
11-07-2022

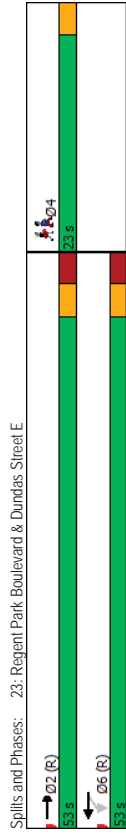
Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4T	4T	4T	4T	4T	4T	4T
Traffic Volume (vph)	415	10	400	45	0	100	50
Future Volume (vph)	415	10	400	45	0	100	50
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2		6		8		4
Permitted Phases	2	6	6	8	8	4	4
Detector Phase							
Switch Phase							
Minimum Initial (s)	17.0	17.0	17.0	7.0	7.0	7.0	7.0
Minimum Split (s)	24.0	24.0	24.0	26.0	26.0	26.0	26.0
Total Split (s)	50.0	50.0	50.0	26.0	26.0	26.0	26.0
Total Split (%)	65.8%	65.8%	65.8%	34.2%	34.2%	34.2%	34.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag							
Lead-Lag Optimize?							
Recall Mode	C-Min	C-Min	C-Min	None	None	None	None
Act Effct Green (s)	46.5	46.5	46.5	19.5	19.5	19.5	19.5
Actuated g/C Ratio	0.61	0.61	0.61	0.26	0.26	0.26	0.26
v/C Ratio	0.23	0.41	0.41	0.22	0.22	0.22	0.22
Control Delay	7.4	6.6	6.6	12.1	12.1	12.1	12.1
Queue Delay	0.0	0.3	0.3	0.0	0.0	0.0	0.0
Total Delay	7.4	6.9	6.9	12.1	12.1	12.1	12.1
LOS	A	A	A	B	B	B	C
Approach Delay	7.4	6.9	6.9	12.1	12.1	12.1	12.1
Approach LOS	A	A	A	B	B	B	C
Intersection Summary							
Cycle Length: 76							
Actuated Cycle Length: 76							
Offset: T1 (14%), Referenced to phase 2EBT and 6,WBTL Start of 1st Green							
Natural Cycle: 50							
Control Type: Actuated-Coordinated							
Maximum v/C Ratio: 0.62							
Intersection Signal Delay: 11.5							
Intersection Capacity Utilization 53.8%							
Analysis Period (min) 15							
	ICU Level of Service A						



	EBT	WBT	NBT	SBT
Lane Group	463	432	73	216
Lane Group Flow (vph)	0.23	0.41	0.22	0.62
v/c Ratio	7.4	6.6	12.1	29.1
Control Delay	0.0	0.3	0.0	0.0
Queue Delay	7.4	6.9	12.1	29.1
Total Delay	9.2	30.5	3.2	23.5
Queue Length 50th (m)	33.4	11.3	11.7	41.5
Queue Length 95th (m)	99.8	79.6	36.2	126.8
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2007	1066	372	388
Station Cap Reductn	0	212	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.23	0.51	0.20	0.56
<b>Intersection Summary</b>				

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4+1			4			4+1					
Traffic Volume (vph)	0	415	25	10	400	0	45	0	25	100	50	55	
Future Volume (vph)	0	415	25	10	400	0	45	0	25	100	50	55	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.0	3.0	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Lane Util. Factor	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frbp. ped/bikes	0.99	0.99	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.98	0.98	0.98	
Frbp. psd/bikes	1.00	1.00	1.00	1.00	1.00	0.97	0.97	0.97	0.97	0.95	0.95	0.95	
Frt	0.99	0.99	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.98	0.98	0.98	
Flt Protected	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Satd. Flow (prot)	3230	3230	1736	1736	1568	1568	1568	1568	1568	1571	1571	1571	
Flt Permitted	1.00	1.00	0.99	0.99	0.75	0.75	0.75	0.75	0.75	0.81	0.81	0.81	
Satd. Flow (perm)	3230	3230	1716	1716	1200	1200	1200	1200	1200	1303	1303	1303	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Adj. Flow (vph)	0	437	26	11	421	0	47	0	26	105	53	58	
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Group Flow (vph)	0	463	0	0	432	0	41	0	41	0	198	0	
Confl. Peds. (#/hr)	65	85	85	65	65	65	65	65	60	60	60	65	
Confl. Bikes (#/hr)	10	10	10	25	25	25	25	25	25	25	25	25	
Heavy Vehicles (%)	0%	8%	13%	0%	8%	0%	0%	0%	8%	6%	2%	4%	
Turn Type	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	
Protected Phases	2			6			8				4		
Permitted Phases		6					8			4			
Actuated Green, G (s)	45.5	45.5	45.5	45.5	45.5	45.5	18.5	18.5	18.5	18.5	18.5	18.5	
Effective Green, g (s)	46.5	46.5	46.5	46.5	46.5	46.5	19.5	19.5	19.5	19.5	19.5	19.5	
Actuated g/C Ratio	0.61	0.61	0.61	0.61	0.61	0.61	0.26	0.26	0.26	0.26	0.26	0.26	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1976	1976	1049	1049	307	307	307	307	307	307	307	307	
v/s Ratio Prot	0.14												
v/s Ratio Perm	0.23			0.25			0.13						
v/c Ratio	6.7	7.7	7.7	21.7	21.7	21.7	24.8	24.8	24.8	24.8	24.8	24.8	
Uniform Delay, d1	0.95	0.61	0.61	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Progression Factor	0.3	1.2	1.2	0.2	0.2	0.2	2.8	2.8	2.8	2.8	2.8	2.8	
Incremental Delay, d2	6.6	5.8	5.8	21.9	21.9	21.9	27.6	27.6	27.6	27.6	27.6	27.6	
Delay (s)	A	A	A	C	C	C	C	C	C	C	C	C	
Level of Service	A	A	A	A	A	A	A	A	A	A	A	A	
Approach Delay (s)	6.6	5.8	5.8	21.9	21.9	21.9	27.6	27.6	27.6	27.6	27.6	27.6	
Approach LOS	A	A	A	A	A	A	A	A	A	A	A	A	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	11.1											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.47												
Actuated Cycle Length (s)	76.0											Sum of lost time (s)	10.0
Intersection Capacity Utilization	53.8%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

Lane Group	EBT	WBL	WBT	Ø4
Lane Configurations	1	1	1	4
Traffic Volume (vph)	520	15	410	4
Future Volume (vph)	520	15	410	4
Turn Type	NA	Perm	NA	NA
Protected Phases	2	6	6	4
Permitted Phases	2	6	6	6
Detector Phase				
Switch Phase				
Minimum Initial (s)	18.0	18.0	18.0	20.0
Minimum Split (s)	23.9	23.9	23.9	23.0
Total Split (s)	53.0	53.0	53.0	23.0
Total Split (%)	69.7%	69.7%	69.7%	30%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.9	4.9	4.9	
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	C-Min	C-Min	C-Min	None
Act Effct Green (s)	64.8	64.8	64.8	
Actuated g/C Ratio	0.85	0.85	0.85	
v/c Ratio	0.38	0.31	0.31	
Control Delay	4.3	4.3	4.6	
Queue Delay	0.0	0.0	0.0	
Total Delay	4.3	4.3	4.6	
LOS	A	A	A	
Approach Delay	4.3	4.3	4.6	
Approach LOS	A	A	A	
<b>Intersection Summary</b>				
Cycle Length: 76				
Actuated Cycle Length: 76				
Offset: 8 (1%) Referenced to phase 2:EBT and 6:WBL, Start of 1st Green				
Natural Cycle: 55				
Control Type: Actuated-Coordinated				
Maximum v/c Ratio: 0.38				
Intersection Signal Delay: 4.5	Intersection LOS: A			
Intersection Capacity Utilization 53.1%	ICU Level of Service A			
Analysis Period (min) 15				



Lane Group	EBT	WBT
Lane Group Flow (vph)	563	443
v/c Ratio	0.38	0.31
Control Delay	4.3	4.6
Queue Delay	0.0	0.0
Total Delay	4.3	4.6
Queue Length 50th (m)	9.8	0.0
Queue Length 95th (m)	31.4	44.5
Internal Link Dist (m)	79.6	86.0
Turn Bay Length (m)		
Base Capacity (vph)	1466	1446
Starvation Cap Reductn	19	115
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.39	0.33
<b>Intersection Summary</b>		



23: Regent Park Boulevard & Dundas Street E

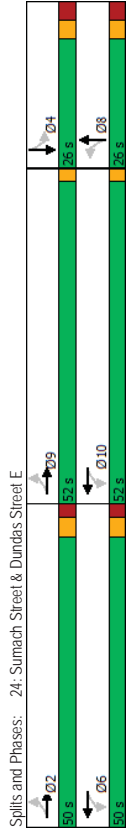
2032 Future Total AM Model  
11-07-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	EB	EB	WB	WB	NB	NB
Traffic Volume (vph)	520	20	15	410	0	0
Future Volume (vph)	520	20	15	410	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.0	3.0	3.5	3.0	3.0
Total Lost time (s)	4.9		4.9			
Lane Util. Factor	1.00		1.00			
Frbp. ped/bikes	0.99		1.00			
Frbp. psd/bikes	1.00		1.00			
Frt	0.99		1.00			
Flt Protected	1.00		1.00			
Sat'd Flow (prot)	1718		1729			
Flt Permitted	1.00		0.98			
Sat'd Flow (perm)	1718		1694			
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	542	21	16	427	0	0
RTOR Reduction (vph)	1	0	0	0	0	0
Lane Group Flow (vph)	562	0	0	443	0	0
Conf. Peds. (#/hr)	125	125	40	60		
Heavy Vehicles (%)	8%	0%	9%	8%	0%	0%
Turn Types	NA	Perm	NA			
Protected Phases	2		6			
Permitted Phases	6		6			
Actuated Green, G (s)	59.1		59.1			
Effective Green, g (s)	60.1		60.1			
Actuated g/C Ratio	0.79		0.79			
Clearance Time (s)	5.9		5.9			
Vehicle Extension (s)	3.0		3.0			
Lane Grp Cap. (vph)	1358		1339			
v/s Ratio Prot	0.33		0.26			
v/s Ratio Perm	0.41		0.33			
Uniform Delay, d1	2.5		2.3			
Progression Factor	0.81		1.00			
Incremental Delay, d2	0.9		0.7			
Delay (s)	2.9		2.9			
Level of Service	A		A			
Approach Delay (s)	2.9		2.9	0.0		
Approach LOS	A		A	A		
Intersection Summary						
HCM 2000 Control Delay	2.9		HCM 2000 Level of Service		A	
HCM 2000 Volume to Capacity ratio	0.37					
Actuated Cycle Length (s)	76.0		Sum of lost time (s)		7.9	
Intersection Capacity Utilization	53.1%		ICU Level of Service		A	
Analysis Period (min)	15					
c. Critical Lane Group						

24: Sumach Street & Dundas Street E

2032 Future Total AM Model  
11-07-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	EB	EB	WB	WB	NB	NB	SB	SB
Traffic Volume (vph)	15	460	30	400	15	30	55	0
Future Volume (vph)	15	460	30	400	15	30	55	0
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2,9		6,10		8		4	
Detector Phase	2		6		8		4	
Switch Phase	2		6		8		4	
Minimum Initial (s)	20.0		20.0		20.0		20.0	
Minimum Split (s)	26.0		26.0		26.0		26.0	
Total Split (s)	26.0		26.0		26.0		26.0	
Total Split (%)	20.3%		20.3%		20.3%		20.3%	
Yellow Time (s)	3.0		3.0		3.0		3.0	
All-Red Time (s)	3.0		3.0		3.0		3.0	
Lost Time Adjust (s)	-1.0		-1.0		-1.0		-1.0	
Total Lost Time (s)	5.0		5.0		5.0		5.0	
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode	None		None		None		None	
Act Effct Green (s)	74.4		74.4		25.7		25.7	
Actuated g/C Ratio	0.78		0.78		0.27		0.27	
v/C Ratio	0.45		0.48		0.28		0.24	
Control Delay	7.0		7.2		29.4		26.6	
Queue Delay	1.6		0.4		0.0		0.0	
Total Delay	8.6		7.6		29.4		26.6	
LOS	A		A		C		C	
Approach Delay	8.6		7.6		29.4		26.6	
Approach LOS	A		A		C		C	
Intersection Summary								
Cycle Length: 128								
Actuated Cycle Length: 95.2								
Natural Cycle: 100								
Control Type: Actuated-Uncoordinated								
Maximum v/c Ratio: 0.48								
Intersection Signal Delay: 10.9								
Intersection Capacity Utilization: 65.8%								
Analysis Period (min): 15								



Queues  
24: Sumach Street & Dundas Street E

HCM Signalized Intersection Capacity Analysis  
24: Sumach Street & Dundas Street E

2032 Future Total AM Model  
11-07-2022

	EBT	WBT	NBT	SBT
Lane Group	565	571	109	71
Lane Group Flow (vph)	0.45	0.48	0.28	0.24
v/c Ratio	7.0	7.2	29.4	26.6
Control Delay	1.6	0.4	0.0	0.0
Queue Delay	8.6	7.6	29.4	26.6
Total Delay	43.2	43.8	12.1	6.5
Queue Length 50th (m)	62.0	64.3	31.5	21.4
Queue Length 95th (m)	86.0	75.5	76.2	121.5
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	1260	1196	384	290
Station Cap Reductn	498	228	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.74	0.59	0.28	0.24
<b>Intersection Summary</b>				

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4	4	4	4	4	4	4	4	4	4	4	
Traffic Volume (vph)	15	460	45	30	400	95	15	30	55	55	0	10	
Future Volume (vph)	15	460	45	30	400	95	15	30	55	55	0	10	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frbp. ped/bikes	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	
Frbp. ped/bikes	1.00	1.00	0.99	0.99	0.99	0.96	0.96	0.96	0.96	0.96	0.92	0.92	
Frt	0.99	0.99	0.98	0.98	0.98	0.93	0.93	0.93	0.93	0.93	0.98	0.98	
Flt Protected	1.00	1.00	1.00	1.00	1.00	0.99	0.99	0.99	0.99	0.99	0.96	0.96	
Satd. Flow (prot)	1654	1654	1617	1617	1617	1402	1402	1402	1402	1402	1527	1527	
Flt Permitted	0.98	0.98	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.64	0.64	
Satd. Flow (perm)	1626	1626	1545	1545	1545	1359	1359	1359	1359	1359	1023	1023	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	16	500	49	33	435	103	16	33	60	60	0	11	
RTOR Reduction (vph)	0	2	0	0	6	0	0	35	0	0	29	0	
Lane Group Flow (vph)	0	563	0	0	565	0	0	74	0	0	42	0	
Confl. Peds. (#/hr)	70	135	135	70	120	70	120	50	50	50	120	120	
Confl. Bikes (#/hr)			5			30							
Heavy Vehicles (%)	13%	7%	14%	9%	8%	3%	11%	3%	10%	0%	3%	10%	
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	NA	
Protected Phases	2.9	6.10	6.10	6.10	6.10	8	8	8	8	4	4	4	
Permitted Phases	2.9	6.10	6.10	6.10	6.10	8	8	8	8	4	4	4	
Actuated Green, G (s)	74.7	74.7	74.7	74.7	74.7	13.9	13.9	13.9	13.9	13.9	13.9	13.9	
Effective Green, g (s)	75.7	75.7	75.7	75.7	75.7	14.9	14.9	14.9	14.9	14.9	14.9	14.9	
Actuated q/C Ratio	0.78	0.78	0.78	0.78	0.78	0.15	0.15	0.15	0.15	0.15	0.15	0.15	
Clearance Time (s)						6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)						3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1274	1274	1210	1210	1210	209	209	209	209	157	157	157	
v/s Ratio Prot													
v/s Ratio Perm	0.35	0.37	0.37	0.37	0.37	0.05	0.05	0.05	0.05	0.04	0.04	0.04	
v/c Ratio	0.44	0.47	0.47	0.47	0.47	0.36	0.36	0.36	0.36	0.27	0.27	0.27	
Uniform Delay, d1	3.5	3.6	3.6	3.6	3.6	36.6	36.6	36.6	36.6	36.0	36.0	36.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.2	0.3	0.3	0.3	0.3	1.0	1.0	1.0	1.0	0.9	0.9	0.9	
Delay (s)	3.7	3.9	3.9	3.9	3.9	37.6	37.6	37.6	37.6	37.0	37.0	37.0	
Level of Service	A	A	A	A	A	D	D	D	D	D	D	D	
Approach Delay (s)	3.7	3.9	3.9	3.9	3.9	37.6	37.6	37.6	37.6	37.0	37.0	37.0	
Approach LOS	A	A	A	A	A	D	D	D	D	D	D	D	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	8.4											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.46												
Actuated Cycle Length (s)	96.6											Sum of lost time (s)	10.0
Intersection Capacity Utilization	66.8%											ICU Level of Service	C
Analysis Period (min)	15												
c Critical Lane Group													

25: Tubman Avenue & Dundas Street E

26: River Street & Dundas Street E

2032 Future Total AM Model  
11-07-2022

2032 Future Total AM Model  
11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	0	555	15	5	525	0	0	0	0	10	0	0
Traffic Volume (veh/h)	0	555	15	5	525	0	0	0	0	10	0	0
Future Volume (Veh/h)	0	555	15	5	525	0	0	0	0	10	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	617	17	6	583	0	0	0	0	11	0	0
Pedestrians	5			5			115			45		
Lane Width (m)	3.5			3.5			0.0			3.5		
Walking Speed (m/s)	1.1			1.1			1.1			1.1		
Percent Blockage	0			0			0			4		
Right turn flare (veh)	None			None								
Median type	None			None								
Median storage (veh)	99			98								
Upstream signal (m)	0.83			0.90			0.88			0.88		
pX platoon unblocked	628			749			1340			1270		
VC, conflicting volume												
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCu, unblocked vol	447			670			1072			666		
IC, single (s)	4.1			4.1			7.1			6.2		
IC, 2 stage (s)	2.2			2.2			3.5			4.0		
p0 queue free %	100			99			100			100		
CM capacity (veh/h)	894			841			168			174		
Direction, Lane #	EB 1	WB 1	SB 1									
Volume Total	634	589	11									
Volume Left	0	6	11									
Volume Right	17	0	0									
cSH	894	841	183									
Volume to Capacity	0.00	0.01	0.06									
Queue Length 95th (m)	0.0	0.2	1.4									
Control Delay (s)	0.0	0.2	25.9									
Lane LOS	A	A	D									
Approach Delay (s)	0.0	0.2	25.9									
Approach LOS	D	D	D									
<b>Intersection Summary</b>												
Average Delay	0.3											
Intersection Capacity Utilization	46.4%											
Analysis Period (min)	15											
	ICU Level of Service A											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	15	490	110	325	75	325	155	445				
Traffic Volume (vph)	15	490	110	325	75	325	155	445				
Future Volume (vph)	15	490	110	325	75	325	155	445				
Sign Control	Perm	MA	custom	MA	Perm	MA	pm+pt	NA				
Turn Type	2.9	1	1.6	10	8	8	7	4				
Protected Phases	2	2	1	1.6	8	8	7	4				
Permitted Phases	2	2	1	1.6	8	8	7	4				
Detector Phase												
Switch Phase												
Minimum Initial (s)	6.0			21.0			5.0	21.0		21.0		5.0
Minimum Split (s)	10.0			27.0			9.0	27.0		27.0		23.0
Total Split (s)	30.0			30.0			30.0	40.0		30.0		23.0
Total Split (%)	9.7%			29.1%			9.7%	38.8%		29%		22%
Yellow Time (s)	3.0			3.0			3.0	3.0		3.0		2.0
All-Red Time (s)	1.0			3.0			3.0	3.0		3.0		0.0
Lost Time Adjust (s)	-1.0			-1.0			-1.0	-1.0		-1.0		-1.0
Total Lost Time (s)				5.0			5.0	5.0		5.0		5.0
Lead/Lag	Lead	Lag	Lag	Lag	Lead	Lead	Lag	Lag		Lag		Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes		Yes
Recall Mode	None	None	None	None	None	None	None	None		None		None
Act Effct Green (s)	29.0			41.2			25.3	37.4		35.4		35.4
Actuated g/C Ratio	0.34			0.49			0.30	0.30		0.44		0.42
v/c Ratio	1.05			0.86			0.70	0.99		0.75		0.92
Control Delay	78.1			33.0			62.8	68.4		41.3		45.6
Queue Delay	0.6			0.0			0.0	0.0		0.0		0.0
Total Delay	78.7			33.0			62.8	68.4		41.3		45.6
LOS	E	C	E	C	E	D	E	D		D		D
Approach Delay	78.7			33.0			67.6	44.7		44.7		44.7
Approach LOS	E	C	E	C	E	D	E	D		D		D
<b>Intersection Summary</b>												
Cycle Length	103											
Actuated Cycle Length	84.6											
Natural Cycle	100											
Control Type	Actuated-Uncoordinated											
Maximum v/c Ratio	1.05											
Intersection Signal Delay	55.8											
Intersection Capacity Utilization	123.3%											
Analysis Period (min)	15											



Queues 26: River Street & Dundas Street E 2032 Future Total AM Model 11-07-2022

	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group	601	516	80	490	165	611
Lane Group Flow (vph)	1.05	0.86	0.70	0.99	0.75	0.92
v/c Ratio	78.1	33.0	62.8	68.4	41.3	45.6
Control Delay	0.6	0.0	0.0	0.0	0.0	0.0
Queue Delay	78.7	33.0	62.8	68.4	41.3	45.6
Total Delay	-108.4	56.3	10.5	69.6	14.4	80.2
Queue Length 50th (m)	141.5	82.9	#43.3	#179.1	#57.2	#206.7
Queue Length 95th (m)	73.4	80.5		131.0		118.9
Internal Link Dist (m)		20.0			25.0	
Turn Bay Length (m)						
Base Capacity (vph)	575	600	115	497	220	664
Station Cap Reductn	1	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.05	0.86	0.70	0.99	0.75	0.92

**Intersection Summary**  
 - Volume exceeds capacity, queue is theoretically infinite.  
 Queue shown is maximum after two cycles.  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 26: River Street & Dundas Street E 2032 Future Total AM Model 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	15	490	60	110	325	50	75	325	135	155	445	130
Future Volume (vph)	15	490	60	110	325	50	75	325	135	155	445	130
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0	5.0	3.0	3.0	5.0	5.0	5.0	5.0	3.0	3.0	5.0	5.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.98	0.98	0.99	0.99	0.99	0.99	0.96	0.96	1.00	0.97	1.00	0.96
Frbp. psd/bikes	1.00	1.00	0.99	0.99	0.99	0.99	0.96	0.96	1.00	0.96	1.00	0.97
Frt	1.00	1.00	0.99	0.99	0.99	0.99	0.95	0.95	1.00	0.95	1.00	0.97
Flt Protected	1.00	1.00	0.99	0.99	0.99	0.99	0.95	0.95	1.00	0.95	1.00	0.97
Satd. Flow (prot)	1696	1696	1708	1708	1708	1480	1630	1630	1601	1577	1601	1577
Flt Permitted	0.98	0.98	0.98	0.65	0.65	0.24	1.00	1.00	0.14	1.00	0.14	1.00
Satd. Flow (perm)	1667	1667	1127	1127	1127	382	1630	1630	239	1577	239	1577
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	16	521	64	117	346	53	80	346	144	165	473	138
RTOR Reduction (vph)	0	5	0	0	5	0	0	13	0	0	9	0
Lane Group Flow (vph)	0	596	0	0	511	0	80	477	0	165	602	0
Conf. Peds. (#/hr)	65	90	90	65	65	65	65	65	45	45	65	15
Conf. Bikes (#/hr)			5			30						
Heavy Vehicles (%)	0%	7%	4%	3%	7%	0%	9%	9%	1%	5%	12%	7%
Turn Type	Perm	NA	NA	custom	NA	NA	Perm	NA	NA	pm+pl	NA	NA
Protected Phases		2.9		1	1.6	1.0		8		7		4
Permitted Phases		2.9		6	1.0			8		4		4
Actuated Green, G (s)		33.6		43.7			24.2	24.2		34.3		34.3
Effective Green, g (s)		34.6		39.7			25.2	25.2		35.3		35.3
Actuated g/C Ratio		0.40		0.46			0.29	0.29		0.41		0.41
Clearance Time (s)							6.0	6.0		4.0		6.0
Vehicle Extension (s)							3.0	3.0		3.0		3.0
Lane Grp Cap (vph)		670		568			111	477		210		647
v/s Ratio Prot		c0.36		c0.34			0.21	0.29		0.06		c0.38
v/c Ratio Perm		0.89		0.90			0.72	1.00		0.79		0.93
Uniform Delay, d1		23.9		21.3			27.2	30.4		19.7		24.2
Progression Factor		1.00		1.00			1.00	1.00		1.00		1.00
Incremental Delay, d2		13.7		16.9			20.4	40.7		17.4		20.2
Delay (s)		37.7		38.2			47.7	71.0		37.1		44.3
Level of Service		D		D			D	E		D		D
Approach Delay (s)		37.7		38.2			67.8	67.8		42.8		42.8
Approach LOS		D		D			E	E		D		D
Intersection Summary												
HCM 2000 Control Delay		46.4		HCM 2000 Level of Service				D				
HCM 2000 Volume to Capacity ratio		1.00										
Actuated Cycle Length (s)		86.0		Sum of lost time (s)				17.0				
Intersection Capacity Utilization		123.3%		ICU Level of Service				H				
Analysis Period (min)		15										
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 27: Dreamers Way & Gerrard Street E

HCM Unsignalized Intersection Capacity Analysis  
 28: Dreamers Way & Site Driveway

11-07-2022

11-07-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4↑	4↑				
Traffic Volume (veh/h)	350	25	5	825	0	0
Future Volume (Veh/h)	350	25	5	825	0	0
Sign Control	Free	Free	Stop	Stop		
Grade	0%	0%	0%	0%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	389	28	6	917	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None					
Median type						
Median storage (veh)						
Upstream signal (m)	88			164		
pX platoon unblocked	0.97			0.89		0.97
VC, conflicting volume	417			874		208
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	348			494		134
IC, single (s)	4.1			6.8		6.9
IC, 2 stage (s)						
p0 queue free %	2.2			3.5		3.3
IF (s)	99			100		100
CM capacity (veh/h)	1190			454		873
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volumes Total	259	158	312	611		
Volume Left	0	0	6	0		
Volume Right	0	28	0	0		
CSH	1700	1700	1190	1700		
Volumes to Capacity	0.15	0.09	0.01	0.36		
Queue Length 95th (m)	0.0	0.0	0.1	0.0		
Control Delay (s)	0.0	0.0	0.2	0.0		
Lane LOS	A	A	A	A		
Approach Delay (s)	0.0		0.1			
Approach LOS			A			
Intersection Summary						
Average Delay			0.0			A
Intersection Capacity Utilization			29.6%			ICU Level of Service
Analysis Period (min)			15			

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	4↑					4↑
Traffic Volume (veh/h)	20	0	0	0	0	30
Future Volume (Veh/h)	20	0	0	0	0	30
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	22	0	0	0	0	33
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None					None
Median type						
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	33					0
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	33					0
IC, single (s)	6.4					4.1
IC, 2 stage (s)						
p0 queue free %	3.5					2.2
IF (s)	98					100
CM capacity (veh/h)	986					1636
Direction, Lane #	WB 1	SB 1				
Volumes Total	22	33				
Volume Left	22	0				
Volume Right	0	0				
CSH	986	1636				
Volumes to Capacity	0.02	0.00				
Queue Length 95th (m)	0.5	0.0				
Control Delay (s)	8.7	0.0				
Lane LOS	A	A				
Approach Delay (s)	8.7	0.0				
Approach LOS	A					
Intersection Summary						
Average Delay			3.5			A
Intersection Capacity Utilization			13.3%			ICU Level of Service
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis  
 29: Sumach Street & Site Driveway  
 11-07-2022

HCM Unsignalized Intersection Capacity Analysis  
 30: River Street & Site Driveway  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	35	0	0	0	0	60	0	115	25	0	0	0
Future Volume (Veh/h)	35	0	0	0	0	60	0	115	25	0	0	0
Sign Control	Stop											
Grade	0%											
Peak Hour Factor	0.90											
Hourly flow rate (vph)	39	0	0	0	0	67	0	128	28	0	0	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)	None											
Median type	None											
Median storage (veh)												
Upstream signal (m)	57											
pX platoon unblocked												
VC, conflicting volume	209	156	0	142	142	142	0		156			
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	209	156	0	142	142	142	0		156			
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1		4.1			
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2		2.2			
p0 queue free %	94	100	100	100	100	93	100		100			
CM capacity (veh/h)	697	740	1091	832	753	911	1636		1436			
Direction, Lane #	EB 1	WB 1	NB 1									
Volumes Total	39	67	156									
Volume Left	39	0	0									
Volume Right	0	67	28									
cSH	697	911	1636									
Volumes to Capacity	0.06	0.07	0.00									
Queue Length 95th (m)	1.3	1.8	0.0									
Control Delay (s)	10.5	9.3	0.0									
Lane LOS	B	A	A									
Approach Delay (s)	10.5	9.3	0.0									
Approach LOS	B	A	A									
Intersection Summary												
Average Delay	3.9											
Intersection Capacity Utilization	22.8%											
ICU Level of Service	A											
Analysis Period (min)	15											

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	65	0	480	605	10
Future Volume (Veh/h)	0	65	0	480	605	10
Sign Control	Stop					
Grade	0%					
Peak Hour Factor	0.90					
Hourly flow rate (vph)	0	72	0	533	672	11
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None					
Median type	None					
Median storage (veh)						
Upstream signal (m)	244					
pX platoon unblocked						
VC, conflicting volume	1210	678	683			
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	893	531	537			
IC, single (s)	6.4	6.2	4.1			
IC, 2 stage (s)						
IF (s)	3.5	3.3	2.2			
p0 queue free %	100	85	100			
CM capacity (veh/h)	285	469	883			
Direction, Lane #	EB 1	NB 1	SB 1			
Volumes Total	72	533	683			
Volume Left	0	0	0			
Volume Right	72	0	11			
cSH	469	1700	1700			
Volumes to Capacity	0.15	0.31	0.40			
Queue Length 95th (m)	4.1	0.0	0.0			
Control Delay (s)	14.1	0.0	0.0			
Lane LOS	B	A	A			
Approach Delay (s)	14.1	0.0	0.0			
Approach LOS	B	A	A			
Intersection Summary						
Average Delay	0.8					
Intersection Capacity Utilization	43.1%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis  
 31: Site Laneway & Gerrard Street E

HCM Unsignalized Intersection Capacity Analysis  
 32: Oak Street & Site Laneway

2032 Future Total AM Model  
 11-07-2022

2032 Future Total AM Model  
 11-07-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	350	10	0	890	0	10
Future Volume (Veh/h)	350	10	0	890	0	10
Sign Control	Free	Free	Stop	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	389	11	0	989	0	11
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)	169			83		
pX platoon unblocked						
VC conflicting volume		400			889	200
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol		400			524	200
IC single (s)		4.1			6.8	6.9
IC 2 stage (s)						
p0 queue free %		2.2			3.5	3.3
IF (s)		100			100	99
CM capacity (veh/h)		1170			416	814
Direction Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	
Volumes Total	259	141	494	494	11	
Volume Left	0	0	0	0	0	
Volume Right	0	11	0	0	11	
CSH	1700	1700	1700	1700	814	
Volumes to Capacity	0.15	0.08	0.29	0.29	0.01	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.3	
Control Delay (s)	0.0	0.0	0.0	0.0	9.5	
Lane LOS	A	A	A	A	A	
Approach Delay (s)	0.0	0.0	0.0	9.5		
Approach LOS	A	A	A	A		
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	27.9%					
ICU Level of Service	A					
Analysis Period (min)	15					

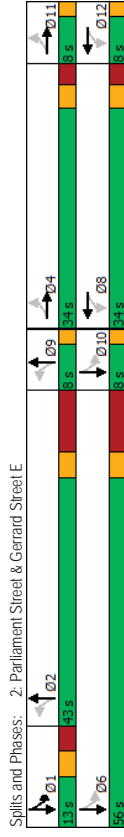
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	10	45	95	0	10	0
Future Volume (Veh/h)	10	45	95	0	10	0
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	11	50	106	0	11	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume		106			178	106
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol		106			178	106
IC single (s)		4.1			6.4	6.2
IC 2 stage (s)						
p0 queue free %		2.2			3.5	3.3
IF (s)		99			99	100
CM capacity (veh/h)		1488			810	954
Direction Lane #	EB 1	WB 1	SB 1			
Volumes Total	61	106	11			
Volume Left	11	0	11			
Volume Right	0	0	0			
CSH	1488	1700	810			
Volumes to Capacity	0.01	0.06	0.01			
Queue Length 95th (m)	0.2	0.0	0.3			
Control Delay (s)	1.4	0.0	9.5			
Lane LOS	A	A	A			
Approach Delay (s)	1.4	0.0	9.5			
Approach LOS	A	A	A			
Intersection Summary						
Average Delay	1.1					
Intersection Capacity Utilization	19.6%					
ICU Level of Service	A					
Analysis Period (min)	15					

2032 Future Total PM Model  
11-07-2022  
HCM Unsignalized Intersection Capacity Analysis  
1: Parliament Street & Gerrard Street E (North Section)

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				4A	4A	
Traffic Volume (veh/h)	0	0	10	525	590	75
Future Volume (Veh/h)	0	0	10	525	590	75
Sign Control	Stop					
Grade	0%					
Peak Hour Factor	0.89					
Hourly flow rate (vph)	0	0	11	590	663	84
Pedestrians	150					
Lane Width (m)	0.0					
Walking Speed (m/s)	1.1					
Percent Blockage	0					
Right turn flare (veh)	None					
Median type	None					
Median storage (veh)	39					
Upstream signal (m)	1182					
pX platoon unblocked	528					
VC, conflicting volume	897					
VC1, stage 1 conf vol	1182					
VC2, stage 2 conf vol	528					
VCU, unblocked vol	6.8					
IC, single (s)	3.5					
IC, 2 stage (s)	100					
p0 queue free %	100					
IF (s)	181					
CM capacity (veh/h)	208					
Direction, Lane #	NB 1		NB 2		SB 2	
Volumes Total	393		442		305	
Volume Left	0		0		0	
Volume Right	0		0		84	
cSH	699		1700		1700	
Volumes to Capacity	0.02		0.23		0.26	
Queue Length 95th (m)	0.4		0.0		0.0	
Control Delay (s)	0.7		0.0		0.0	
Lane LOS	A		A		A	
Approach Delay (s)	0.2		0.0		0.0	
Approach LOS	A		A		A	
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	33.2%					
Analysis Period (min)	15					
ICU Level of Service	A					

2032 Future Total PM Model  
11-07-2022  
Timings  
2: Parliament Street & Gerrard Street E

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations		4TB		4TB		4TB		4TB
Traffic Volume (vph)	30	57.5	45	265	80	365	245	275
Future Volume (vph)	30	57.5	45	265	80	365	245	275
Turn Type	Perm	NA	Perm	NA	Perm	NA	custom	NA
Protected Phases	4 11							
Permitted Phases	4 8 12							
Detector Phase	4 8 2 2 2 1 1 6							
Switch Phase	4 8 2 2 2 1 1 6							
Minimum Initial (s)	6.0							
Minimum Split (s)	12.5							
Total Split (s)	13.0							
Total Split (%)	12.3%							
Yellow Time (s)	3.3							
All-Red Time (s)	3.2							
Lost Time Adjust (s)								
Total Lost Time (s)								
Lead/Lag	Lead Lag							
Recall Mode	Yes Yes							
Ad Effct Green (s)	33.3							
Actuated g/C Ratio	0.34							
v/C Ratio	0.66							
Control Delay	30.7							
Queue Delay	0.0							
Total Delay	30.7							
LOS	C							
Approach Delay	30.7							
Approach LOS	C							
Intersection Summary								
Cycle Length: 106								
Actuated Cycle Length: 98								
Natural Cycle: 95								
Control Type: Actuated-Uncoordinated								
Maximum v/C Ratio: 0.82								
Intersection Signal Delay: 27.9								
Intersection Capacity Utilization 101.1%								
Analysis Period (min) 15								





Lane Group	Ø9	Ø10	Ø11	Ø12
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Turn Type	9	10	11	12
Protected Phases				
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	5.0	5.0	5.0	5.0
Minimum Split (s)	8.0	8.0	8.0	8.0
Total Split (s)	8.0	8.0	8.0	8.0
Total Split (%)	8%	8%	8%	8%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	None	None	None
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Intersection Summary				

	→	←	↑	↓
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	701	463	607	609
v/c Ratio	0.66	0.52	0.82	0.53
Control Delay	30.7	24.2	41.0	14.3
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	30.7	24.2	41.0	14.3
Queue Length 50th (m)	61.7	32.6	55.9	32.2
Queue Length 95th (m)	81.4	47.9	#84.6	44.3
Internal Link Dist (m)	33.0	65.6	119.2	15.0
Turn Bay Length (m)				
Base Capacity (vph)	1089	881	814	1190
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.64	0.53	0.75	0.51
Intersection Summary				

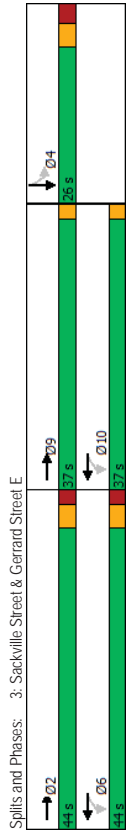
# 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

2. Parliament Street & Gerrard Street E  
 HCM Signalized Intersection Capacity Analysis  
 2032 Future Total PM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB							4TB	
Traffic Volume (vph)	30	575	75	45	265	140	80	365	145	245	275	70
Future Volume (vph)	30	575	75	45	265	140	80	365	145	245	275	70
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.8	4.8		4.8		10.2					5.5	
Lane Util. Factor	0.95	0.95		0.95		0.95					0.95	
Fpb. ped/bikes	0.98	0.98		0.95		0.92					0.99	
Fpb. ped/bikes	1.00	1.00		1.00		0.99					0.97	
Frt	0.98	0.98		0.95		0.96					0.98	
Frt Protected	1.00	1.00		1.00		0.99					0.98	
Sat'd. Flow (prot)	3398	3107		3107		3053					3174	
Flt Permitted	0.91	0.80		0.80		0.75					0.58	
Sat'd. Flow (perm)	3116	2487		2487		2307					1886	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	31	593	77	46	273	144	82	376	149	263	284	72
RTOR Reduction (vph)	0	9	0	0	46	0	0	23	0	0	11	0
Lane Group Flow (vph)	0	692	0	0	417	0	0	584	0	0	598	0
Conf. Peds. (#/hr)	130	120	120	130	125	235	235	235	235	235	125	125
Conf. Bikes (#/hr)			25			10			15		5	
Heavy Vehicles (%)	10%	0%	5%	0%	0%	10%	4%	2%	2%	6%	4%	0%
Turn Type	Perim	MA	Perm	MA	Perm	NA	Perm	NA	Perm	custom	MA	Perm
Protected Phases	4 11			8 12		2 9				1 6 10		
Permitted Phases										6 10		
Actuated Green, G (s)	36.6			36.6		44.9				58.0		
Effective Green, g (s)	37.6			37.6		45.9				48.8		
Actuated g/C Ratio	0.38			0.38		0.47				0.49		
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1188			948		1073				1032		
v/s Ratio Prot										c0.04		
v/s Ratio Perm	c0.22			0.17		60.25				c0.24		
v/c Ratio	0.58			0.44		0.54				0.58		
Uniform Delay, d1	24.3			22.7		18.9				17.6		
Progression Factor	1.00			1.00		1.00				1.00		
Incremental Delay, d2	0.7			0.3		0.6				0.8		
Delay (s)	25.0			23.0		19.4				18.4		
Level of Service	C			C		B				B		
Approach Delay (s)	25.0			23.0		19.4				18.4		
Approach LOS	C			C		B				B		
Intersection Summary												
HCM 2000 Control Delay	21.5 HCM 2000 Level of Service C											
HCM 2000 Volume to Capacity ratio	0.67											
Actuated Cycle Length (s)	98.6											
Intersection Capacity Utilization	101.1%											
Sum of lost time (s)	22.5											
ICU Level of Service	G											
Analysis Period (min)	15											
c Critical Lane Group												

3. Sackville Street & Gerrard Street E  
 Timings  
 2032 Future Total PM Model  
 11-07-2022

Lane Group	EBT	WBL	WBT	SBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations	4TB			4TB				
Traffic Volume (vph)	950	105	565	40				
Future Volume (vph)	950	105	565	40				
Turn Type	NA	Perm	NA	NA				
Protected Phases	2 9		6 10	4 2	6	9	10	
Permitted Phases								
Detector Phase	2	6	6	4				
Switch Phase								
Minimum Initial (s)	20.0	16.0	16.0	5.0				
Minimum (s)	25.7	21.0	21.0	37.0				
Total Split (s)	26.0	44.0	44.0	37.0				
Total Split (%)	24.3%	41%	41%	35%				
Yellow Time (s)	3.0	3.0	3.0	2.0				
All-Red Time (s)	2.7	2.0	2.0	0.0				
Lost Time Adjust (s)	-1.0							
Total Lost Time (s)	4.7							
Lead-Lag								
Lead-Lag Optimize?								
Recall Mode		Min	Min	None				
Ad. Eff. Green (s)	40.2	40.2	21.7					
Actuated g/C Ratio	0.57	0.57	0.31					
v/c Ratio	0.56	0.55	0.42					
Control Delay	10.1	10.8	25.4					
Queue Delay	0.0	0.0	0.0					
Total Delay	10.1	10.8	25.4					
LOS	B	B	C					
Approach Delay	10.1	10.8	25.4					
Approach LOS	B	B	C					
Intersection Summary								
Cycle Length	107							
Actuated Cycle Length	70.8							
Natural Cycle	85							
Control Type	Actuated-Uncoordinated							
Maximum v/c Ratio	0.56							
Intersection Signal Delay	12.0							
Intersection LOS	B							
Intersection Capacity Utilization	76.9%							
Analysis Period (min)	15							



Queues  
3: Sackville Street & Gerrard Street E

2032 Future Total PM Model  
11-07-2022

	→	←	←	→
	EBT	WBT	SBT	SBT
Lane Group	1073	698	209	
Lane Group Flow (vph)	0.56	0.55	0.42	
v/c Ratio	10.1	10.8	25.4	
Control Delay	0.0	0.0	0.0	
Queue Delay	10.1	10.8	25.4	
Total Delay	40.5	26.7	22.4	
Queue Length 50th (m)	54.3	39.1	48.0	
Queue Length 95th (m)	57.2	47.8	15.3	
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2161	1438	507	
Starvation Cap Reductn	0	0	0	
Spillback Cap Reductn	0	0	0	
Storage Cap Reductn	0	0	0	
Reduced v/c Ratio	0.50	0.49	0.41	
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis  
3: Sackville Street & Gerrard Street E

2032 Future Total PM Model  
11-07-2022

	→	←	←	→								
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4+4		4+4								
Traffic Volume (vph)	0	950	80	105	565	0	0	0	0	135	40	25
Future Volume (vph)	0	950	80	105	565	0	0	0	0	135	40	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	4.0	4.0										4.7
Lane Util. Factor		0.95		0.95								1.00
Frbp. ped/bikes		0.98		1.00								0.99
Frbp. ped/bikes		1.00		1.00								0.98
Frt		0.99		1.00								0.97
Flt Protected		1.00		0.99								1675
Satd. Flow (prot)		3380		3358								1675
Flt Permitted		1.00		0.66								0.97
Satd. Flow (perm)		3380		2243								1675
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	0	990	83	109	589	0	0	0	0	141	42	26
RTOR Reduction (vph)	0	8	0	0	0	0	0	0	0	0	0	4
Lane Group Flow (vph)	0	1065	0	0	698	0	0	0	0	0	0	205
Confl. Peds. (#/hr)	85	70	70	85	90	85	90	60	60	60	60	90
Confl. Bikes (#/hr)	0	60	60	15	15	15	15	5	5	5	5	10
Heavy Vehicles (%)	0%	3%	0%	0%	6%	0%	0%	0%	0%	1%	0%	0%
Turn Type	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
Protected Phases	2.9	6.10										4
Permitted Phases		6.10										4
Actuated Green, G (s)	42.7	42.7		42.7								20.6
Effective Green, g (s)	43.7	43.7		43.7								21.6
Actuated g/C Ratio	0.62	0.62		0.62								0.30
Clearance Time (s)												5.7
Vehicle Extension (s)												3.0
Lane Grp Cap (vph)	2080	1380		1380								509
v/s Ratio Prot	c0.32											
v/s Ratio Perm	0.51	0.31		0.51								0.12
v/c Ratio	7.7	7.6		7.6								0.40
Uniform Delay, d1	1.00	1.00		1.00								19.6
Progression Factor	0.2	0.3		0.3								1.00
Incremental Delay, d2	7.9	7.9		7.9								0.5
Delay (s)	A	A		A								20.1
Level of Service	A	A		A								C
Approach Delay (s)	7.9	7.9		7.9		0.0						20.1
Approach LOS	A	A		A		A						C
<b>Intersection Summary</b>												
HCM 2000 Control Delay	9.2		HCM 2000 Level of Service		A							
HCM 2000 Volume to Capacity ratio	0.49											
Actuated Cycle Length (s)	71.0											
Sum of lost time (s)	9.7											
Intersection Capacity Utilization	76.9%											
ICU Level of Service	D											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Unsignalized Intersection Capacity Analysis  
 4: Gerrard Street E & Gifford Street

HCM Unsignalized Intersection Capacity Analysis  
 5: Gerrard Street E & Nasmith Avenue

11-07-2022

11-07-2022



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4A	4B		W	
Traffic Volume (veh/h)	10	1075	665	5	15	5
Future Volume (Veh/h)	10	1075	665	5	15	5
Sign Control		Free	Free	Stop		
Grade		0%	0%	0%		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	12	1265	782	6	18	6
Pedestrians		5			65	
Lane Width (m)		3.5	3.0			
Walking Speed (m/s)		1.1	1.1			
Percent Blockage		0			5	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)						
Upstream signal (m)		72	141			
PX platoon unblocked	0.95			0.85	0.95	
VC conflicting volume	853			1506	464	
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	750			1004	342	
IC single (s)	4.1			6.8	6.9	
IC 2 stage (s)						
p0 queue free %	2.2			3.5	3.3	
IF (s)	98			91	99	
CM capacity (veh/h)	788			193	596	
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 1
Volumes Total	434	843	521	267	24	
Volume Left	12	0	0	0	18	
Volume Right	0	0	0	6	6	
cSH	788	1700	1700	232		
Volumes to Capacity	0.02	0.50	0.31	0.16	0.10	
Queue Length 95th (m)	0.4	0.0	0.0	0.0	2.6	
Control Delay (s)	0.5	0.0	0.0	0.0	22.3	
Lane LOS	A				C	
Approach Delay (s)	0.2		0.0		22.3	
Approach LOS					C	
Intersection Summary						
Average Delay	0.4					
Intersection Capacity Utilization	48.3%					
ICU Level of Service	A					
Analysis Period (min)	15					

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4A	4B		W	
Traffic Volume (veh/h)	5	1085	665	5	5	5
Future Volume (Veh/h)	5	1085	665	5	5	5
Sign Control		Free	Free	Stop		
Grade		0%	0%	0%		
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87
Hourly flow rate (vph)	6	1247	764	6	6	6
Pedestrians					60	
Lane Width (m)		3.0				
Walking Speed (m/s)		1.1				
Percent Blockage					5	
Right turn flare (veh)		None	None			
Median type		None	None			
Median storage (veh)						
Upstream signal (m)		145	68			
PX platoon unblocked	0.93			0.88	0.93	
VC conflicting volume	830			1462	445	
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	671			904	258	
IC single (s)	4.1			6.8	6.9	
IC 2 stage (s)						
p0 queue free %	2.2			3.5	3.3	
IF (s)	99			97	99	
CM capacity (veh/h)	826			234	665	
Direction Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 1
Volumes Total	422	831	509	261	12	
Volume Left	6	0	0	0	6	
Volume Right	0	0	0	6	6	
cSH	826	1700	1700	346		
Volumes to Capacity	0.01	0.49	0.30	0.15	0.03	
Queue Length 95th (m)	0.2	0.0	0.0	0.0	0.8	
Control Delay (s)	0.2	0.0	0.0	0.0	15.8	
Lane LOS	A				C	
Approach Delay (s)	0.1		0.0		15.8	
Approach LOS					C	
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	43.5%					
ICU Level of Service	A					
Analysis Period (min)	15					

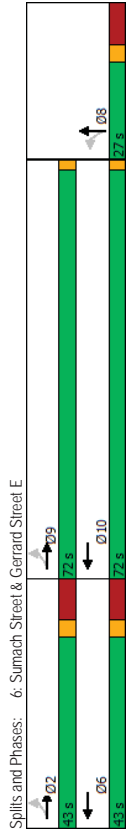
Timings 2032 Future Total PM Model 11-07-2022

Queues 2032 Future Total PM Model 11-07-2022

6: Summach Street & Gerrard Street E

6: Summach Street & Gerrard Street E

Lane Group	EBL	EBT	WBT	NBT	Ø2	Ø6	Ø9	Ø10
Lane Configurations								
Traffic Volume (vph)	10	1080	600	25				
Future Volume (vph)	10	1080	600	25				
Turn Type	Perm	NA	NA	NA				
Protected Phases		2 9	6 10	8	2	6	9	10
Permitted Phases	2 9							
Detector Phase	2 9	2 9	6 10	8				
Switch Phase								
Minimum Initial (s)					15.0	11.0	5.0	5.0
Minimum Split (s)					25.4	26.2	72.0	72.0
Total Split (s)					27.0	43.0	72.0	72.0
Total Split (%)					19.0%	30%	51%	51%
Yellow Time (s)					3.0	3.0	2.0	2.0
All-Red Time (s)					7.4	7.2	0.0	0.0
Lost Time Adjust (s)					-1.0			
Total Lost Time (s)					9.4			
Lead/Lag								
Lead-Lag Optimize?								
Recall Mode					None	Min	None	None
Act Effct Green (s)		56.2	56.2	17.8				
Actuated g/C Ratio		0.60	0.60	0.19				
v/c Ratio		0.56	0.36	0.68				
Control Delay		11.8	9.1	44.0				
Queue Delay		0.0	0.0	0.0				
Total Delay		11.9	9.1	44.0				
LOS		B	A	D				
Approach Delay		11.9	9.1	44.0				
Approach LOS		B	A	D				
Intersection Summary								
Cycle Length: 142								
Actuated Cycle Length: 92.9								
Natural Cycle: 125								
Control Type: Actuated-Uncoordinated								
Maximum v/c Ratio: 0.68								
Intersection Signal Delay: 14.2								
Intersection Capacity Utilization: 72.6%								
Analysis Period (min): 15								



Lane Group	EBT	WBT	NBT
Lane Group Flow (vph)	1123	707	211
v/c Ratio	0.56	0.36	0.68
Control Delay	11.8	9.1	44.0
Queue Delay	0.0	0.0	0.0
Total Delay	11.9	9.1	44.0
Queue Length 50th (m)	57.1	28.8	33.1
Queue Length 95th (m)	72.2	38.4	#67.5
Internal Link Dist (m)	44.0	75.7	30.7
Turn Bay Length (m)			
Base Capacity (vph)	3209	3036	311
Starvation Cap Reductn	258	0	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.38	0.23	0.68
Intersection Summary			
# 95th percentile volume exceeds capacity, queue may be longer.			
Queue shown is maximum after two cycles.			

6: Sumach Street & Gerrard Street E  
 HCM Signalized Intersection Capacity Analysis  
 2032 Future Total PM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4+1			1+1			4+1				
Traffic Volume (vph)	10	1080	0	0	600	85	70	25	110	0	0	0
Future Volume (vph)	10	1080	0	0	600	85	70	25	110	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	9.2			9.2			9.4					
Lane Util. Factor	0.95			0.95			1.00					
Frbp. ped/bikes	1.00			0.96			0.96					
Frbp. ped/bikes	1.00			1.00			0.95					
Frt	1.00			0.98			0.93					
Flt Protected	1.00			1.00			0.98					
Sat'd. Flow (prot)	3496			3257			1557					
Flt Permitted	0.95			1.00			0.98					
Sat'd. Flow (perm)	3311			3257			1557					
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	10	1113	0	0	619	88	72	26	113	0	0	0
RTOR Reduction (vph)	0	0	0	0	9	0	0	27	0	0	0	0
Lane Group Flow (vph)	0	1123	0	0	698	0	0	184	0	0	0	0
Confl. Peds. (#/hr)	95	95	95	95	95	80	35	35	35	35	35	80
Heavy Vehicles (%)	0%	2%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Turn Type	Perim	MA	NA	NA	Perm	NA	Perm	NA	NA	NA	NA	NA
Protected Phases	2.9			6.10			8					
Permitted Phases	2.9			6.10			8					
Effective Green, G (s)	63.4			63.4			16.8					
Actuated Green, g (s)	64.4			64.4			17.8					
Actuated g/C Ratio	0.70			0.70			0.19					
Clearance Time (s)							10.4					
Vehicle Extension (s)							3.0					
Lane Grp Cap (vph)	2302			2265			299					
v/s Ratio Prot	0.21			0.21								
v/s Ratio Perm	60.34			0.31			0.12					
v/c Ratio	0.49			0.31			0.62					
Uniform Delay, d1	6.5			5.5			34.3					
Progression Factor	1.00			1.00			1.00					
Incremental Delay, d2	0.2			0.1			3.8					
Delay (s)	6.7			5.5			38.0					
Level of Service	A			A			D					
Approach Delay (s)	6.7			5.5			38.0					0.0
Approach LOS	A			A			D					A
Intersection Summary												
HCM 2000 Control Delay	9.5 HCM 2000 Level of Service											
HCM 2000 Volume to Capacity ratio	0.57											
Actuated Cycle Length (s)	92.6 Sum of lost time (s)											
Intersection Capacity Utilization	72.6% ICU Level of Service											
Analysis Period (min)	15											
c Critical Lane Group												

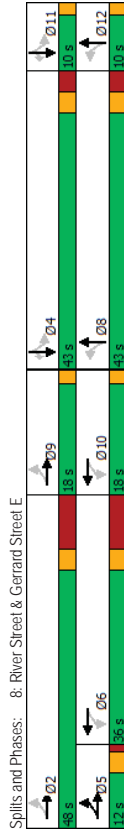
7: Site Laneway/Sword Street & Gerrard Street E  
 HCM Unsignalized Intersection Capacity Analysis  
 2032 Future Total PM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1+1			1+1			1+1				
Traffic Volume (veh/h)	0	1170	20	0	675	0	0	0	0	15	0	10
Future Volume (Veh/h)	0	1170	20	0	675	0	0	0	0	15	0	10
Sign Control		Free			Free			Stop				Stop
Grade		0%			0%			0%				0%
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	1300	22	0	750	0	0	0	0	17	0	11
Pedestrians												65
Lane Width (m)												3.0
Walking Speed (m/s)												1.1
Percent Blockage												5
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)		100			91							
pX platoon unblocked	0.93			0.85			0.89	0.89	0.85	0.89	0.89	0.93
vC, conflicting volume	815			1322			1697	2126	661	1465	2137	440
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	663			1033			1198	1682	258	936	1695	262
IC, single (s)	4.1			4.1			7.5	6.5	6.9	7.5	6.5	7.1
IC, 2 stage (s)												
IF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.4
p0 queue free %	100			100			100	100	100	91	100	98
d0 capacity (veh/h)	831			581			120	80	637	181	79	639
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1							
Volumes Total	867	455	375	375	28							
Volume Left	0	0	0	0	17							
Volume Right	0	22	0	0	11							
CSH	1700	1700	1700	1700	252							
Volumes to Capacity	0.51	0.27	0.22	0.22	0.11							
Queue Length 95th (m)	0.0	0.0	0.0	0.0	2.8							
Control Delay (s)	0.0	0.0	0.0	0.0	21.1							
Lane LOS					C							
Approach Delay (s)	0.0			0.0	21.1							
Approach LOS					C							
Intersection Summary												
Average Delay	0.3											
Intersection Capacity Utilization	43.0% ICU Level of Service											
Analysis Period (min)	15											

Timings  
8: River Street & Gerrard Street E

2032 Future Total PM Model  
11-07-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Lane Configurations	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR	Ø2	Ø4	Ø6
Traffic Volume (vph)	220	855	85	380	85	340	165	465	210			
Future Volume (vph)	220	855	85	380	85	340	165	465	210			
Turn Type	custom	NA	Perm	NA	Perm	NA	Perm	NA	Perm			
Protected Phases	5	5	2	9	6	10	8	12	4	11	2	4
Permitted Phases	5	5	2	9	6	10	8	12	4	11	2	4
Detector Phase												
Switch Phase												
Minimum Initial (s)	6.0									23.0	19.0	23.0
Minimum Spilt (s)	10.0									33.8	30.0	33.8
Total Spilt (s)	12.0									48.0	43.0	36.0
Total Spilt (%)	10.1%									40%	36%	30%
Yellow Time (s)	3.0									3.0	3.0	3.0
All-Red Time (s)	1.0									7.8	3.0	7.8
Lost Time Adjust (s)												
Total Lost Time (s)												
Lead/Lag	Lead											
Lead-Lag Optimize?	Yes											
Recall Mode	None									Min	Min	Min
Act Effct Green (s)	55.6									39.0	39.0	39.0
Actuated g/C Ratio	0.54									0.38	0.38	0.38
v/C Ratio	0.93									1.00	0.69	0.32
Control Delay	33.1									36.8	104.7	33.8
Queue Delay	24.4									0.0	0.0	0.0
Total Delay	57.5									35.4	104.7	33.8
LOS	E									D	F	C
Approach Delay	57.5									36.6	40.4	40.4
Approach LOS	E									D	D	D
Intersection Summary												
Cycle Length:	119											
Actuated Cycle Length:	102.7											
Natural Cycle:	105											
Control Type:	Actuated-Uncoordinated											
Maximum v/c Ratio:	1.00											
Intersection Signal Delay:	48.5											
Intersection Capacity Utilization:	117.6%											
Analysis Period (min):	15											



Timings  
8: River Street & Gerrard Street E

2032 Future Total PM Model  
11-07-2022

Lane Group	Ø8	Ø9	Ø10	Ø11	Ø12
Lane Configurations	Ø8	Ø9	Ø10	Ø11	Ø12
Traffic Volume (vph)					
Future Volume (vph)					
Turn Type					
Protected Phases	8	9	10	11	12
Permitted Phases					
Detector Phase					
Switch Phase					
Minimum Initial (s)	19.0	5.0	5.0	5.0	5.0
Minimum Spilt (s)	30.0	18.0	18.0	10.0	10.0
Total Spilt (s)	43.0	18.0	18.0	10.0	10.0
Total Spilt (%)	36%	15%	15%	8%	8%
Yellow Time (s)	3.0	2.0	2.0	2.0	2.0
All-Red Time (s)	3.0	0.0	0.0	0.0	0.0
Lost Time Adjust (s)					
Total Lost Time (s)					
Lead/Lag					
Lead-Lag Optimize?					
Recall Mode	Min	None	None	None	None
Actuated g/C Ratio					
v/C Ratio					
Control Delay					
Queue Delay					
Total Delay					
LOS					
Approach Delay					
Approach LOS					
Intersection Summary					

Queues  
8: River Street & Gerrard Street E

2032 Future Total PM Model  
11-07-2022

	EBT	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group							
Lane Group Flow (vph)	1235	594	89	521	172	484	219
v/c Ratio	0.93	0.94	0.46	0.77	1.00	0.69	0.32
Control Delay	33.1	54.2	35.4	36.8	104.7	33.8	4.5
Queue Delay	24.4	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	57.5	54.2	35.4	36.8	104.7	33.8	4.5
Queue Length 50th (m)	85.6	56.1	14.8	96.9	~41.0	89.8	0.0
Queue Length 95th (m)	#155.1	#101.1	31.1	138.5	#82.6	126.1	14.6
Internal Link Dist (m)	67.0	81.6		27.9		61.7	
Turn Bay Length (m)			30.0				
Base Capacity (vph)	1334	635	200	697	178	730	690
Stavation Cap Reductn	155	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	1.05	0.94	0.45	0.75	0.97	0.66	0.32
<b>Intersection Summary</b>							
-	Volume exceeds capacity, queue is theoretically infinite.						
-	Queue shown is maximum after two cycles.						
#	95th percentile volume exceeds capacity, queue may be longer.						
-	Queue shown is maximum after two cycles.						

HCM Signalized Intersection Capacity Analysis  
8: River Street & Gerrard Street E

2032 Future Total PM Model  
11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4TB		4TB								
Traffic Volume (vph)	220	855	110	85	380	105	85	340	160	165	465	210
Future Volume (vph)	220	855	110	85	380	105	85	340	160	165	465	210
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	3.0	9.5	3.0	9.8	9.8	3.0	5.0	5.0	3.0	5.0	5.0	3.0
Lane Util. Factor	0.90	0.95	0.95	0.95	0.95	0.95	1.00	1.00	0.98	1.00	1.00	0.96
Frpb, ped/bikes	0.99	0.99	0.99	0.99	0.99	0.99	1.00	0.98	1.00	1.00	1.00	0.96
Flpb, psd/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	0.98	1.00	1.00	0.96
Flt	0.99	0.99	0.99	0.97	0.97	0.97	1.00	0.95	1.00	1.00	1.00	0.85
Flt Protected	0.99	0.99	0.99	0.99	0.99	0.99	1.00	0.95	1.00	1.00	1.00	0.85
Satd. Flow (prot)	3354	3272	3272	1625	1742	1625	1742	1643	1860	1860	1860	1424
Flt Permitted	0.67	0.53	0.53	0.53	0.53	0.53	0.30	1.00	0.26	1.00	1.00	1.00
Satd. Flow (perm)	2277	1745	1745	512	1742	512	1742	456	1860	1860	1424	1424
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	229	891	115	89	396	109	89	354	167	172	484	219
RTOR Reduction (vph)	0	8	0	0	15	0	0	14	0	0	0	92
Lane Group Flow (vph)	0	1227	0	0	579	0	89	507	0	172	484	93
Conf. Peds. (#/hr)	40	80	80	40	15	40	15	40	40	40	15	15
Conf. Bikes (#/hr)	35	35	35	15	15	15	15	15	15	15	15	10
Heavy Vehicles (%)	3%	2%	3%	0%	4%	0%	3%	0%	1%	1%	1%	2%
Turn Type	custom	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
Protected Phases	5	5	2	9	6	10	8	12	8	12	4	11
Permitted Phases	2	9		6	10		8	12		8	11	4
Actuated Green, G (s)	57.4	57.4	57.4	45.3	45.3	45.3	43.6	43.6	43.6	43.6	43.6	43.6
Effective Green, g (s)	48.6	48.6	48.6	46.3	46.3	46.3	44.6	44.6	44.6	44.6	44.6	44.6
Actuated g/C Ratio	0.46	0.46	0.46	0.44	0.44	0.44	0.42	0.42	0.42	0.42	0.42	0.42
Clearance Time (s)												
Vehicle Extension (s)												
Lane Grp Cap (vph)	1147	1147	1147	769	769	769	217	739	193	790	604	604
v/s Ratio Prot	c0.09	c0.09	c0.09	c0.33	c0.33	c0.33	0.17	0.29	0.26	0.26	0.26	0.26
v/c Ratio Perm	c0.40	c0.40	c0.40	0.75	0.75	0.75	0.41	0.69	0.89	0.89	0.61	0.15
v/c Ratio	1.07	1.07	1.07	0.75	0.75	0.75	0.41	0.69	0.89	0.89	0.61	0.15
Uniform Delay, d1	28.2	28.2	28.2	24.6	24.6	24.6	21.0	24.5	28.0	23.5	18.6	18.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	47.5	47.5	47.5	4.2	4.2	4.2	1.3	2.7	36.2	1.4	0.1	0.1
Delay (s)	75.7	75.7	75.7	28.8	28.8	28.8	22.3	27.2	64.1	24.9	18.7	18.7
Level of Service	E	E	E	C	C	C	C	C	E	C	C	B
Approach Delay (s)	75.7	75.7	75.7	28.8	28.8	28.8	26.5	31.1	31.1	31.1	31.1	31.1
Approach LOS	E	E	E	C	C	C	C	C	E	C	C	B
<b>Intersection Summary</b>												
HCM 2000 Control Delay	46.4											
HCM 2000 Level of Service	D											
HCM 2000 Volume to Capacity ratio	1.04											
Actuated Cycle Length (s)	105.0											
Sum of lost time (s)	19.8											
Intersection Capacity Utilization	117.6%											
ICU Level of Service	H											
Analysis Period (min)	15											
c Critical Lane Group												



HCM Unsignalized Intersection Capacity Analysis  
 9. Sackville Street & Site Driveway

HCM Unsignalized Intersection Capacity Analysis  
 10. Parliament Street & Oak Street

11-07-2022

11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	40	25	0	0	0	0	0	45	125	55
Future Volume (Veh/h)	0	0	40	25	0	0	0	0	0	45	125	55
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	0	0	47	29	0	0	0	0	0	53	147	65
Pedestrians	60	3.5	3.5	3.5	0.0	0.0	5	5	5	15	15	15
Lane Width (m)	3.5	3.5	3.5	3.5	1.1	1.1	1.1	1.1	1.1	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	5	5	3	3	0	0	0	0	0	1	1	1
Right turn flare (veh)												
Median type							None	None	None	None	None	None
Median storage (veh)												
Upstream signal (m)												58
pX platoon unblocked												
VC, conflicting volume	360	380	244	372	413	50	272			35		
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	360	380	244	372	413	50	272			35		
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
IC, 2 stage (s)												
p0 queue free %	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
IF (s)	100	100	94	94	100	100	100			97		
CM capacity (veh/h)	511	492	757	488	472	979	1234			1540		
Direction, Lane #	EB 1	WB 1	SB 1									
Volumes Total	47	29	265									
Volume Left	0	29	53									
Volume Right	47	0	65									
cSH	757	488	1540									
Volumes to Capacity	0.06	0.06	0.03									
Queue Length 95th (m)	1.5	1.4	0.8									
Control Delay (s)	10.1	12.8	1.7									
Lane LOS	B	B	A									
Approach Delay (s)	10.1	12.8	1.7									
Approach LOS	B	B	A									
Intersection Summary												
Average Delay			3.8									
Intersection Capacity Utilization			37.0%									A
Analysis Period (min)			15									

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	45	70	520	30	15	380
Future Volume (Veh/h)	45	70	520	30	15	380
Sign Control	Stop	Stop	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85
Hourly flow rate (vph)	53	82	612	35	18	447
Pedestrians	220	3.0	3.5	5	5	55
Lane Width (m)	3.0	3.0	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	17	17	0	0	0	5
Right turn flare (veh)						
Median type			None	None	None	None
Median storage (veh)						
Upstream signal (m)			151			143
pX platoon unblocked						
VC, conflicting volume	1114	598				867
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	943	381				674
IC, single (s)	6.8	6.9				4.3
IC, 2 stage (s)						
p0 queue free %	3.5	3.3				2.3
IF (s)	73	82				97
CM capacity (veh/h)	196	453				649
Direction, Lane #	WB 1	NB 2	SB 1	SB 2		
Volumes Total	135	408	239	167	298	
Volume Left	53	0	0	18	0	
Volume Right	82	0	35	0	0	
cSH	299	1700	1700	649	1700	
Volumes to Capacity	0.45	0.24	0.14	0.03	0.18	
Queue Length 95th (m)	17.0	0.0	0.0	0.6	0.0	
Control Delay (s)	26.6	0.0	0.0	1.4	0.0	
Lane LOS	D	D	A	A	A	
Approach Delay (s)	26.6	0.0	0.0	0.5	0.0	
Approach LOS	D	D	A	A	A	
Intersection Summary						
Average Delay			3.1			
Intersection Capacity Utilization			40.8%			A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis  
 11: Oak Street & Dreamer's Way

HCM Unsignalized Intersection Capacity Analysis  
 12: Regent Street & Oak Street

2032 Future Total PM Model  
 11-07-2022

2032 Future Total PM Model  
 11-07-2022

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Traffic Volume (veh/h)	0	50	65	0	10	45
Future Volume (Veh/h)	0	50	65	0	10	45
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	54	70	0	11	48
Pedestrians		25	10		40	
Lane Width (m)		3.5	3.5		3.0	
Walking Speed (m/s)		1.1	1.1		1.1	
Percent Blockage		2	1		3	
Right turn flare (veh)						
Median type		None	None		None	
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume	110				174	135
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol	110				174	135
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)	2.2				3.5	3.3
p0 queue free %	100				99	94
CM capacity (veh/h)	1448				789	872
Direction_Lane #	EB 1	WB 1	SB 1			
Volumes Total	54	70	59			
Volume Left	0	0	11			
Volume Right	0	0	48			
CSH	1700	1700	855			
Volumes to Capacity	0.03	0.04	0.07			
Queue Length 95th (m)	0.0	0.0	1.7			
Control Delay (s)	0.0	0.0	9.5			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	9.5			
Approach LOS			A			
Intersection Summary						
Average Delay			3.1			
Intersection Capacity Utilization			27.2%		ICU Level of Service	A
Analysis Period (min)			15			

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑		↑	↑	↑
Traffic Volume (veh/h)	60	0	0	50	15	45
Future Volume (Veh/h)	60	0	0	50	15	45
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83
Hourly flow rate (vph)	72	0	0	60	18	54
Pedestrians	15			15	40	
Lane Width (m)	3.5			3.5	3.0	
Walking Speed (m/s)	1.1			1.1	1.1	
Percent Blockage	1			1	3	
Right turn flare (veh)						
Median type	None	None	None	None	None	None
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC conflicting volume			112		187	127
VC1 stage 1 conf vol						
VC2 stage 2 conf vol						
VCu unblocked vol			112		187	127
IC single (s)			4.1		6.4	6.2
IC 2 stage (s)			2.2		3.5	3.3
p0 queue free %	100		100		98	94
CM capacity (veh/h)	1445		1445		772	889
Direction_Lane #	EB 1	WB 1	NB 1			
Volumes Total	72	60	72			
Volume Left	0	0	18			
Volume Right	0	0	54			
CSH	1700	1700	856			
Volumes to Capacity	0.04	0.04	0.08			
Queue Length 95th (m)	0.0	0.0	2.1			
Control Delay (s)	0.0	0.0	9.6			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	9.6			
Approach LOS			A			
Intersection Summary						
Average Delay			3.4			
Intersection Capacity Utilization			27.2%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis  
 14: Sackville Street & Oak Street

2032 Future Total PM Model  
 15: Sumach Street & Oak Street

11-07-2022

11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop		Stop		Stop		Stop				Stop
Traffic Volume (vph)	0	80	15	0	5	0	0	0	0	0	65	95
Future Volume (vph)	0	80	15	0	5	0	0	0	0	0	65	95
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	89	17	0	6	0	0	0	0	0	72	106
Direction, Lane #												
	EB 1	WB 1	SB 1									
Volume Total (vph)	106	6	211									
Volume Left (vph)	0	0	72									
Volume Right (vph)	17	0	33									
Head (s)	-0.07	0.00	0.04									
Departure Headway (s)	4.3	4.5	4.2									
Degree Utilization, x	0.13	0.01	0.25									
Capacity (veh/h)	795	746	832									
Control Delay (s)	8.0	7.5	8.6									
Approach Delay (s)	8.0	7.5	8.6									
Approach LOS	A	A	A									
Intersection Summary												
Delay	8.3											
Level of Service	A											
Intersection Capacity Utilization	36.9%											
ICU Level of Service	A											
Analysis Period (min)	15											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop		Stop		Stop		Stop				Stop
Traffic Volume (vph)	55	65	25	0	0	0	0	5	175	75	0	0
Future Volume (vph)	55	65	25	0	0	0	0	5	175	75	0	0
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	60	71	27	0	0	0	0	5	192	82	0	0
Direction, Lane #												
	EB 1	NB 1										
Volume Total (vph)	188	279										
Volume Left (vph)	60	5										
Volume Right (vph)	27	82										
Head (s)	0.04	-0.10										
Departure Headway (s)	4.6	4.2										
Degree Utilization, x	0.20	0.32										
Capacity (veh/h)	740	832										
Control Delay (s)	8.7	9.2										
Approach Delay (s)	8.7	9.2										
Approach LOS	A	A										
Intersection Summary												
Delay	9.0											
Level of Service	A											
Intersection Capacity Utilization	40.5%											
ICU Level of Service	A											
Analysis Period (min)	15											

16: Tubman Avenue/Site Laneway & Oak Street  
 HCM Unsignalized Intersection Capacity Analysis  
 2032 Future Total PM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	130	10	0	0	0	0	0	0	10	10	0
Future Volume (Veh/h)	0	130	10	0	0	0	0	0	0	10	10	0
Sign Control		Free		Free			Stop			Stop		
Grade		0%		0%			0%			0%		
Peak Hour Factor	0.93	0.93	0.83	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	0	140	11	0	0	0	0	0	0	11	11	0
Pedestrians		20		5			30					
Lane Width (m)		3.5		0.0			0.0					
Walking Speed (m/s)		1.1		1.1			1.1					
Percent Blockage		2		0			0					
Right turn flare (veh)												
Median type		None		None			None					
Median storage (veh)												
Upstream signal (m)												
pX platoon unblocked												
VC, conflicting volume	0			181			201		176	180	150	181
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	0			181			201		176	180	150	181
IC, single (s)	4.1			4.1			7.1		6.5	6.2	7.1	6.5
IC, 2 stage (s)												
IF (s)	2.2			2.2			3.5		4.0	3.3	3.5	4.0
p0 queue free %	100			100			100		100	100	99	98
pM capacity (veh/h)	1636			1407			740		722	867	822	717
Direction, Lane #	EB 1	SB 1										
Volumes Total	151	22										
Volume Left	0	11										
Volume Right	11	0										
cSH	1700	766										
Volumes to Capacity	0.09	0.03										
Queue Length 95th (m)	0.0	0.7										
Control Delay (s)	0.0	9.8										
Lane LOS	A	A										
Approach Delay (s)	0.0	9.8										
Approach LOS	A	A										
Intersection Summary												
Average Delay		1.3										A
Intersection Capacity Utilization		28.4%										
Analysis Period (min)		15										

18: River Street & Oak Street  
 HCM Unsignalized Intersection Capacity Analysis  
 2032 Future Total PM Model  
 11-07-2022

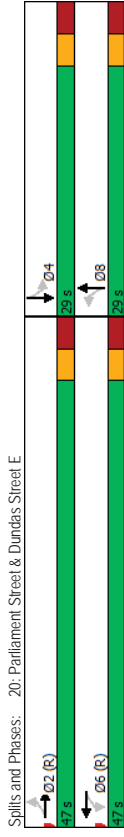
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	85	5	50	20	0	50	0	450	40	50	595	0
Future Volume (Veh/h)	85	5	50	20	0	50	0	450	40	50	595	0
Sign Control		Stop		Stop			Free			Free		
Grade		0%		0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	92	5	54	22	0	54	0	489	43	54	647	0
Pedestrians		75		45			50					5
Lane Width (m)		3.5		3.5			3.5					3.5
Walking Speed (m/s)		1.1		1.1			1.1					1.1
Percent Blockage		7		4			4					0
Right turn flare (veh)												
Median type		None		None			None					
Median storage (veh)												
Upstream signal (m)									143			151
pX platoon unblocked	0.86	0.86	0.80	0.86	0.86	0.87	0.80			0.87		
VC, conflicting volume	1400	1407	772	1417	1386	560	722			577		
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	1063	1062	587	1074	1037	425	524			444		
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	29	97	85	81	100	90	100			94		
pM capacity (veh/h)	130	164	359	115	170	529	784			946		
Direction, Lane #	EB 1	WB 1	NB 1				SB 1					
Volumes Total	151	76	532	701								
Volume Left	92	22	0	54								
Volume Right	54	54	43	0								
cSH	170	259	1700	946								
Volumes to Capacity	0.89	0.29	0.31	0.06								
Queue Length 95th (m)	48.8	9.0	0.0	1.4								
Control Delay (s)	96.0	24.5	0.0	1.5								
Lane LOS	F	C	A	A								
Approach Delay (s)	96.0	24.5	0.0	1.5								
Approach LOS	F	C	C	C								
Intersection Summary												
Average Delay			11.9									E
Intersection Capacity Utilization			86.6%									
Analysis Period (min)			15									

11-07-2022  
 2032 Future Total PM Model  
 HCM Unsignalized Intersection Capacity Analysis  
 19: Parliament Street & Cole Street

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			←	←	←	←
Traffic Volume (veh/h)	0	0	550	20	25	400
Future Volume (Veh/h)	0	0	550	20	25	400
Sign Control	Stop	Stop	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84
Hourly flow rate (vph)	0	0	655	24	30	476
Pedestrians	255	0	0	0	0	15
Lane Width (m)	0.0	0.0	3.5	3.5	3.5	3.5
Walking Speed (m/s)	1.1	1.1	1.1	1.1	1.1	1.1
Percent Blockage	0	0	2	2	2	1
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)			80			214
pX platoon unblocked	0.88	0.88				0.88
VC, conflicting volume	1245	610				934
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	1012	293				660
IC, single (s)	6.8	6.9				4.1
IC, 2 stage (s)						
IF (s)	3.5	3.3				2.2
p0 queue free %	100	100				96
CM capacity (veh/h)	199	618				828
Direction, Lane #	NB 1	NB 2	SB 1	SB 2		
Volumes Total	437	242	189	317		
Volume Left	0	0	30	0		
Volume Right	0	24	0	0		
cSH	1700	1700	828	1700		
Volumes to Capacity	0.26	0.14	0.04	0.19		
Queue Length 95th (m)	0.0	0.0	0.9	0.0		
Control Delay (s)	0.0	0.0	1.8	0.0		
Lane LOS			A			
Approach Delay (s)	0.0		0.7			
Approach LOS						
Intersection Summary						
Average Delay	0.3					
Intersection Capacity Utilization	43.8%					
Analysis Period (min)	15					
ICU Level of Service	A					

11-07-2022  
 2032 Future Total PM Model  
 Timings  
 20: Parliament Street & Dundas Street E

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBT
Lane Configurations	←	←	←	←	←	←	←
Traffic Volume (vph)	65	490	50	190	45	435	45
Future Volume (vph)	65	490	50	190	45	435	45
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm
Protected Phases	2		6		8		4
Permitted Phases	2	2	6	6	8	8	4
Detector Phase							
Switch Phase							
Minimum Initial (s)	23.0	23.0	23.0	23.0	23.0	23.0	23.0
Minimum Split (s)	29.0	29.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	47.0	47.0	47.0	47.0	47.0	47.0	47.0
Total Split (%)	61.8%	61.8%	61.8%	61.8%	38.2%	38.2%	38.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)							
Lead/Lag		5.0		5.0		5.0	5.0
Lead-Lag Optimize?							
Recall Mode							
Act Effct Green (s)	41.0	41.0	C-Min	C-Min	Min	Min	Min
Actuated g/C Ratio	0.54	0.54	0.54	0.54	0.33	0.33	0.33
v/C Ratio	0.45	0.27	0.62	0.62	0.52	0.52	0.52
Control Delay	11.7	12.6	24.1	24.1	22.6	22.6	22.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.7	12.6	24.1	24.1	22.6	22.6	22.6
LOS	B	B	C	C	C	C	C
Approach Delay	11.7	12.6	24.1	24.1	22.6	22.6	22.6
Approach LOS	B	B	C	C	C	C	C
Intersection Summary							
Cycle Length: 76							
Actuated Cycle Length: 76							
Offset: 9 (12%) Referenced to phase 2:EBTL and 6:WBTL Start of 1st Green							
Natural Cycle: 60							
Control Type: Actuated-Coordinated							
Maximum v/C Ratio: 0.62							
Intersection Signal Delay: 17.7							
Intersection Capacity Utilization 93.3%							
Analysis Period (min) 15							



Queues  
20: Parliament Street & Dundas Street E

HCM Signalized Intersection Capacity Analysis  
20: Parliament Street & Dundas Street E

11-07-2022

2032 Future Total PM Model  
11-07-2022

	EBT	WBT	NBT	SBT
Lane Group	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	691	348	585	449
v/c Ratio	0.45	0.27	0.62	0.52
Control Delay	11.7	12.6	24.1	22.6
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	11.7	12.6	24.1	22.6
Queue Length 50th (m)	27.3	10.1	36.7	27.2
Queue Length 95th (m)	44.2	28.5	47.7	36.8
Internal Link Dist (m)	85.0	103.5	45.0	56.1
Turn Bay Length (m)				
Base Capacity (vph)	1557	1325	949	867
Starvation Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.44	0.26	0.62	0.52
Intersection Summary				

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4TB			4TB			4TB					
Traffic Volume (vph)	65	490	60	50	190	70	45	435	40	45	335	20	
Future Volume (vph)	65	490	60	50	190	70	45	435	40	45	335	20	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	
Lane Util. Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Frbp. ped/bikes	0.98	0.98	0.98	0.94	0.94	0.98	0.98	0.98	0.98	0.98	0.99	0.99	
Frbp. psd/bikes	0.98	0.98	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Frt	0.99	0.99	0.97	0.97	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Flt Protected													
Satd. Flow (prot)	3205	3008	3008	3008	3280	3280	3280	3280	3280	3280	3216	3216	
Flt Permitted	0.87	0.79	0.79	0.79	0.87	0.87	0.87	0.87	0.87	0.87	0.81	0.81	
Satd. Flow (perm)	2799	2384	2384	2384	2868	2868	2868	2868	2868	2868	2627	2627	
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	
Adj. Flow (vph)	73	551	67	56	213	79	51	489	45	51	376	22	
RTOR Reduction (vph)	0	11	0	0	8	0	0	8	0	0	5	0	
Lane Group Flow (vph)	0	680	0	0	340	0	0	577	0	0	444	0	
Confl. Peds. (#/hr)	260	155	155	155	260	165	225	225	225	225	165	165	
Confl. Bikes (#/hr)	25	25	25	25	20	20	20	20	20	20	10	10	
Heavy Vehicles (%)	3%	6%	5%	5%	6%	6%	3%	5%	0%	3%	8%	10%	
Parking (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0	
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA	
Protected Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Permitted Phases	2	2	2	6	6	6	8	8	8	8	4	4	
Actuated Green, G (s)	40.0	40.0	40.0	40.0	40.0	40.0	24.0	24.0	24.0	24.0	24.0	24.0	
Effective Green, g (s)	41.0	41.0	41.0	41.0	41.0	41.0	25.0	25.0	25.0	25.0	25.0	25.0	
Actuated g/C Ratio	0.54	0.54	0.54	0.54	0.54	0.54	0.33	0.33	0.33	0.33	0.33	0.33	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	1509	1286	1286	1286	1286	943	943	943	943	943	864	864	
v/s Ratio Prot													
v/s Ratio Perm	0.24	0.14	0.14	0.14	0.20	0.20	0.17	0.17	0.17	0.17	0.17	0.17	
v/c Ratio	0.45	0.26	0.26	0.26	0.61	0.61	0.51	0.51	0.51	0.51	0.51	0.51	
Uniform Delay, d1	10.6	9.4	9.4	9.4	21.4	21.4	20.6	20.6	20.6	20.6	20.6	20.6	
Progression Factor	1.00	1.29	1.29	1.29	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	1.0	0.5	0.5	0.5	1.2	1.2	0.5	0.5	0.5	0.5	0.5	0.5	
Delay (s)	11.6	12.6	12.6	12.6	22.6	22.6	21.1	21.1	21.1	21.1	21.1	21.1	
Level of Service	B	B	B	B	C	C	C	C	C	C	C	C	
Approach Delay (s)	11.6	12.6	12.6	12.6	22.6	22.6	21.1	21.1	21.1	21.1	21.1	21.1	
Approach LOS	B	B	B	B	C	C	C	C	C	C	C	C	
Intersection Summary													
HCM 2000 Control Delay	16.9											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.51												
Actuated Cycle Length (s)	76.0											Sum of lost time (s)	10.0
Intersection Capacity Utilization	93.3%											ICU Level of Service	F
Analysis Period (min)	15												
c Critical Lane Group													

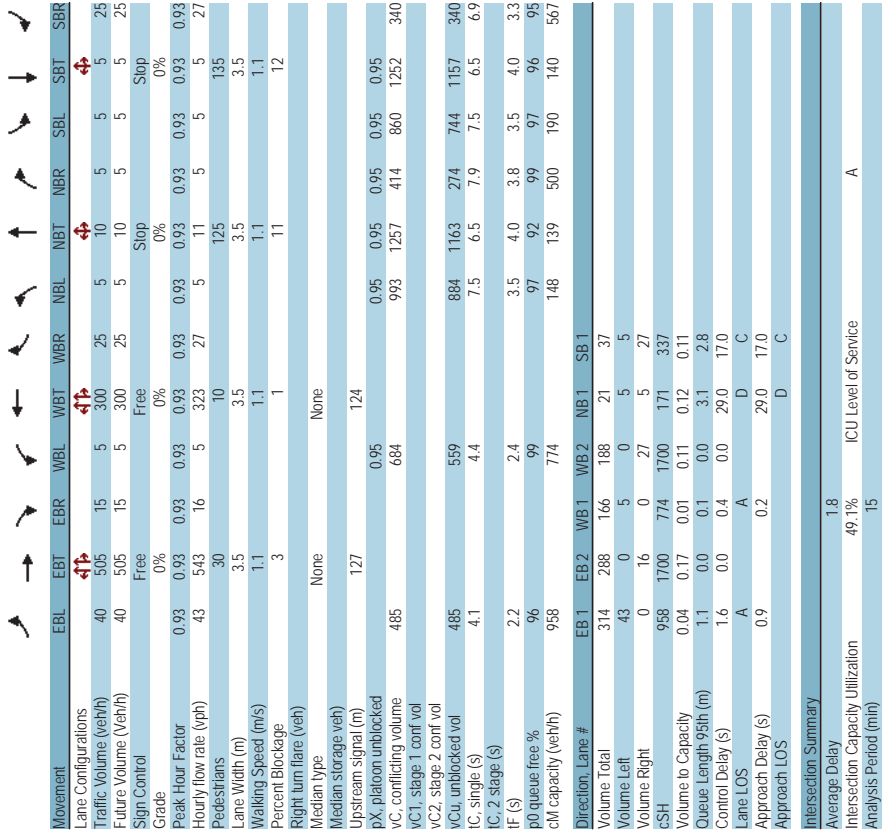
21: Regent Street & Dundas Street E

22: Sackville Street & Dundas Street E

2032 Future Total PM Model  
11-07-2022

2032 Future Total PM Model  
11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	40	505	15	5	300	25	5	10	5	5	5	25
Traffic Volume (veh/h)	40	505	15	5	300	25	5	10	5	5	5	25
Future Volume (Veh/h)	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop	Slop	Slop
Sign Control	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Grade	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Peak Hour Factor	43	543	16	5	323	27	5	11	5	5	5	27
Hourly flow rate (vph)	30	30	30	10	10	10	125	125	125	135	135	135
Pedestrians	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Lane Width (m)	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1
Walking Speed (m/s)	3	3	3	3	3	3	3	3	3	3	3	3
Percent Blockage	None	None	None	None	None	None	None	None	None	None	None	None
Right turn flare (veh)	None	None	None	None	None	None	None	None	None	None	None	None
Median type	127	127	124	124	124	124	124	124	124	124	124	124
Median storage (veh)	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Upstream signal (m)	684	684	684	684	684	684	684	684	684	684	684	684
pX platoon unblocked	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
VC, conflicting volume	993	993	1257	414	860	1252	340	340	340	340	340	340
VC1, stage 1 conf vol	884	884	1163	274	744	1157	340	340	340	340	340	340
VC2, stage 2 conf vol	7.5	7.5	6.5	7.9	7.5	6.5	6.9	6.9	6.9	6.9	6.9	6.9
VCU, unblocked vol	2.2	2.2	2.4	3.5	4.0	3.8	3.5	4.0	3.3	3.3	3.3	3.3
IC, single (s)	96	96	99	97	92	99	97	96	95	95	95	95
IF (s)	958	958	774	148	139	500	190	140	567	567	567	567
CM capacity (veh/h)	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1	SB 1	SB 1	SB 1	SB 1	SB 1
Direction, Lane #	314	288	166	188	21	37	37	37	37	37	37	37
Volumes Total	43	0	5	0	5	5	5	5	5	5	5	5
Volume Left	0	16	0	27	5	27	27	27	27	27	27	27
Volume Right	958	1700	774	1700	171	337	337	337	337	337	337	337
cSH	0.04	0.17	0.01	0.11	0.12	0.11	0.11	0.11	0.12	0.11	0.11	0.11
Volumes to Capacity	1.1	0.0	0.1	0.0	3.1	2.8	2.8	2.8	2.8	2.8	2.8	2.8
Queue Length 95th (m)	1.6	0.0	0.4	0.0	29.0	17.0	17.0	17.0	17.0	17.0	17.0	17.0
Control Delay (s)	A	A	A	A	D	C	C	C	C	C	C	C
Lane LOS	0.9	0.2	0.2	29.0	17.0	17.0	17.0	17.0	17.0	17.0	17.0	17.0
Approach Delay (s)	D	D	D	D	D	D	D	D	D	D	D	D
Approach LOS	1.8	1.8	1.8	1.8	1.8	1.8	1.8	1.8	1.8	1.8	1.8	1.8
Average Delay	49.1%	49.1%	49.1%	49.1%	49.1%	49.1%	49.1%	49.1%	49.1%	49.1%	49.1%	49.1%
Intersection Capacity Utilization	15	15	15	15	15	15	15	15	15	15	15	15
Analysis Period (min)	A	A	A	A	A	A	A	A	A	A	A	A
ICU Level of Service	A	A	A	A	A	A	A	A	A	A	A	A



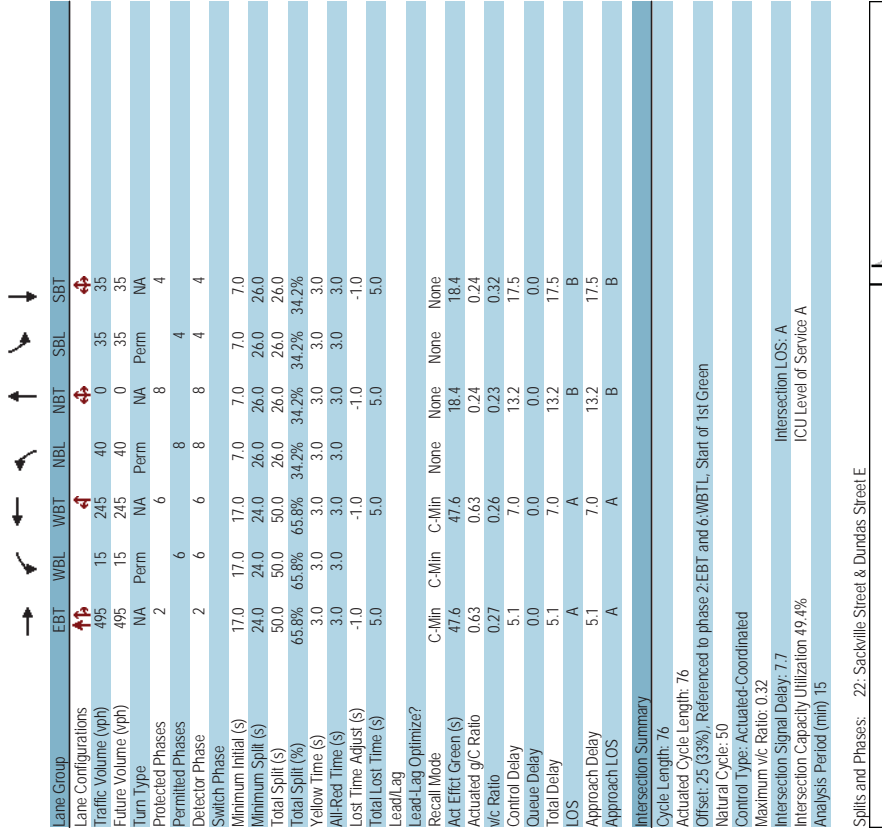
Spills and Phases: 22: Sackville Street & Dundas Street E

Regent Park Phases 4 & 5  
BA Group - NHY

Synchro 11 Report  
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2032 Future Total PM Model  
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Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4	4	4	4	4	4	4
Traffic Volume (vph)	495	15	245	40	0	35	35
Future Volume (vph)	495	15	245	40	0	35	35
Turn Type	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2	6	6	8	8	4	4
Permitted Phases	2	6	6	6	8	8	4
Detector Phase	2	6	6	6	8	8	4
Switch Phase	17.0	17.0	17.0	7.0	7.0	7.0	7.0
Minimum Initial (s)	24.0	24.0	24.0	26.0	26.0	26.0	26.0
Minimum Split (s)	50.0	50.0	50.0	26.0	26.0	26.0	26.0
Total Split (s)	65.8%	65.8%	65.8%	34.2%	34.2%	34.2%	34.2%
Total Split (%)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Lost Time Adjust (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag	None	None	None	None	None	None	None
Lead-Lag Optimize?	C-Min	C-Min	C-Min	C-Min	C-Min	C-Min	C-Min
Recall Mode	47.6	47.6	47.6	18.4	18.4	18.4	18.4
Act Effct Green (s)	0.63	0.63	0.63	0.24	0.24	0.24	0.24
Actuated g/C Ratio	0.27	0.27	0.27	0.23	0.23	0.23	0.23
v/C Ratio	5.1	7.0	7.0	13.2	13.2	17.5	17.5
Control Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	5.1	7.0	7.0	13.2	13.2	17.5	17.5
Total Delay	A	A	A	B	B	B	B
LOS	5.1	7.0	7.0	13.2	13.2	17.5	17.5
Approach Delay	A	A	A	B	B	B	B
Approach LOS	A	A	A	B	B	B	B
Intersection Summary	A	A	A	B	B	B	B
Cycle Length: 76	A	A	A	B	B	B	B
Actuated Cycle Length: 76	A	A	A	B	B	B	B
Offset: 25 (33%), Referenced to phase 2EBT and 6WBTL Start of 1st Green	A	A	A	B	B	B	B
Natural Cycle: 50	A	A	A	B	B	B	B
Control Type: Actuated-Coordinated	A	A	A	B	B	B	B
Maximum v/C Ratio: 0.32	A	A	A	B	B	B	B
Intersection Signal Delay: 7.7	A	A	A	B	B	B	B
Intersection Capacity Utilization 49.4%	A	A	A	B	B	B	B
Analysis Period (min) 15	A	A	A	B	B	B	B



Spills and Phases: 22: Sackville Street & Dundas Street E

Regent Park Phases 4 & 5  
BA Group - NHY

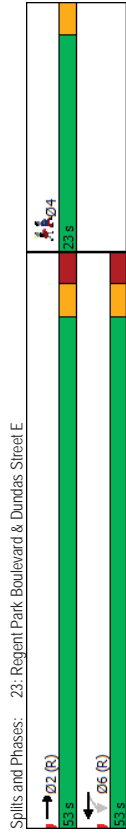
Synchro 11 Report  
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	EBT	WBT	NBT	SBT
Lane Group	564	279	75	124
Lane Group Flow (vph)	0.27	0.26	0.23	0.32
v/c Ratio	5.1	7.0	13.2	17.5
Control Delay	0.0	0.0	0.0	0.0
Queue Delay	5.1	7.0	13.2	17.5
Total Delay	12.6	17.6	3.4	9.1
Queue Length 50th (m)	16.5	21.8	12.9	21.9
Queue Length 95th (m)	99.8	79.6	36.2	126.8
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	2059	1061	367	430
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.27	0.26	0.20	0.29
<b>Intersection Summary</b>				

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	4					4				
Traffic Volume (vph)	0	495	30	15	245	0	40	0	30	35	35	45
Future Volume (vph)	0	495	30	15	245	0	40	0	30	35	35	45
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.0	3.5
Total Lost time (s)	0.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.98	0.98	1.00	1.00	1.00	0.95	0.95	0.95	0.95	0.96	0.96	0.96
Frbp. psd/bikes	1.00	1.00	0.99	0.99	0.99	0.96	0.96	0.96	0.96	0.97	0.97	0.97
Frt	1.00	0.99	1.00	1.00	1.00	0.94	0.94	0.94	0.94	0.98	0.98	0.98
Flt Protected	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Satd. Flow (prot)	3290	1755	1755	1755	1755	1466	1466	1466	1466	1601	1601	1601
Flt Permitted	1.00	0.96	0.96	0.96	0.96	0.79	0.79	0.79	0.79	0.89	0.89	0.89
Satd. Flow (perm)	3290	1694	1694	1694	1694	1217	1217	1217	1217	1450	1450	1450
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	0	532	32	16	263	0	43	0	32	38	38	48
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	564	0	0	279	0	42	0	42	0	0	93
Conf. Peds. (#/hr)	105	175	175	175	105	70	55	55	55	70	70	70
Conf. Bikes (#/hr)	0	35	35	35	20	20	20	20	20	20	20	20
Heavy Vehicles (%)	0%	5%	6%	0%	6%	0%	0%	0%	12%	0%	0%	5%
Turn Type	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
Protected Phases	2	6	6	6	6	8	8	8	8	4	4	4
Permitted Phases												
Actuated Green, G (s)	46.6	46.6	46.6	46.6	46.6	17.4	17.4	17.4	17.4	17.4	17.4	17.4
Effective Green, g (s)	47.6	47.6	47.6	47.6	47.6	18.4	18.4	18.4	18.4	18.4	18.4	18.4
Actuated g/C Ratio	0.63	0.63	0.63	0.63	0.63	0.24	0.24	0.24	0.24	0.24	0.24	0.24
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	2060	1060	1060	1060	1060	294	294	294	294	351	351	351
v/s Ratio Prot	c0.17											
v/s Ratio Perm	0.27	0.16	0.16	0.16	0.16	0.03	0.03	0.03	0.03	0.06	0.06	0.06
v/c Ratio	6.4	6.4	6.4	6.4	6.4	0.14	0.14	0.14	0.14	0.26	0.26	0.26
Uniform Delay, d1	0.67	0.87	0.87	0.87	0.87	22.6	22.6	22.6	22.6	23.3	23.3	23.3
Progression Factor	0.3	0.3	0.3	0.3	0.3	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	4.6	6.1	6.1	6.1	6.1	0.2	0.2	0.2	0.2	0.4	0.4	0.4
Delay (s)	A	A	A	A	A	C	C	C	C	C	C	C
Level of Service	A	A	A	A	A	C	C	C	C	C	C	C
Approach Delay (s)	4.6	6.1	6.1	6.1	6.1	22.8	22.8	22.8	22.8	23.7	23.7	23.7
Approach LOS	A	A	A	A	A	C	C	C	C	C	C	C
<b>Intersection Summary</b>												
HCM 2000 Control Delay	8.6 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.27											
Actuated Cycle Length (s)	76.0 Sum of lost time (s) 10.0											
Intersection Capacity Utilization	49.4% ICU Level of Service A											
Analysis Period (min)	15											
c Critical Lane Group												



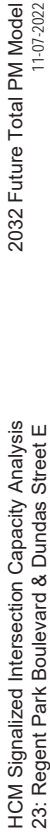
Lane Group	EBT	WBL	WBT	Ø4
Lane Configurations	1	1	1	4
Traffic Volume (vph)	530	15	260	
Future Volume (vph)	530	15	260	
Turn Type	NA	Perm	NA	NA
Protected Phases	2	6	6	4
Permitted Phases	2	6	6	4
Detector Phase	2	6	6	
Switch Phase				
Minimum Initial (s)	18.0	18.0	18.0	20.0
Minimum Split (s)	23.9	23.9	23.9	23.0
Total Split (s)	53.0	53.0	53.0	23.0
Total Split (%)	69.7%	69.7%	69.7%	30%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	2.9	2.9	2.9	0.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	
Total Lost Time (s)	4.9	4.9	4.9	
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	C-Min	C-Min	C-Min	None
Act Effct Green (s)	64.8	64.8	64.8	
Actuated g/C Ratio	0.85	0.85	0.85	
v/c Ratio	0.43	0.21	0.21	
Control Delay	3.3	4.0	4.0	
Queue Delay	0.0	0.0	0.0	
Total Delay	3.3	4.0	4.0	
LOS	A	A	A	
Approach Delay	3.3	4.0	4.0	
Approach LOS	A	A	A	
<b>Intersection Summary</b>				
Cycle Length: 76				
Actuated Cycle Length: 76				
Offset: 22 (29%), Referenced to phase 2EBT and 6WBT, Start of 1st Green				
Natural Cycle: 60				
Control Type: Actuated-Coordinated				
Maximum v/c Ratio: 0.43				
Intersection Signal Delay: 3.5				
Intersection Capacity Utilization 49.8%				
Analysis Period (min) 15				



Lane Group	EBT	WBT
Lane Group Flow (vph)	630	309
v/c Ratio	0.43	0.21
Control Delay	3.3	4.0
Queue Delay	0.0	0.0
Total Delay	3.3	4.0
Queue Length 50th (m)	1.7	0.0
Queue Length 95th (m)	52.4	28.6
Internal Link Dist (m)	79.6	86.0
Turn Bay Length (m)		
Base Capacity (vph)	1462	1448
Starvation Cap Reductn	41	0
Spillback Cap Reductn	0	0
Storage Cap Reductn	0	0
Reduced v/c Ratio	0.44	0.21
<b>Intersection Summary</b>		

2032 Future Total PM Model  
11-07-2022

23: Regent Park Boulevard & Dundas Street E



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4	4	4	4	4	4
Traffic Volume (vph)	530	30	15	260	0	0
Future Volume (vph)	530	30	15	260	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.0	3.0	3.5	3.0	3.0
Total Lost time (s)	4.9	4.9	4.9	4.9	4.9	4.9
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.97	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00
Frt	0.99	1.00	1.00	1.00	1.00	1.00
Flt Protected	1.00	1.00	1.00	1.00	1.00	1.00
Sat'd. Flow (prot)	1712	1712	1755	1755	1755	1755
Flt Permitted	1.00	0.96	0.96	0.96	0.96	0.96
Sat'd. Flow (perm)	1712	1695	1695	1695	1695	1695
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	591	34	17	292	0	0
RTOR Reduction (vph)	1	0	0	0	0	0
Lane Group Flow (vph)	629	0	0	309	0	0
Conf. Peds. (#/hr)	235	235	60	65	65	65
Heavy Vehicles (%)	6%	6%	0%	6%	0%	0%
Turn Types	NA	Perm	NA	NA	NA	NA
Protected Phases	2	6	6	6	6	6
Permitted Phases	6	6	6	6	6	6
Actuated Green, G (s)	59.1	59.1	60.1	60.1	60.1	60.1
Effective Green, g (s)	60.1	60.1	60.1	60.1	60.1	60.1
Actuated g/C Ratio	0.79	0.79	0.79	0.79	0.79	0.79
Clearance Time (s)	5.9	5.9	5.9	5.9	5.9	5.9
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap. (vph)	1353	1340	1340	1340	1340	1340
v/s Ratio Prot	0.37	0.37	0.37	0.37	0.37	0.37
v/s Ratio Perm	0.18	0.18	0.18	0.18	0.18	0.18
w/s Ratio	0.46	0.46	0.23	0.23	0.23	0.23
Uniform Delay, d1	2.6	2.6	2.0	2.0	2.0	2.0
Progression Factor	0.50	0.50	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.1	1.1	0.4	0.4	0.4	0.4
Delay (s)	2.4	2.4	2.4	2.4	2.4	2.4
Level of Service	A	A	A	A	A	A
Approach Delay (s)	2.4	2.4	2.4	2.4	2.4	2.4
Approach LOS	A	A	A	A	A	A
Intersection Summary						
HCM 2000 Control Delay	2.4 HCM 2000 Level of Service A					
HCM 2000 Volume to Capacity ratio	0.41					
Actuated Cycle Length (s)	76.0 Sum of lost time (s) 7.9					
Intersection Capacity Utilization	49.8% ICU Level of Service A					
Analysis Period (min)	15					
c. Critical Lane Group						

2032 Future Total PM Model  
11-07-2022

24: Sumach Street & Dundas Street E



Movement	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	4	4	4	4	4	4	4	4
Traffic Volume (vph)	45	465	30	245	25	90	25	10
Future Volume (vph)	45	465	30	245	25	90	25	10
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	2, 9	6, 10	8	8	4	4	4	4
Detector Phase	2	2	6	6	8	8	4	4
Switch Phase	2	2	6	6	8	8	4	4
Minimum Initial (s)	20.0	20.0	20.0	20.0	20.0	20.0	20.0	20.0
Minimum Split (s)	26.0	26.0	26.0	26.0	21.0	21.0	49.0	49.0
Total Split (s)	26.0	26.0	26.0	26.0	50.0	50.0	49.0	49.0
Total Split (%)	20.8%	20.8%	20.8%	20.8%	40%	40%	39%	39%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	2.0	2.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag	None	None	None	None	None	None	None	None
Lead-Lag Optimize?	63.1	63.1	63.1	63.1	22.8	22.8	22.8	22.8
Recall Mode	0.66	0.66	0.66	0.66	0.24	0.24	0.24	0.24
Actuated g/C Ratio	0.55	0.55	0.51	0.47	0.13	0.13	0.13	0.13
w/C Ratio	9.4	9.4	7.8	40.9	34.8	34.8	34.8	34.8
Control Delay	2.2	2.2	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	11.6	11.6	7.8	40.9	34.8	34.8	34.8	34.8
Total Delay	B	B	A	D	D	D	C	C
LOS	B	B	A	D	D	D	C	C
Approach Delay	11.6	11.6	7.8	40.9	34.8	34.8	34.8	34.8
Approach LOS	B	B	A	D	D	D	C	C
Intersection Summary								
Cycle Length	125							
Actuated Cycle Length	95.7							
Natural Cycle	100							
Control Type	Actuated-Uncoordinated							
Maximum w/C Ratio	0.55							
Intersection Signal Delay	15.1							
Intersection Capacity Utilization	62.9%							
Analysis Period (min)	15							
Spills and Phases	24: Sumach Street & Dundas Street E							
Phase	Ø2	Ø9	Ø4	Ø6	Ø8	Ø5	Ø7	Ø3
Duration (s)	59	59	59	59	59	59	59	59
Level of Service	A	A	A	A	A	A	A	A

Queues 2032 Future Total PM Model  
 24: Sumach Street & Dundas Street E 11-07-2022

	EBT	WBT	NBT	SBT
Lane Group	570	456	172	43
Lane Group Flow (vph)	0.55	0.51	0.47	0.13
v/c Ratio	9.4	7.8	40.9	34.8
Control Delay	2.2	0.0	0.0	0.0
Queue Delay	11.6	7.8	40.9	34.8
Total Delay	44.8	28.9	28.8	6.3
Queue Length 50th (m)	65.1	46.0	56.2	17.4
Queue Length 95th (m)	86.0	75.5	76.2	121.5
Internal Link Dist (m)				
Turn Bay Length (m)	1187	1015	363	322
Base Capacity (vph)	461	0	0	0
Station Cap Reductn	0	0	0	0
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.79	0.45	0.47	0.13
<b>Intersection Summary</b>				

HCM Signalized Intersection Capacity Analysis 2032 Future Total PM Model  
 24: Sumach Street & Dundas Street E 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4	4		4	4		4			4	4	
Traffic Volume (vph)	45	465	20	30	245	150	25	90	45	25	10	5	
Future Volume (vph)	45	465	20	30	245	150	25	90	45	25	10	5	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	4.0			4.0							5.0		
Lane Util. Factor	1.00			1.00							1.00		
Frbp. ped/bikes	0.98			0.89							0.96		
Frbp. psd/bikes	0.99			0.99							0.96		
Frt	1.00			1.00							0.99		
Frt Protected													
Sat'd. Flow (prot)	1692			1460							1581		
Flt Permitted	0.94			0.94							0.95		
Sat'd. Flow (perm)	1590			1383							1517		
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Adj. Flow (vph)	48	500	22	32	263	161	27	97	48	27	11	5	
RTOR Reduction (vph)	0	2	0	0	20	0	0	10	0	0	4	0	
Lane Group Flow (vph)	0	568	0	0	436	0	0	162	0	0	39	0	
Confl. Peds. (#/hr)	100	185	185	100	105	100	105	50	50	50	105	105	
Confl. Bikes (#/hr)		25			20								
Heavy Vehicles (%)	12%	6%	8%	0%	9%	5%	4%	5%	6%	0%	9%	0%	
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	
Protected Phases	2.9			6.10			8				4		
Permitted Phases	2.9			6.10			8				4		
Actuated Green, G (s)	65.6			65.6			21.7				21.7		
Effective Green, g (s)	66.6			66.6			22.7				22.7		
Actuated g/C Ratio	0.70			0.70			0.24				0.24		
Clearance Time (s)							6.0				6.0		
Vehicle Extension (s)							3.0				3.0		
Lane Grp Cap (vph)	1111			966			361				323		
v/s Ratio Prot													
v/s Ratio Perm	60.36			0.32			60.11				0.03		
v/c Ratio	0.51			0.45			0.45				0.12		
Uniform Delay, d1	6.7			6.3			31.0				28.5		
Progression Factor	1.00			1.00			1.00				1.00		
Incremental Delay, d2	0.4			0.3			0.9				0.2		
Delay (s)	7.1			6.7			31.9				28.6		
Level of Service	A			A			C				C		
Approach Delay (s)	7.1			6.7			31.9				28.6		
Approach LOS	A			A			C				C		
<b>Intersection Summary</b>													
HCM 2000 Control Delay	11.1											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.51												
Actuated Cycle Length (s)	95.3											Sum of lost time (s)	10.0
Intersection Capacity Utilization	62.9%											ICU Level of Service	B
Analysis Period (min)	15												
c Critical Lane Group													

25: Tubman Avenue & Dundas Street E

2032 Future Total PM Model  
11-07-2022

26: River Street & Dundas Street E

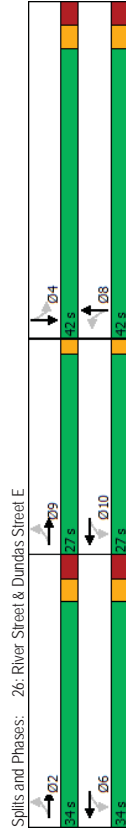
2032 Future Total PM Model  
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SBL	SBR
Lane Configurations	0	525	10	10	420	0	0	0	15	0
Traffic Volume (veh/h)	0	525	10	10	420	0	0	0	15	0
Future Volume (Veh/h)	0	525	10	10	420	0	0	0	15	0
Sign Control	Free	Free	Free	Free	Free	Free	Slop	Slop	Slop	Slop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	553	11	11	442	0	0	0	16	0
Pedestrians	10					130			50	
Lane Width (m)	3.5					0.0			3.5	
Walking Speed (m/s)	1.1					1.1			1.1	
Percent Blockage	1					0			4	
Right turn flare (veh)	None					None			None	
Median type	None					None			None	
Median storage (veh)	99					98			98	
Upstream signal (m)	694					694			694	
pX platoon unblocked	492					1168			1082	
VC, conflicting volume										
VC1, stage 1 conf vol										
VC2, stage 2 conf vol										
VCU, unblocked vol	492					1116			1017	
IC, single (s)	4.1					7.1			6.2	
IC, 2 stage (s)	2.2					3.5			3.5	
IF (s)	100					100			100	
p0 queue free %	1034					152			171	
CM capacity (veh/h)										
Direction, Lane #	EB 1	WB 1	SB 1							
Volume Total	564	453	21							
Volume Left	0	11	16							
Volume Right	11	0	5							
cSH	1034	877	204							
Volume to Capacity	0.00	0.01	0.10							
Queue Length 95th (m)	0.0	0.3	2.6							
Control Delay (s)	0.0	0.4	24.6							
Lane LOS	A	A	C							
Approach Delay (s)	0.0	0.4	24.6							
Approach LOS	C	C	C							
Intersection Summary										
Average Delay			0.7							
Intersection Capacity Utilization			46.4%						A	
Analysis Period (min)			15							

26: River Street & Dundas Street E

2032 Future Total PM Model  
11-07-2022

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	Ø1	Ø2	Ø6	Ø9	Ø10
Lane Configurations	4	4	4	4	4	4	4	4					
Traffic Volume (vph)	25	450	95	255	75	370	120	450					
Future Volume (vph)	25	450	95	255	75	370	120	450					
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA					
Protected Phases	2, 9		6, 10		8		4						
Permitted Phases	2		6		8		4						
Detector Phase													
Switch Phase													
Minimum Initial (s)					21.0		21.0		21.0		21.0		5.0
Minimum Split (s)					27.0		27.0		27.0		27.0		27.0
Total Split (s)					42.0		42.0		42.0		42.0		34.0
Total Split (%)					40.8%		40.8%		40.8%		40.8%		26%
Yellow Time (s)					3.0		3.0		3.0		3.0		2.0
All-Red Time (s)					3.0		3.0		3.0		3.0		0.0
Lost Time Adjust (s)					-1.0		-1.0		-1.0		-1.0		0.0
Total Lost Time (s)					5.0		5.0		5.0		5.0		0.0
Lead-Lag													
Lead-Lag Optimize?													
Recall Mode					Min		Min		Min		Min		None
Act Effct Green (s)		32.0			37.7		37.7		37.7		37.7		None
Actuated g/C Ratio		0.40			0.47		0.47		0.47		0.47		0.47
w/C Ratio		0.85			0.95		0.79		0.67		0.76		0.76
Control Delay		33.4			51.0		25.1		28.7		41.8		27.8
Queue Delay		0.7			0.0		0.0		0.0		0.0		0.0
Total Delay		34.1			51.0		25.1		28.7		41.8		27.8
LOS		C			D		C		D		C		C
Approach Delay		34.1			51.0		28.3		30.3		30.3		0.0
Approach LOS		C			D		C		C		C		C
Intersection Summary													
Cycle Length		103											
Actuated Cycle Length		79.9											
Natural Cycle		95											
Control Type		Actuated-Uncoordinated											
Maximum w/C Ratio		0.95											
Intersection Signal Delay		34.7											
Intersection Capacity Utilization		124.3%											
Analysis Period (min)		15											



Queues 26: River Street & Dundas Street E 2032 Future Total PM Model 11-07-2022

	EBT	WBT	NBL	NBT	SBL	SBT
Lane Group	568	479	79	615	126	579
Lane Group Flow (vph)	0.85	0.95	0.38	0.79	0.67	0.76
v/c Ratio	33.4	51.0	25.1	28.7	41.8	27.8
Control Delay	0.7	0.0	0.0	0.0	0.0	0.0
Queue Delay	34.1	51.0	25.1	28.7	41.8	27.8
Total Delay	73.2	64.2	6.7	65.1	12.8	61.5
Queue Length 50th (m)	108.4	104.7	28.6	#193.8	#58.8	#183.7
Queue Length 95th (m)	73.4	80.5		131.0		118.9
Internal Link Dist (m)			20.0		25.0	
Turn Bay Length (m)						
Base Capacity (vph)	707	532	207	775	187	762
Station Cap Reductn	24	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.83	0.90	0.38	0.79	0.67	0.76

**Intersection Summary**  
 # 95th percentile volume exceeds capacity, queue may be longer.  
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 26: River Street & Dundas Street E 2032 Future Total PM Model 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		+		+			+			+			
Traffic Volume (vph)	25	450	65	95	255	105	75	370	215	120	450	100	
Future Volume (vph)	25	450	65	95	255	105	75	370	215	120	450	100	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	5.0			5.0			5.0			5.0			
Lane Util. Factor	1.00			1.00			1.00			1.00			
Frbp. ped/bikes	0.98			0.97			1.00	0.94		1.00	0.97		
Frbp. psd/bikes	1.00			0.99			0.96	1.00		0.96	1.00		
Frt	1.00			0.97			1.00	0.94		1.00	0.97		
Flt Protected	1.00			0.99			0.95	1.00		0.95	1.00		
Satd. Flow (prot)	1722			1641			1539	1634		1592	1614		
Flt Permitted	0.97			0.75			0.26	1.00		0.23	1.00		
Satd. Flow (perm)	1669			1241			422	1634		384	1614		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Adj. Flow (vph)	26	474	68	100	268	111	79	389	226	126	474	105	
RTOR Reduction (vph)	0	5	0	0	12	0	0	17	0	0	6	0	
Lane Group Flow (vph)	0	563	0	0	467	0	79	598	0	126	573	0	
Confl. Peds. (#/hr)	55	75	75	75	75	55	75	75	75	75	75	5	
Confl. Bikes (#/hr)			25			20			10			5	
Heavy Vehicles (%)	9%	5%	0%	1%	9%	0%	5%	3%	0%	2%	12%	0%	
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	NA	
Protected Phases	2.9			6.10			8				4		
Permitted Phases	2.9			6.10			8				4		
Actuated Green, G (s)	36.7			36.7			36.7	36.7		36.7	36.7		
Effective Green, g (s)	37.7			37.7			37.7	37.7		37.7	37.7		
Actuated g/C Ratio	0.46			0.46			0.46	0.46		0.46	0.46		
Clearance Time (s)							6.0	6.0		6.0	6.0		
Vehicle Extension (s)							3.0	3.0		3.0	3.0		
Lane Grp Cap (vph)	772			574			195	756		177	747		
v/s Ratio Prot							c0.37						
v/s Ratio Perm	0.34			c0.38			0.19			0.33			
v/c Ratio	0.73			0.81			0.41	0.79		0.71	0.77		
Uniform Delay, d1	17.7			18.8			14.4	18.5		17.5	18.2		
Progression Factor	1.00			1.00			1.00	1.00		1.00	1.00		
Incremental Delay, d2	3.5			8.6			1.4	5.7		12.7	4.7		
Delay (s)	21.2			27.4			15.8	24.2		30.2	22.9		
Level of Service	C			C			B	C		C	C		
Approach Delay (s)	21.2			27.4			23.2			24.2			
Approach LOS	C			C			C			C			
Intersection Summary													
HCM 2000 Control Delay	23.8											HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.84												
Actuated Cycle Length (s)	81.4											Sum of lost time (s)	11.0
Intersection Capacity Utilization	124.3%											ICU Level of Service	H
Analysis Period (min)	15												
c Critical Lane Group													

HCM Unsignalized Intersection Capacity Analysis  
 27: Dreamers Way & Gerrard Street E

HCM Unsignalized Intersection Capacity Analysis  
 28: Dreamers Way & Site Driveway

11-07-2022

11-07-2022

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		
Traffic Volume (veh/h)	900	65	15	450	0	0
Future Volume (Veh/h)	900	65	15	450	0	0
Sign Control	Free	Free	Stop	Stop	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	947	68	16	474	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None			None		None
Median type						
Median storage (veh)	90			162		
Upstream signal (m)	0.86			0.86		0.86
pX platoon unblocked	1015			1250		508
VC, conflicting volume						
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	686			960		95
IC, single (s)	4.1			6.8		6.9
IC, 2 stage (s)	2.2			3.5		3.3
p0 queue free %	98			100		100
CM capacity (veh/h)	787			217		815
Direction, Lane #	EB 1	EB 2	WB 1	WB 2		
Volumes Total	631	384	174	316		
Volume Left	0	0	16	0		
Volume Right	0	68	0	0		
cSH	1700	1700	787	1700		
Volumes to Capacity	0.37	0.23	0.02	0.19		
Queue Length 95th (m)	0.0	0.0	0.5	0.0		
Control Delay (s)	0.0	0.0	1.1	0.0		
Lane LOS	A	A	A	A		
Approach Delay (s)	0.0		0.4			
Approach LOS			A			
Intersection Summary						
Average Delay			0.1			A
Intersection Capacity Utilization			30.3%			ICU Level of Service
Analysis Period (min)			15			

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑					↑
Traffic Volume (veh/h)	5	0	0	0	15	65
Future Volume (Veh/h)	5	0	0	0	15	65
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	5	0	0	0	16	68
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)	None			None		None
Median type						
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	100			0		0
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	100			0		0
IC, single (s)	6.4			6.2		4.1
IC, 2 stage (s)	3.5			3.3		2.2
p0 queue free %	99			100		99
CM capacity (veh/h)	895			1091		1636
Direction, Lane #	WB 1	SB 1				
Volumes Total	5	84				
Volume Left	5	16				
Volume Right	0	0				
cSH	895	1636				
Volumes to Capacity	0.01	0.01				
Queue Length 95th (m)	0.1	0.2				
Control Delay (s)	9.0	1.4				
Lane LOS	A	A				
Approach Delay (s)	9.0	1.4				
Approach LOS	A	A				
Intersection Summary						
Average Delay			1.9			A
Intersection Capacity Utilization			14.3%			ICU Level of Service
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis  
 29: Sumach Street & Site Driveway

HCM Unsignalized Intersection Capacity Analysis  
 30: River Street & Site Driveway

11-07-2022

2032 Future Total PM Model  
 11-07-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	10	0	0	0	0	35	20	160	50	0	0	0
Future Volume (Veh/h)	10	0	0	0	0	35	20	160	50	0	0	0
Sign Control	Stop	0%	0%	Stop	0%	0%	Free	Free	Free	Free	0%	0%
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	0	0	0	0	37	21	168	53	0	0	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None											
Median storage (veh)												
Upstream signal (m)	55											
pX platoon unblocked												
VC, conflicting volume	274	263	0	236	236	194	0	221				
VC1, stage 1 conf vol												
VC2, stage 2 conf vol												
VCU, unblocked vol	274	263	0	236	236	194	0	221				
IC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1	4.1				
IC, 2 stage (s)												
IF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2	2.2				
p0 queue free %	98	100	100	100	100	96	99	100				
CM capacity (veh/h)	647	637	1091	715	659	852	1636	1360				
Direction, Lane #	EB 1	WB 1	NB 1									
Volumes Total	11	37	242									
Volume Left	11	0	21									
Volume Right	0	37	53									
cSH	647	852	1636									
Volumes to Capacity	0.02	0.04	0.01									
Queue Length 95th (m)	0.4	1.0	0.3									
Control Delay (s)	10.7	9.4	0.7									
Lane LOS	B	A	A									
Approach Delay (s)	10.7	9.4	0.7									
Approach LOS	B	A	A									
Intersection Summary												
Average Delay	2.2											
Intersection Capacity Utilization	26.5%											
ICU Level of Service	A											
Analysis Period (min)	15											

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	0	25	0	585	620	40
Future Volume (Veh/h)	0	25	0	585	620	40
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	26	0	616	653	42
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (m)	241					
pX platoon unblocked						
VC, conflicting volume	1290	674	695			
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCU, unblocked vol	1236	460	486			
IC, single (s)	6.4	6.2	4.1			
IC, 2 stage (s)						
IF (s)	3.5	3.3	2.2			
p0 queue free %	100	95	100			
CM capacity (veh/h)	156	481	863			
Direction, Lane #	EB 1	NB 1	SB 1			
Volumes Total	26	616	695			
Volume Left	0	0	0			
Volume Right	26	0	42			
cSH	481	1700	1700			
Volumes to Capacity	0.05	0.36	0.41			
Queue Length 95th (m)	1.3	0.0	0.0			
Control Delay (s)	12.9	0.0	0.0			
Lane LOS	B	A	A			
Approach Delay (s)	12.9	0.0	0.0			
Approach LOS	B	A	A			
Intersection Summary						
Average Delay	0.3					
Intersection Capacity Utilization	45.1%					
ICU Level of Service	A					
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis  
 31: Site Laneway & Gerrard Street E

HCM Unsignalized Intersection Capacity Analysis  
 32: Oak Street & Site Laneway

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑			↑↑		↑
Traffic Volume (veh/h)	1020	0	0	590	0	10
Future Volume (Veh/h)	1020	0	0	590	0	10
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	1074	0	0	621	0	11
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)	171			81		
pX platoon unblocked	0.89			0.92		0.89
VC, conflicting volume	1074			1384		537
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	825			890		219
IC, single (s)	4.1			6.8		6.9
IC, 2 stage (s)						
p0 queue free %	2.2			3.5		3.3
IF (s)	100			100		98
CM capacity (veh/h)	721			264		701
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 2
Volumes Total	716	358	310	310	11	
Volume Left	0	0	0	0	0	
Volume Right	0	0	0	0	11	
CSH	1700	1700	1700	1700	701	
Volumes to Capacity	0.42	0.21	0.18	0.18	0.02	
Queue Length 95th (m)	0.0	0.0	0.0	0.0	0.4	
Control Delay (s)	0.0	0.0	0.0	0.0	10.2	
Lane LOS					B	
Approach Delay (s)	0.0	0.0	0.0	10.2		
Approach LOS				B		
Intersection Summary						
Average Delay				0.1		
Intersection Capacity Utilization				38.2%		
ICU Level of Service						A
Analysis Period (min)				15		

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Traffic Volume (veh/h)	10	95	50	0	0	0
Future Volume (Veh/h)	10	95	50	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	100	53	0	0	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX platoon unblocked						
VC, conflicting volume	53				175	53
VC1, stage 1 conf vol						
VC2, stage 2 conf vol						
VCu, unblocked vol	53				175	53
IC, single (s)	4.1				6.4	6.2
IC, 2 stage (s)						
p0 queue free %	2.2				3.5	3.3
IF (s)	99				100	100
CM capacity (veh/h)	1566				814	1020
Direction, Lane #	EB 1	WB 1	SB 1			
Volumes Total	111	53	0			
Volume Left	11	0	0			
Volume Right	0	0	0			
CSH	1566	1700	1700			
Volumes to Capacity	0.01	0.03	0.00			
Queue Length 95th (m)	0.2	0.0	0.0			
Control Delay (s)	0.8	0.0	0.0			
Lane LOS	A		A			
Approach Delay (s)	0.8	0.0	0.0			
Approach LOS			A			
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization			15.6%			
ICU Level of Service						A
Analysis Period (min)			15			



Timings 2032 Future Total AM Model 10-25-2022

Queues 2032 Future Total AM Model 10-25-2022

10: Parliament Street & Oak Street

Lane Group	WBL	NBT	SBL	SBT
Lane Configurations	W	W	S	S
Traffic Volume (vph)	70	395	5	480
Future Volume (vph)	70	395	5	480
Turn Type	Prot	NA	Perm	NA
Protected Phases	8	2	6	6
Permitted Phases	8	2	6	6
Detector Phase				
Switch Phase				
Minimum Initial (s)	7.0	21.0	21.0	21.0
Minimum Split (s)	28.0	28.0	27.0	27.0
Total Split (s)	28.0	48.0	48.0	48.0
Total Split (%)	36.8%	63.2%	63.2%	63.2%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0

10: Parliament Street & Oak Street

Lane Group	WBL	NBT	SBT
Lane Group Flow (vph)	185	427	510
v/c Ratio	0.53	0.20	0.23
Control Delay	17.3	15.5	6.2
Queue Delay	0.0	0.0	0.0
Total Delay	17.3	15.5	6.2
Queue Length 50th (m)	11.0	22.2	10.3
Queue Length 95th (m)	24.2	37.4	26.2
Internal Link Dist (m)	65.1	47.1	119.2
Turn Bay Length (m)			
Base Capacity (vph)	486	2116	2198
Starvation Cap Reductn	0	0	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.38	0.20	0.23

Intersection Summary

Cycle Length: 76
Actuated Cycle Length: 76
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL - Start of 1st Green
Natural Cycle: 60
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.53
Intersection Signal Delay: 11.6
Intersection Capacity Utilization 44.2%
Analysis Period (min) 15

Intersection Summary

Intersection Summary
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10:25-2022  
 2032 Future Total AM Model  
 HCM Signalized Intersection Capacity Analysis  
 10: Parliament Street & Oak Street

Movement	WBL	WBR	NBT	NBR	SBL	SBR
Lane Configurations	W		W	W	W	W
Traffic Volume (vph)	70	105	395	10	5	480
Future Volume (vph)	70	105	395	10	5	480
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.0	3.5	3.0	3.0	3.5
Total Lost Time (s)	5.0	5.0	5.0	3.0	3.0	3.5
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95
Frb. ped/bikes	0.89	0.99	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.92	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.98	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1375	3135	3135	3423	3423	3423
Flt Permitted	0.98	1.00	1.00	0.95	0.95	0.95
Satd. Flow (perm)	1375	3135	3135	3259	3259	3259
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	74	111	416	11	5	505
RTOR Reduction (vph)	82	0	2	0	0	0
Lane Group Flow (vph)	103	0	425	0	0	510
Conf. Peds. (#/hr)	5	175	260	260	260	260
Conf. Bikes (#/hr)				5		
Heavy Vehicles (%)	0%	5%	12%	9%	0%	4%
Turn Type	Prot	NA	NA	Perm	NA	NA
Permitted Phases	8	2			6	6
Protected Phases						
Actuated Green, G (s)	13.7	50.3			50.3	50.3
Effective Green, g (s)	14.7	51.3			51.3	51.3
Actuated g/C Ratio	0.19	0.67			0.67	0.67
Clearance Time (s)	6.0	6.0			6.0	6.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0
Lane Grp Cap (vph)	265	2116			2199	2199
v/s Ratio Prot	0.07	0.14				
v/s Ratio Perm					0.16	0.23
v/c Ratio	0.39	0.20			0.23	0.23
Uniform Delay, d1	26.7	4.6			4.8	4.8
Progression Factor	1.00	2.63			1.00	1.00
Incremental Delay, d2	0.9	0.2			0.2	0.2
Delay (s)	27.7	12.4			5.0	5.0
Level of Service	C	B			A	A
Approach Delay (s)	27.7	12.4			5.0	5.0
Approach LOS	C	B			A	A
Intersection Summary						
HCM 2000 Control Delay	11.6 HCM 2000 Level of Service B					
HCM 2000 Volume to Capacity ratio	0.27					
Actuated Cycle Length (s)	76.0 Sum of lost Time (s) 10.0					
Intersection Capacity Utilization	44.2% ICU Level of Service A					
Analysis Period (min)	15					
c Critical Lane Group						

10:25-2022  
 2032 Future Total AM Model  
 Timings  
 18: River Street & Oak Street

Lane Group	EBL	EBT	WBL	WBT	NBT	SBL	SBT
Lane Configurations	4	4	4	4	4	4	4
Traffic Volume (vph)	40	0	35	0	375	40	630
Future Volume (vph)	40	0	35	0	375	40	630
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA
Protected Phases	4	8	8	8	2	6	6
Permitted Phases	4	4	8	8	2	6	6
Switch Phase							
Minimum Initial (s)	7.0	7.0	7.0	7.0	21.0	21.0	21.0
Minimum Split (s)	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (s)	28.0	28.0	28.0	28.0	48.0	48.0	48.0
Total Split (%)	36.8%	36.8%	36.8%	36.8%	63.2%	63.2%	63.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag Optimize?							
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Act Effct Green (s)	14.1	14.1	14.1	14.1	55.5	55.5	55.5
Actuated g/C Ratio	0.19	0.19	0.19	0.19	0.73	0.73	0.73
v/c Ratio	0.28	0.34	0.34	0.34	0.61	0.61	0.61
Control Delay	14.7	12.7	7.2	7.2	11.8	11.8	11.8
Queue Delay	0.0	0.0	0.0	0.0	0.8	0.8	0.8
Total Delay	14.7	12.7	7.2	7.2	12.6	12.6	12.6
LOS	B	B	A	A	B	B	B
Approach Delay	14.7	12.7	7.2	7.2	12.6	12.6	12.6
Approach LOS	B	B	A	A	B	B	B
Intersection Summary							
Cycle Length: 76							
Actuated Cycle Length: 76							
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBLT. Start of 1st Green							
Natural Cycle: 65							
Control Type: Actuated-Coordinated							
Maximum v/c Ratio: 0.61							
Intersection Signal Delay: 11.0							
Intersection Capacity Utilization 84.5%							
Analysis Period (min) 15							
Spills and Phases: 18: River Street & Oak Street							

Queues  
18: River Street & Oak Street

2032 Future Total AM Model  
10-25-2022

	EBT	WBT	NBT	SBT
Lane Group Flow (vph)	77	109	439	736
v/c Ratio	0.28	0.34	0.34	0.61
Control Delay	14.7	12.7	7.2	11.8
Queue Delay	0.0	0.0	0.0	0.8
Total Delay	14.7	12.7	7.2	12.6
Queue Length 50th (m)	4.5	5.1	15.2	35.2
Queue Length 95th (m)	12.6	14.6	52.3	#125.3
Internal Link Dist (m)	70.5	11.7	118.9	77.4
Turn Bay Length (m)				
Base Capacity (vph)	416	472	1282	1207
Starvation Cap Reductn	0	0	0	207
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.19	0.23	0.34	0.74

**Intersection Summary**  
# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis  
18: River Street & Oak Street

2032 Future Total AM Model  
10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		+	+		+	+							
Traffic Volume (vph)	40	0	30	35	0	65	0	37.5	25	40	630	0	
Future Volume (vph)	40	0	30	35	0	65	0	37.5	25	40	630	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	
Total Lost time (s)	5.0											5.0	
Lane Util. Factor	1.00											1.00	
Frbp. ped/bikes	0.97											1.00	
Frbp. psd/bikes	1.00											1.00	
Frt	0.94											1.00	
Flt Protected	0.97											1.00	
Satd. Flow (prot)	1566											1721	
Flt Permitted	0.79											0.96	
Satd. Flow (perm)	1276											1652	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	
Adj. Flow (vph)	44	0	33	38	0	71	0	41.2	27	44	692	0	
RTOR Reduction (vph)	0	36	0	0	59	0	0	2	0	0	0	0	
Lane Group Flow (vph)	0	41	0	0	50	0	0	43.7	0	0	736	0	
Confl. Peds. (#/hr)	5		35	35		5	75		65	65		75	
Confl. Bikes (#/hr)												10	
Heavy Vehicles (%)	10%	0%	0%	5%	50%	2%	0%	5%	11%	0%	9%	0%	
Turn Type	Perm	NA	Perm	NA	NA	NA	NA	NA	Perm	NA	NA	NA	
Protected Phases	4								2			6	
Permitted Phases	4				8				2			6	
Actuated Green, G (s)	11.7				11.7				52.3			52.3	
Effective Green, g (s)	12.7				12.7				53.3			53.3	
Actuated g/C Ratio	0.17				0.17				0.70			0.70	
Clearance Time (s)	6.0				6.0				6.0			6.0	
Vehicle Extension (s)	3.0				3.0				3.0			3.0	
Lane Grp Cap (vph)	213				233				1230			1158	
v/s Ratio Prot									0.25				
v/s Ratio Perm	0.03				0.04				0.45			0.64	
v/c Ratio	0.19				0.21				0.36			0.64	
Uniform Delay, d1	27.2				27.3				4.5			6.1	
Progression Factor	1.00				1.00				1.00			1.00	
Incremental Delay, d2	0.4				0.5				0.8			2.7	
Delay (s)	27.7				27.8				5.3			8.8	
Level of Service	C				C				A			A	
Approach Delay (s)	27.7				27.8				5.3			8.8	
Approach LOS	C				C				A			A	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	10.3											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.55												
Actuated Cycle Length (s)	76.0											Sum of lost time (s)	10.0
Intersection Capacity Utilization	84.5%											ICU Level of Service	E
Analysis Period (min)	15												
c Critical Lane Group													

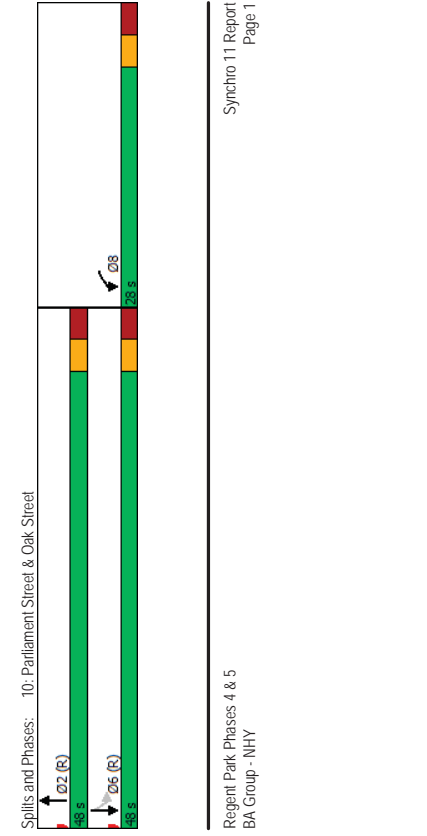
Timings 2032 Future Total PM Model 10-25-2022

Queues 2032 Future Total PM Model 10-25-2022

	WBL	NBT	SBL	SBT
Lane Group	WBL	NBT	SBL	SBT
Lane Configurations	W	T	T	T
Traffic Volume (vph)	45	520	15	380
Future Volume (vph)	45	520	15	380
Turn Type	Prot	NA	Perm	NA
Protected Phases	8	2	6	6
Permitted Phases	8	2	6	6
Detector Phase				
Switch Phase				
Minimum Initial (s)	7.0	21.0	21.0	21.0
Minimum Split (s)	28.0	27.0	27.0	27.0
Total Split (s)	28.0	48.0	48.0	48.0
Total Split (%)	36.8%	63.2%	63.2%	63.2%
Yellow Time (s)	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0
Lead/Lag				
Lead-Lag Optimize?				
Recall Mode	None	C-Min	C-Min	C-Min
Act Effct Green (s)	14.2	55.4	55.4	55.4
Actuated g/C Ratio	0.19	0.73	0.73	0.73
v/c Ratio	0.38	0.27	0.21	0.21
Control Delay	13.6	2.3	5.8	5.8
Queue Delay	0.0	0.0	0.0	0.0
Total Delay	13.6	2.3	5.8	5.8
LOS	B	A	A	A
Approach Delay	13.6	2.3	5.8	5.8
Approach LOS	B	A	A	A

	WBL	NBT	SBT
Lane Group	WBL	NBT	SBT
Lane Group Flow (vph)	135	647	465
v/c Ratio	0.38	0.27	0.21
Control Delay	13.6	2.3	5.8
Queue Delay	0.0	0.0	0.0
Total Delay	13.6	2.3	5.8
Queue Length 50th (m)	7.1	2.4	8.0
Queue Length 95th (m)	15.9	10.4	22.5
Internal Link Dist (m)	65.1	47.1	119.2
Turn Bay Length (m)			
Base Capacity (vph)	520	2380	2208
Starvation Cap Reductn	0	0	0
Spillback Cap Reductn	0	0	0
Storage Cap Reductn	0	0	0
Reduced v/c Ratio	0.26	0.27	0.21

Intersection Summary	
Cycle Length:	76
Actuated Cycle Length:	76
Offset:	0 (0%), Referenced to phase 2:NBT and 6:SBTL - Start of 1st Green
Natural Cycle:	55
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.38
Intersection Signal Delay:	4.8
Intersection Capacity Utilization:	46.7%
Analysis Period (min):	15



10-25-2022  
 2032 Future Total PM Model  
 HCM Signalized Intersection Capacity Analysis  
 10: Parliament Street & Oak Street

Movement	WBL	WBR	NBT	NBR	SBL	SBR
Lane Configurations	W	W	T	T	T	T
Traffic Volume (vph)	45	70	520	30	15	380
Future Volume (vph)	45	70	520	30	15	380
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.0	3.5	3.0	3.0	3.5
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	0.95	0.95	0.98	1.00	0.95
Frb. ped/bikes	0.96	0.98	0.98	1.00	1.00	0.99
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frb. protected	0.98	0.92	0.99	1.00	1.00	1.00
Sat'd Flow (prot)	1531	3262	3262	3272	3272	3272
Frb. permitted	0.98	1.00	1.00	0.92	0.92	0.92
Sat'd Flow (perm)	1531	3262	3262	3030	3030	3030
Peak-hour factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Adj. Flow (vph)	53	82	612	35	18	447
RTOR Reduction (vph)	68	0	4	0	0	0
Lane Group Flow (vph)	67	0	643	0	0	465
Conf. Peds. (#/hr)	5	55	220	220	220	220
Conf. Bikes (#/hr)	0	0	6	4%	12%	8%
Heavy Vehicles (%)	0%	0%	6%	4%	12%	8%
Turn Type	Prot	NA	NA	Perm	NA	NA
Protected Phases	8	2			6	6
Permitted Phases						
Actuated Green, G (s)	11.8	52.2	52.2		52.2	52.2
Effective Green, g (s)	12.8	53.2	53.2		53.2	53.2
Actuated g/C Ratio	0.17	0.70	0.70		0.70	0.70
Clearance Time (s)	6.0	6.0	6.0		6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	257	2283	2283		2121	2121
v/s Ratio Prot	c0.04		c0.20			
v/s Ratio Perm					0.15	0.22
v/c Ratio	0.26	0.28	0.28		0.22	0.22
Uniform Delay, d1	27.5	4.3	4.0		4.0	4.0
Progression Factor	1.00	0.35	1.00		1.00	1.00
Incremental Delay, d2	0.5	0.3	0.2		0.2	0.2
Delay (s)	28.0	1.8	4.3		4.3	4.3
Level of Service	C	A	A		A	A
Approach Delay (s)	28.0	1.8	4.3		4.3	4.3
Approach LOS	C	A	A		A	A
Intersection Summary						
HCM 2000 Control Delay		5.5				HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio		0.28				A
Actuated Cycle Length (s)		76.0				Sum of lost time (s)
Intersection Capacity Utilization		46.7%				10.0
Analysis Period (min)		15				ICU Level of Service
						A
c Critical Lane Group						

10-25-2022  
 2032 Future Total PM Model  
 Timings  
 18: River Street & Oak Street

Lane Group	EBL	EBT	WBL	WBT	NBT	SBL	SBT
Lane Configurations							
Traffic Volume (vph)	85	5	20	0	450	50	595
Future Volume (vph)	85	5	20	0	450	50	595
Turn Type	Perm	NA	Perm	NA	NA	Perm	NA
Protected Phases	4	8	8	2		6	6
Permitted Phases	4	4	8	8	2	6	6
Detector Phase							
Switch Phase							
Minimum Initial (s)	7.0	7.0	7.0	7.0	21.0	21.0	21.0
Minimum Split (s)	28.0	28.0	28.0	28.0	28.0	28.0	28.0
Total Split (s)	28.0	28.0	28.0	28.0	48.0	48.0	48.0
Total Spill (%)	36.8%	36.8%	36.8%	36.8%	63.2%	63.2%	63.2%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead-Lag							
Lead-Lag Optimize?							
Recall Mode	None	None	None	None	C-Min	C-Min	C-Min
Ad Effct Green (s)	15.5	15.5	50.5	50.5	50.5	50.5	50.5
Actuated g/C Ratio	0.20	0.20	0.66	0.66	0.66	0.66	0.66
v/c Ratio	0.49	0.22	0.44	0.44	0.64	0.64	0.64
Control Delay	23.6	10.8	8.7	12.7			
Queue Delay	0.0	0.0	0.6	1.0			
Total Delay	23.6	10.8	9.3	13.7			
LOS	C	B	A	B			
Approach Delay	23.6	10.8	9.3	13.7			
Approach LOS	C	B	A	B			
Intersection Summary							
Cycle Length: 76							
Actuated Cycle Length: 76							
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SRTL Start of 1st Green							
Natural Cycle: 65							
Control Type: Actuated-Coordinated							
Maximum v/c Ratio: 0.64							
Intersection Signal Delay: 13.0							Intersection LOS: B
Intersection Capacity Utilization 90.9%							ICU Level of Service E
Analysis Period (min) 15							
Spills and Phases: 18: River Street & Oak Street							

Queues 18: River Street & Oak Street 2032 Future Total PM Model 10-25-2022

	EBT	WBT	NBT	SBT
Lane Group	151	76	532	701
Lane Group Flow (vph)	0.49	0.22	0.44	0.64
v/c Ratio	23.6	10.8	8.7	12.7
Control Delay	0.0	0.0	0.6	1.0
Queue Delay	23.6	10.8	9.3	13.7
Total Delay	14.9	2.7	26.3	43.7
Queue Length 50th (m)	26.9	11.1	65.7	112.9
Queue Length 95th (m)	70.5	11.7	118.9	74.6
Internal Link Dist (m)				
Turn Bay Length (m)				
Base Capacity (vph)	436	482	1217	1099
Station Cap Reductn	0	0	330	178
Spillback Cap Reductn	0	0	0	0
Storage Cap Reductn	0	0	0	0
Reduced v/c Ratio	0.35	0.16	0.60	0.76
Intersection Summary				

HCM Signalized Intersection Capacity Analysis 18: River Street & Oak Street 2032 Future Total PM Model 10-25-2022

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	4		4	4		4				
Traffic Volume (vph)	85	5	50	20	0	50	0	450	40	50	595	0
Future Volume (vph)	85	5	50	20	0	50	0	450	40	50	595	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0	3.0	3.5	3.0
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frbp. ped/bikes	0.96	0.96	0.98	0.98	0.98	0.98	0.99	0.99	1.00	1.00	1.00	1.00
Frbp. ped/bikes	1.00	1.00	0.98	0.98	0.98	0.98	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.95	0.95	0.90	0.90	0.99	0.99	1.00	1.00	1.00	1.00	1.00	1.00
Flt Protected	0.97	0.97	0.90	0.90	0.99	0.99	1.00	1.00	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1633	1633	1611	1611	1827	1827	1769	1769	1769	1769	1769	1769
Flt Permitted	0.80	0.80	0.90	0.90	0.90	0.90	1.00	1.00	1.00	1.00	0.93	0.93
Satd. Flow (perm)	1353	1353	1471	1471	1827	1827	1655	1655	1655	1655	1655	1655
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	92	5	54	22	0	54	0	489	43	54	647	0
RTOR Reduction (vph)	0	30	0	0	43	0	0	3	0	0	0	0
Lane Group Flow (vph)	0	121	0	0	33	0	0	529	0	0	701	0
Conf. Bikes (#/hr)	5	50	50	50	50	50	75	45	45	45	75	5
Heavy Vehicles (%)	0%	0%	5%	0%	0%	0%	0%	1%	0%	0%	6%	0%
Turn Type	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Protected Phases	4			8				2				6
Permitted Phases	4			8				2				6
Actuated Green, G (s)	14.5			14.5				49.5				49.5
Effective Green, g (s)	15.5			15.5				50.5				50.5
Actuated g/C Ratio	0.20			0.20				0.66				0.66
Clearance Time (s)	6.0			6.0				6.0				6.0
Vehicle Extension (s)	3.0			3.0				3.0				3.0
Lane Grp Cap (vph)	275			300				1213				1099
v/s Ratio Prot								0.29				
v/s Ratio Perm	0.09			0.02								0.42
v/c Ratio	0.44			0.11				0.44				0.64
Uniform Delay, d1	26.4			24.6				7.4				7.4
Progression Factor	1.00			1.00				1.00				1.00
Incremental Delay, d2	1.1			0.2				1.1				2.8
Delay (s)	27.6			24.8				7.2				10.3
Level of Service	C			C				A				B
Approach Delay (s)	27.6			24.8				7.2				10.3
Approach LOS	C			C				A				B
Intersection Summary												
HCM 2000 Control Delay			11.7					HCM 2000 Level of Service				B
HCM 2000 Volume to Capacity ratio			0.59									
Actuated Cycle Length (s)			76.0					Sum of lost time (s)				10.0
Intersection Capacity Utilization			90.9%					ICU Level of Service				E
Analysis Period (min)			15									
c Critical Lane Group												

## Appendix H: Updated Signal Warrant Review



START TIME	VOL	START HOUR	HOURLY	PEDS	HOURLY P
07:00:00	102			2	
07:15:00	96			7	
07:30:00	117			10	
07:45:00	127	07:00:00	442	8	27
08:00:00	134	07:15:00	474	6	31
08:15:00	138	07:30:00	516	13	37
08:30:00	158	07:45:00	557	97	124
08:45:00	183	08:00:00	613	64	180
09:00:00	175	08:15:00	654	21	195
09:15:00	121	08:30:00	637	12	194
09:30:00	157	08:45:00	636	7	104
09:45:00	140	09:00:00	593	11	51
10:00:00	157	09:15:00	575	6	36
10:15:00	145	09:30:00	599	7	31
10:30:00	133	09:45:00	575	11	35
10:45:00	152	10:00:00	587	8	32
11:00:00	145	10:15:00	575	10	36
11:15:00	139	10:30:00	569	36	65
11:30:00	143	10:45:00	579	17	71
11:45:00	155	11:00:00	582	17	80
12:00:00	176	11:15:00	613	15	85
12:15:00	168	11:30:00	642	27	76
12:30:00	162	11:45:00	661	21	80
12:45:00	169	12:00:00	675	12	75
13:00:00	184	12:15:00	683	6	66
13:15:00	176	12:30:00	691	5	44
13:30:00	173	12:45:00	702	13	36
13:45:00	196	13:00:00	729	12	36
14:00:00	159	13:15:00	704	13	43
14:15:00	183	13:30:00	711	12	50
14:30:00	163	13:45:00	701	5	42
14:45:00	174	14:00:00	679	26	56
15:00:00	177	14:15:00	697	58	101
15:15:00	181	14:30:00	695	95	184
15:30:00	178	14:45:00	710	52	231
15:45:00	171	15:00:00	707	18	223
16:00:00	168	15:15:00	698	22	187
16:15:00	155	15:30:00	672	15	107
16:30:00	157	15:45:00	651	10	65
16:45:00	176	16:00:00	656	18	65
17:00:00	179	16:15:00	667	34	77
17:15:00	188	16:30:00	700	37	99
17:30:00	181	16:45:00	724	11	100
17:45:00	169	17:00:00	717	15	97

F\_TOTAL  
PED  
FACTOR  
2.5

07:00:00	442	27	70
08:00:00	613	180	450
09:00:00	593	51	130
10:00:00	587	32	80
11:00:00	582	80	200
12:00:00	675	75	190
13:00:00	729	36	90
14:00:00	679	56	140
15:00:00	707	223	560
16:00:00	656	65	165
17:00:00	717	97	245

FUTURE BACKGROUND TRAFFIC FORECASTS												
	NBL	NBT	NBR	EBL	EBT	EBR	SBL	SBT	SBR	WBL	WBT	WBR
84%	0	395	10				15	475		45		65
81%		382	10	0	0	0	15	460	0	44	0	63
93%		435	11	0	0	0	17	523	0	50	0	72
100%		470	12	0	0	0	18	565	0	54	0	77
95%		469	24	0	0	0	24	360		33		52
99%		488	25	0	0	0	25	375		35		54
91%		453	23	0	0	0	23	348		32		50
100%	0	495	25				25	380		35		55
8:00:00	0	395	10	0	0	0	15	475	0	45	0	65
9:00:00	0	382	10	0	0	0	15	460	0	44	0	63
12:00:00	0	435	11	0	0	0	17	523	0	50	0	72
13:00:00	0	470	12	0	0	0	18	565	0	54	0	77
14:00:00	0	469	24	0	0	0	24	360	0	33	0	52
15:00:00	0	488	25	0	0	0	25	375	0	35	0	54
16:00:00	0	453	23	0	0	0	23	348	0	32	0	50
17:00:00	0	495	25	0	0	0	25	380	0	35	0	55
FUTURE TOTAL TRAFFIC FORECASTS												
	NBL	NBT	NBR	EBL	EBT	EBR	SBL	SBT	SBR	WBL	WBT	WBR
84%	0	395	10				5	480		70		105
81%		382	10	0	0	0	5	464	0	68	0	102
93%		435	11	0	0	0	6	529	0	77	0	116
100%		470	12	0	0	0	6	571	0	83	0	125
95%		492	28	0	0	0	14	360		43		66
99%		513	30	0	0	0	15	375		44		69
91%		476	27	0	0	0	14	348		41		64
100%	0	520	30				15	380		45		70
8:00:00	0	395	10	0	0	0	5	480	0	70	0	105
9:00:00	0	382	10	0	0	0	5	464	0	68	0	102
12:00:00	0	435	11	0	0	0	6	529	0	77	0	116
13:00:00	0	470	12	0	0	0	6	571	0	83	0	125
14:00:00	0	492	28	0	0	0	14	360	0	43	0	66
15:00:00	0	513	30	0	0	0	15	375	0	44	0	69
16:00:00	0	476	27	0	0	0	14	348	0	41	0	64
17:00:00	0	520	30	0	0	0	15	380	0	45	0	70



# Input Data Sheet

Analysis Sheet

Results Sheet

Proposed Collision

GO TO Justification:

What are the intersecting roadways?

Parliament Street / Oak Street (future background 2027)

What is the direction of the Main Road street?

North-South

When was the data collected?

## Justification 1 - 4: Volume Warrants

a.- Number of lanes on the Main Road?

2 or more

b.- Number of lanes on the Minor Road?

1

c.- How many approaches?

4

d.- What is the operating environment?

Urban

Population >= 10,000

AND

Speed < 70 km/hr

e.- What is the eight hour vehicle volume at the intersection? (Please fill in table below)

Hour Ending	Main Northbound Approach			Minor Eastbound Approach			Main Southbound Approach			Minor Westbound Approach			Pedestrians Crossing Main Road
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
8:00	0	395	10	0	0	0	15	475	0	45	0	65	180
9:00	0	382	10	0	0	0	15	460	0	44	0	63	51
12:00	0	435	11	0	0	0	17	523	0	50	0	72	75
13:00	0	470	12	0	0	0	18	565	0	54	0	77	36
14:00	0	469	24	0	0	0	24	360	0	33	0	52	56
15:00	0	488	25	0	0	0	25	375	0	35	0	54	223
16:00	0	453	23	0	0	0	23	348	0	32	0	50	65
17:00	0	495	25	0	0	0	25	380	0	35	0	55	97
<b>Total</b>	<b>0</b>	<b>3,587</b>	<b>140</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>162</b>	<b>3,486</b>	<b>0</b>	<b>328</b>	<b>0</b>	<b>488</b>	<b>783</b>

## Justification 5: Collision Experience

Preceding Months	Number of Collisions*
1-12	0
13-24	0
25-36	0

\* Include only collisions that are susceptible to correction through the installation of traffic signal control

## Justification 6: Pedestrian Volume

a.- Please fill in table below summarizing total pedestrians crossing major roadway at the intersection or in proximity to the intersection (zones). Please reference Section 4.8 of the Manual for further explanation and graphical representation.

	Zone 1		Zone 2		Zone 3 (if needed)		Zone 4 (if needed)		Total
	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	
<b>Total 8 hour pedestrian volume</b>		392		392					
<b>Factored 8 hour pedestrian volume</b>	392		392		0		0		
<b>% Assigned to crossing rate</b>	100%		100%						
<b>Net 8 Hour Pedestrian Volume at Crossing</b>									784
<b>Net 8 Hour Vehicular Volume on Street Being Crossed</b>									7,375

b.- Please fill in table below summarizing delay to pedestrians crossing major roadway at the intersection or in proximity to the intersection (zones). Please reference Section 4.8 of the Manual for further explanation and graphical representation.

	Zone 1		Zone 2		Zone 3 (if needed)		Zone 4 (if needed)		Total
	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	
<b>Total 8 hour pedestrian volume</b>	0	392	0	392	0	0	0	0	
<b>Total 8 hour pedestrians delayed greater than 10 seconds</b>	0		0				0	0	
<b>Factored volume of total pedestrians</b>	392		392		0		0		
<b>Factored volume of delayed pedestrians</b>	0		0		0		0		
<b>% Assigned to Crossing Rate</b>	100%		100%		0%		0%		
<b>Net 8 Hour Volume of Total Pedestrians</b>									784
<b>Net 8 Hour Volume of Delayed Pedestrians</b>									0

**Justification 1: Minimum Vehicle Volumes**

**Restricted Flow Urban Conditions**

Justification	Guidance Approach Lanes				Percentage Warrant								Total Across	Section Percent
	1 Lanes		2 or More Lanes		Hour Ending									
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	8:00	9:00	12:00	13:00	14:00	15:00	16:00	17:00		
	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>										
1A	480	720	600	900	1,005	974	1,108	1,196	962	1,002	929	1,015		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
1B	120	170	120	170	110	107	122	131	85	89	82	90		
	COMPLIANCE %				65	63	72	77	50	52	48	53	480	60
<b>Restricted Flow</b>					Both 1A and 1B 100% Fulfilled each of 8 hours								Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
<b>Signal Justification 1:</b>					Lesser of 1A or 1B at least 80% fulfilled each of 8 hours								Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>

**Justification 2: Delay to Cross Traffic**

**Restricted Flow Urban Conditions**

Justification	Guidance Approach Lanes				Percentage Warrant								Total Across	Section Percent
	1 lanes		2 or More lanes		Hour Ending									
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	8:00	9:00	12:00	13:00	14:00	15:00	16:00	17:00		
	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>										
2A	480	720	600	900	895	867	986	1,065	877	913	847	925		
	COMPLIANCE %				99	96	100	100	97	100	94	100	787	98
2B	50	75	50	75	225	95	125	90	89	258	97	132		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
<b>Restricted Flow</b>					Both 2A and 2B 100% Fulfilled each of 8 hours								Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
<b>Signal Justification 2:</b>					Lesser of 2A or 2B at least 80% fulfilled each of 8 hours								Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**Justification 3: Combination**

**Combination Justification 1 and 2**

Justification Satisfied 80% or More				Two Justifications Satisfied 80% or More	
Justification 1	Minimum Vehicular Volume	YES <input type="checkbox"/>	NO <input checked="" type="checkbox"/>	YES <input type="checkbox"/>	NO <input checked="" type="checkbox"/>
Justification 2	Delay Cross Traffic	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>	NOT JUSTIFIED	

**Justification 4: Four Hour Volume**

Justification	Time Period	Total Volume of Both Approaches (Main)	Heaviest Minor Approach	Required Value	Average % Compliance	Overall % Compliance
		X	Y (actual)	Y (warrant threshold)		
Justification 4	12:00	986	122	199	61 %	54 %
	13:00	1,065	131	173	76 %	
	15:00	913	89	227	39 %	
	17:00	925	90	222	40 %	

# Results Sheet

Input Sheet

Analysis Sheet

Proposed Collision

Intersection: Parliament Street / Oak Street (Future Background 2) Count Date:

## Summary Results

	Justification	Compliance	Signal Justified?	
			YES	NO
1. Minimum Vehicular Volume	A Total Volume	100 %	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Crossing Volume	60 %	<input type="checkbox"/>	<input checked="" type="checkbox"/>
2. Delay to Cross Traffic	A Main Road	98 %	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Crossing Road	100 %	<input type="checkbox"/>	<input checked="" type="checkbox"/>
3. Combination	A Justificaton 1	60 %	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Justification 2	98 %	<input type="checkbox"/>	<input checked="" type="checkbox"/>
4. 4-Hr Volume		54 %	<input type="checkbox"/>	<input checked="" type="checkbox"/>

5. Collision Experience		0 %	<input type="checkbox"/>	<input checked="" type="checkbox"/>
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6. Pedestrians	A Volume	Justification met	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Delay	Justification not met	<input type="checkbox"/>	<input checked="" type="checkbox"/>

# Input Data Sheet

Analysis Sheet

Results Sheet

Proposed Collision

GO TO Justification:

What are the intersecting roadways?

Parliament Street / Oak Street (future total 2052)

What is the direction of the Main Road street?

North-South

When was the data collected?

## Justification 1 - 4: Volume Warrants

a.- Number of lanes on the Main Road?

2 or more

b.- Number of lanes on the Minor Road?

1

c.- How many approaches?

4

d.- What is the operating environment?

Urban

Population >= 10,000

AND

Speed < 70 km/hr

e.- What is the eight hour vehicle volume at the intersection? (Please fill in table below)

Hour Ending	Main Northbound Approach			Minor Eastbound Approach			Main Southbound Approach			Minor Westbound Approach			Pedestrians Crossing Main Road
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
8:00	0	395	10	0	0	0	5	480	0	70	0	105	450
9:00	0	382	10	0	0	0	5	464	0	68	0	102	130
12:00	0	435	11	0	0	0	6	529	0	77	0	116	190
13:00	0	470	12	0	0	0	6	571	0	83	0	125	90
14:00	0	492	28	0	0	0	14	360	0	43	0	66	140
15:00	0	513	30	0	0	0	15	375	0	44	0	69	560
16:00	0	476	27	0	0	0	14	348	0	41	0	64	165
17:00	0	520	30	0	0	0	15	380	0	45	0	70	245
<b>Total</b>	<b>0</b>	<b>3,683</b>	<b>158</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>80</b>	<b>3,507</b>	<b>0</b>	<b>471</b>	<b>0</b>	<b>717</b>	<b>1,970</b>

## Justification 5: Collision Experience

Preceding Months	Number of Collisions*
1-12	0
13-24	0
25-36	0

\* Include only collisions that are susceptible to correction through the installation of traffic signal control

## Justification 6: Pedestrian Volume

a.- Please fill in table below summarizing total pedestrians crossing major roadway at the intersection or in proximity to the intersection (zones). Please reference Section 4.8 of the Manual for further explanation and graphical representation.

	Zone 1		Zone 2		Zone 3 (if needed)		Zone 4 (if needed)		Total
	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	
Total 8 hour pedestrian volume		985		985					
Factored 8 hour pedestrian volume	985		985		0		0		
% Assigned to crossing rate	100%		100%						
Net 8 Hour Pedestrian Volume at Crossing									1,970
Net 8 Hour Vehicular Volume on Street Being Crossed									7,428

b.- Please fill in table below summarizing delay to pedestrians crossing major roadway at the intersection or in proximity to the intersection (zones). Please reference Section 4.8 of the Manual for further explanation and graphical representation.

	Zone 1		Zone 2		Zone 3 (if needed)		Zone 4 (if needed)		Total
	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	
Total 8 hour pedestrian volume	0	985	0	985	0	0	0	0	
Total 8 hour pedestrians delayed greater than 10 seconds	0		0				0	0	
Factored volume of total pedestrians	985		985		0		0		
Factored volume of delayed pedestrians	0		0		0		0		
% Assigned to Crossing Rate	100%		100%		0%		0%		
Net 8 Hour Volume of Total Pedestrians									1,970
Net 8 Hour Volume of Delayed Pedestrians									0

**Justification 1: Minimum Vehicle Volumes**

**Restricted Flow Urban Conditions**

Justification	Guidance Approach Lanes				Percentage Warrant								Total Across	Section Percent
	1 Lanes		2 or More Lanes		Hour Ending									
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	8:00	9:00	12:00	13:00	14:00	15:00	16:00	17:00		
	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>										
1A	480	720	600	900	1,065	1,031	1,174	1,267	1,003	1,046	970	1,060		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
1B	120	170	120	170	175	170	193	208	109	113	105	115		
	COMPLIANCE %				100	100	100	100	64	66	62	68	660	83
<b>Restricted Flow Signal Justification 1:</b>					Both 1A and 1B 100% Fulfilled each of 8 hours Lesser of 1A or 1B at least 80% fulfilled each of 8 hours								Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
													Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**Justification 2: Delay to Cross Traffic**

**Restricted Flow Urban Conditions**

Justification	Guidance Approach Lanes				Percentage Warrant								Total Across	Section Percent
	1 lanes		2 or More lanes		Hour Ending									
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	8:00	9:00	12:00	13:00	14:00	15:00	16:00	17:00		
	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>										
2A	480	720	600	900	890	861	981	1,059	894	933	865	945		
	COMPLIANCE %				99	96	100	100	99	100	96	100	790	99
2B	50	75	50	75	520	198	267	173	183	604	206	290		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
<b>Restricted Flow Signal Justification 2:</b>					Both 2A and 2B 100% Fulfilled each of 8 hours Lesser of 2A or 2B at least 80% fulfilled each of 8 hours								Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
													Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**Justification 3: Combination**

**Combination Justification 1 and 2**

Justification Satisfied 80% or More				Two Justifications Satisfied 80% or More	
Justification 1	Minimum Vehicular Volume	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>
Justification 2	Delay Cross Traffic	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>	JUSTIFIED	

**Justification 4: Four Hour Volume**

Justification	Time Period	Total Volume of Both Approaches (Main)	Heaviest Minor Approach	Required Value	Average % Compliance	Overall % Compliance
		X	Y (actual)	Y (warrant threshold)		
Justification 4	12:00	981	193	201	96 %	75 %
	13:00	1,059	208	175	100 %	
	15:00	933	113	219	52 %	
	17:00	945	115	215	54 %	

# Results Sheet

Input Sheet

Analysis Sheet

Proposed Collision

Intersection: Parliament Street / Oak Street (Future Total 2032) Count Date:

## Summary Results

Justification		Compliance		Signal Justified?	
				YES	NO
1. Minimum Vehicular Volume	A Total Volume	100	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Crossing Volume	83	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
2. Delay to Cross Traffic	A Main Road	99	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Crossing Road	100	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
3. Combination	A Justificaton 1	83	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
	B Justification 2	99	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
4. 4-Hr Volume		75	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>

5. Collision Experience	0	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
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6. Pedestrians	A Volume	Justification met	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Delay	Justification not met	<input type="checkbox"/>	<input checked="" type="checkbox"/>

START TIME	VOL	START HOUR	HOURLY	PEDS	HOURLY P
07:00:00	98			16	
07:15:00	116			7	
07:30:00	128			5	
07:45:00	123	07:00:00	465	5	33
08:00:00	166	07:15:00	533	10	27
08:15:00	157	07:30:00	574	13	33
08:30:00	153	07:45:00	599	14	42
08:45:00	166	08:00:00	642	12	49
09:00:00	134	08:15:00	610	10	49
09:15:00	168	08:30:00	621	18	54
09:30:00	135	08:45:00	603	13	53
09:45:00	156	09:00:00	593	6	47
10:00:00	150	09:15:00	609	8	45
10:15:00	151	09:30:00	592	9	36
10:30:00	151	09:45:00	608	13	36
10:45:00	146	10:00:00	598	9	39
11:00:00	134	10:15:00	582	6	37
11:15:00	152	10:30:00	583	10	38
11:30:00	160	10:45:00	592	12	37
11:45:00	164	11:00:00	610	15	43
12:00:00	164	11:15:00	640	14	51
12:15:00	160	11:30:00	648	21	62
12:30:00	176	11:45:00	664	18	68
12:45:00	140	12:00:00	640	10	63
13:00:00	176	12:15:00	652	10	59
13:15:00	157	12:30:00	649	8	46
13:30:00	160	12:45:00	633	13	41
13:45:00	175	13:00:00	668	11	42
14:00:00	167	13:15:00	659	4	36
14:15:00	157	13:30:00	659	8	36
14:30:00	177	13:45:00	676	13	36
14:45:00	157	14:00:00	658	15	40
15:00:00	201	14:15:00	692	7	43
15:15:00	201	14:30:00	736	16	51
15:30:00	189	14:45:00	748	30	68
15:45:00	179	15:00:00	770	13	66
16:00:00	192	15:15:00	761	19	78
16:15:00	170	15:30:00	730	16	78
16:30:00	186	15:45:00	727	13	61
16:45:00	198	16:00:00	746	6	54
17:00:00	181	16:15:00	735	18	53
17:15:00	204	16:30:00	769	4	41
17:30:00	187	16:45:00	770	23	51
17:45:00	204	17:00:00	776	14	59

F\_TOTAL  
PED  
FACTOR  
2.5

07:00:00	465	33	85
08:00:00	642	49	125
09:00:00	593	47	120
10:00:00	598	39	100
11:00:00	610	43	110
12:00:00	640	63	160
13:00:00	668	42	105
14:00:00	658	40	100
15:00:00	770	66	165
16:00:00	746	54	135
17:00:00	776	59	150

FUTURE BACKGROUND TRAFFIC FORECASTS												
	NBL	NBT	NBR	EBL	EBT	EBR	SBL	SBT	SBR	WBL	WBT	WBR
07:00:00												
08:00:00	96%	375	25	40		15	40	525		35		65
09:00:00												
10:00:00												
11:00:00	91%		356	24	38		14	38	499		33	62
12:00:00	96%		374	25	40		15	40	523		35	65
13:00:00	100%		390	26	42		16	42	546		36	68
14:00:00	85%		369	34	72		4	38	42	475	17	42
15:00:00	99%		432	40	84		5	45	50	556	20	50
16:00:00	96%		418	38	82		5	43	48	538	19	48
17:00:00	100%		435	40	85		5	45	50	560	20	50
8:00:00	0	375	25	40	0	15	40	525	0	35	0	65
11:00:00	0	356	24	38	0	14	38	499	0	33	0	62
12:00:00	0	374	25	40	0	15	40	523	0	35	0	65
13:00:00	0	390	26	42	0	16	42	546	0	36	0	68
14:00:00	0	369	34	72	4	38	42	475	0	17	0	42
15:00:00	0	432	40	84	5	45	50	556	0	20	0	50
16:00:00	0	418	38	82	5	43	48	538	0	19	0	48
17:00:00	0	435	40	85	5	45	50	560	0	20	0	50
FUTURE TOTAL TRAFFIC FORECASTS												
	NBL	NBT	NBR	EBL	EBT	EBR	SBL	SBT	SBR	WBL	WBT	WBR
07:00:00												
08:00:00	96%	375	25	40		30	40	630		35		65
09:00:00												
10:00:00												
11:00:00	91%		356	24	38		29	38	599		33	62
12:00:00	96%		374	25	40		30	40	628		35	65
13:00:00	100%		390	26	42		31	42	656		36	68
14:00:00	85%		382	34	72		4	42	42	505	17	42
15:00:00	99%		447	40	84		5	50	50	590	20	50
16:00:00	96%		433	38	82		5	48	48	572	19	48
17:00:00	100%		450	40	85		5	50	50	595	20	50
8:00:00	0	375	25	40	0	30	40	630	0	35	0	65
11:00:00	0	356	24	38	0	29	38	599	0	33	0	62
12:00:00	0	374	25	40	0	30	40	628	0	35	0	65
13:00:00	0	390	26	42	0	31	42	656	0	36	0	68
14:00:00	0	382	34	72	4	42	42	505	0	17	0	42
15:00:00	0	447	40	84	5	50	50	590	0	20	0	50
16:00:00	0	433	38	82	5	48	48	572	0	19	0	48
17:00:00	0	450	40	85	5	50	50	595	0	20	0	50

# Input Data Sheet

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Proposed Collision

GO TO Justification:

What are the intersecting roadways?

River Street / Oak Street (Future Background 2032)

What is the direction of the Main Road street?

North-South

When was the data collected?

## Justification 1 - 4: Volume Warrants

a.- Number of lanes on the Main Road?

1

b.- Number of lanes on the Minor Road?

1

c.- How many approaches?

4

d.- What is the operating environment?

Urban

Population >= 10,000

AND

Speed < 70 km/hr

e.- What is the eight hour vehicle volume at the intersection? (Please fill in table below)

Hour Ending	Main Northbound Approach			Minor Eastbound Approach			Main Southbound Approach			Minor Westbound Approach			Pedestrians Crossing Main Road
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
8:00	0	375	25	40	0	15	40	525	0	35	0	65	49
11:00	0	356	24	38	0	14	38	499	0	33	0	62	43
12:00	0	374	25	40	0	15	40	523	0	35	0	65	63
13:00	0	390	26	42	0	16	42	546	0	36	0	68	42
14:00	0	369	34	72	4	38	42	475	0	17	0	42	40
15:00	0	432	40	84	5	45	50	556	0	20	0	50	66
16:00	0	418	38	82	5	43	48	538	0	19	0	48	54
17:00	0	435	40	85	5	45	50	560	0	20	0	50	59
<b>Total</b>	<b>0</b>	<b>3,149</b>	<b>252</b>	<b>483</b>	<b>19</b>	<b>231</b>	<b>350</b>	<b>4,222</b>	<b>0</b>	<b>215</b>	<b>0</b>	<b>450</b>	<b>416</b>

## Justification 5: Collision Experience

Preceding Months	Number of Collisions*
1-12	0
13-24	0
25-36	0

\* Include only collisions that are susceptible to correction through the installation of traffic signal control

## Justification 6: Pedestrian Volume

a.- Please fill in table below summarizing total pedestrians crossing major roadway at the intersection or in proximity to the intersection (zones). Please reference Section 4.8 of the Manual for further explanation and graphical representation.

	Zone 1		Zone 2		Zone 3 (if needed)		Zone 4 (if needed)		Total
	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	
<b>Total 8 hour pedestrian volume</b>		208		208					
<b>Factored 8 hour pedestrian volume</b>	208		208		0		0		
<b>% Assigned to crossing rate</b>	100%		100%						
<b>Net 8 Hour Pedestrian Volume at Crossing</b>									416
<b>Net 8 Hour Vehicular Volume on Street Being Crossed</b>									7,973

b.- Please fill in table below summarizing delay to pedestrians crossing major roadway at the intersection or in proximity to the intersection (zones). Please reference Section 4.8 of the Manual for further explanation and graphical representation.

	Zone 1		Zone 2		Zone 3 (if needed)		Zone 4 (if needed)		Total
	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	
<b>Total 8 hour pedestrian volume</b>	0	208	0	208	0	0	0	0	
<b>Total 8 hour pedestrians delayed greater than 10 seconds</b>	0		0				0	0	
<b>Factored volume of total pedestrians</b>	208		208		0		0		
<b>Factored volume of delayed pedestrians</b>	0		0		0		0		
<b>% Assigned to Crossing Rate</b>	100%		100%		0%		0%		
<b>Net 8 Hour Volume of Total Pedestrians</b>									416
<b>Net 8 Hour Volume of Delayed Pedestrians</b>									0



**Justification 1: Minimum Vehicle Volumes**

**Restricted Flow Urban Conditions**

Justification	Guidance Approach Lanes				Percentage Warrant								Total Across	Section Percent
	1 Lanes		2 or More Lanes		Hour Ending									
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	8:00	11:00	12:00	13:00	14:00	15:00	16:00	17:00		
	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>										
1A	480	720	600	900	1,120	1,064	1,117	1,166	1,093	1,282	1,239	1,290		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
1B	120	170	120	170	155	147	155	162	173	204	197	205		
	COMPLIANCE %				91	86	91	95	100	100	100	100	764	96
<b>Restricted Flow</b>					Both 1A and 1B 100% Fulfilled each of 8 hours								Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
<b>Signal Justification 1:</b>					Lesser of 1A or 1B at least 80% fulfilled each of 8 hours								Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**Justification 2: Delay to Cross Traffic**

**Restricted Flow Urban Conditions**

Justification	Guidance Approach Lanes				Percentage Warrant								Total Across	Section Percent
	1 lanes		2 or More lanes		Hour Ending									
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	8:00	11:00	12:00	13:00	14:00	15:00	16:00	17:00		
	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>										
2A	480	720	600	900	965	917	962	1,004	920	1,078	1,042	1,085		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
2B	50	75	50	75	124	114	138	120	133	175	160	169		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
<b>Restricted Flow</b>					Both 2A and 2B 100% Fulfilled each of 8 hours								Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>
<b>Signal Justification 2:</b>					Lesser of 2A or 2B at least 80% fulfilled each of 8 hours								Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**Justification 3: Combination**

**Combination Justification 1 and 2**

Justification Satisfied 80% or More				Two Justifications Satisfied 80% or More	
Justification 1	Minimum Vehicular Volume	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>
Justification 2	Delay Cross Traffic	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>	JUSTIFIED	

**Justification 4: Four Hour Volume**

Justification	Time Period	Total Volume of Both Approaches (Main)	Heaviest Minor Approach	Required Value	Average % Compliance	Overall % Compliance
		X	Y (actual)	Y (warrant threshold)		
Justification 4	13:00	1,004	104	101	100 %	100 %
	15:00	1,078	134	88	100 %	
	16:00	1,042	130	94	100 %	
	17:00	1,085	135	87	100 %	

# Results Sheet

[Input Sheet](#)

[Analysis Sheet](#)

[Proposed Collision](#)

Intersection: River Street / Oak Street (Future Background 2032) Count Date:

## Summary Results

Justification		Compliance		Signal Justified?	
				YES	NO
1. Minimum Vehicular Volume	A Total Volume	100	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Crossing Volume	96	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
2. Delay to Cross Traffic	A Main Road	100	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
	B Crossing Road	100	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
3. Combination	A Justificaton 1	96	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
	B Justification 2	100	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
4. 4-Hr Volume		100	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>

5. Collision Experience	0	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
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6. Pedestrians	A Volume	Justification met	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Delay	Justification not met	<input type="checkbox"/>	<input checked="" type="checkbox"/>

# Input Data Sheet

Analysis Sheet

Results Sheet

Proposed Collision

GO TO Justification:

What are the intersecting roadways?

River Street / Oak Street (Future Total 2032)

What is the direction of the Main Road street?

North-South

When was the data collected?

## Justification 1 - 4: Volume Warrants

a.- Number of lanes on the Main Road?

1

b.- Number of lanes on the Minor Road?

1

c.- How many approaches?

4

d.- What is the operating environment?

Urban

Population >= 10,000

AND

Speed < 70 km/hr

e.- What is the eight hour vehicle volume at the intersection? (Please fill in table below)

Hour Ending	Main Northbound Approach			Minor Eastbound Approach			Main Southbound Approach			Minor Westbound Approach			Pedestrians Crossing Main Road
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
8:00	0	375	25	40	0	30	40	630	0	35	0	65	125
11:00	0	356	24	38	0	29	38	599	0	33	0	62	110
12:00	0	374	25	40	0	30	40	628	0	35	0	65	160
13:00	0	390	26	42	0	31	42	656	0	36	0	68	105
14:00	0	382	34	72	4	42	42	505	0	17	0	42	100
15:00	0	447	40	84	5	50	50	590	0	20	0	50	165
16:00	0	433	38	82	5	48	48	572	0	19	0	48	135
17:00	0	450	40	85	5	50	50	595	0	20	0	50	150
<b>Total</b>	<b>0</b>	<b>3,207</b>	<b>252</b>	<b>483</b>	<b>19</b>	<b>310</b>	<b>350</b>	<b>4,775</b>	<b>0</b>	<b>215</b>	<b>0</b>	<b>450</b>	<b>1,050</b>

## Justification 5: Collision Experience

Preceding Months	Number of Collisions*
1-12	0
13-24	0
25-36	0

\* Include only collisions that are susceptible to correction through the installation of traffic signal control

## Justification 6: Pedestrian Volume

a.- Please fill in table below summarizing total pedestrians crossing major roadway at the intersection or in proximity to the intersection (zones). Please reference Section 4.8 of the Manual for further explanation and graphical representation.

	Zone 1		Zone 2		Zone 3 (if needed)		Zone 4 (if needed)		Total
	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	
Total 8 hour pedestrian volume		525		525					
Factored 8 hour pedestrian volume	525		525		0		0		
% Assigned to crossing rate	100%		100%						
Net 8 Hour Pedestrian Volume at Crossing									1,050
Net 8 Hour Vehicular Volume on Street Being Crossed									8,584

b.- Please fill in table below summarizing delay to pedestrians crossing major roadway at the intersection or in proximity to the intersection (zones). Please reference Section 4.8 of the Manual for further explanation and graphical representation.

	Zone 1		Zone 2		Zone 3 (if needed)		Zone 4 (if needed)		Total
	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	Assisted	Unassisted	
Total 8 hour pedestrian volume	0	525	0	525	0	0	0	0	
Total 8 hour pedestrians delayed greater than 10 seconds	0		0				0	0	
Factored volume of total pedestrians		525		525		0		0	
Factored volume of delayed pedestrians		0		0		0		0	
% Assigned to Crossing Rate	100%		100%		0%		0%		
Net 8 Hour Volume of Total Pedestrians									1,050
Net 8 Hour Volume of Delayed Pedestrians									0

**Justification 1: Minimum Vehicle Volumes**

**Restricted Flow Urban Conditions**

Justification	Guidance Approach Lanes				Percentage Warrant								Total Across	Section Percent
	1 Lanes		2 or More Lanes		Hour Ending									
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	8:00	11:00	12:00	13:00	14:00	15:00	16:00	17:00		
	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>										
1A	480	720	600	900	1,240	1,179	1,237	1,291	1,140	1,336	1,293	1,345		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
1B	120	170	120	170	170	162	170	177	177	209	202	210		
	COMPLIANCE %				100	95	100	100	100	100	100	100	795	99
<b>Restricted Flow Signal Justification 1:</b>					Both 1A and 1B 100% Fulfilled each of 8 hours Lesser of 1A or 1B at least 80% fulfilled each of 8 hours								Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
													Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**Justification 2: Delay to Cross Traffic**

**Restricted Flow Urban Conditions**

Justification	Guidance Approach Lanes				Percentage Warrant								Total Across	Section Percent
	1 lanes		2 or More lanes		Hour Ending									
Flow Condition	FREE FLOW	RESTR. FLOW	FREE FLOW	RESTR. FLOW	8:00	11:00	12:00	13:00	14:00	15:00	16:00	17:00		
	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>										
2A	480	720	600	900	1,070	1,017	1,067	1,114	963	1,127	1,091	1,135		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
2B	50	75	50	75	200	181	235	183	193	274	241	260		
	COMPLIANCE %				100	100	100	100	100	100	100	100	800	100
<b>Restricted Flow Signal Justification 2:</b>					Both 2A and 2B 100% Fulfilled each of 8 hours Lesser of 2A or 2B at least 80% fulfilled each of 8 hours								Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>
													Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**Justification 3: Combination**

**Combination Justification 1 and 2**

Justification Satisfied 80% or More				Two Justifications Satisfied 80% or More	
Justification 1	Minimum Vehicular Volume	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>
Justification 2	Delay Cross Traffic	YES <input checked="" type="checkbox"/>	NO <input type="checkbox"/>	JUSTIFIED	

**Justification 4: Four Hour Volume**

Justification	Time Period	Total Volume of Both Approaches (Main)	Heaviest Minor Approach	Required Value	Average % Compliance	Overall % Compliance
		X	Y (actual)	Y (warrant threshold)		
Justification 4	13:00	1,114	104	82	100 %	100 %
	15:00	1,127	139	80	100 %	
	16:00	1,091	135	86	100 %	
	17:00	1,135	140	79	100 %	

# Results Sheet

[Input Sheet](#)

[Analysis Sheet](#)

[Proposed Collision](#)

Intersection: River Street / Oak Street (Future Total 2032)

Count Date:

## Summary Results

Justification		Compliance		Signal Justified?	
				YES	NO
1. Minimum Vehicular Volume	A Total Volume	100	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Crossing Volume	99	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
2. Delay to Cross Traffic	A Main Road	100	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
	B Crossing Road	100	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
3. Combination	A Justificaton 1	99	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
	B Justification 2	100	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>
4. 4-Hr Volume		100	%	<input checked="" type="checkbox"/>	<input type="checkbox"/>

5. Collision Experience	0	%	<input type="checkbox"/>	<input checked="" type="checkbox"/>
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6. Pedestrians	A Volume	Justification met	<input type="checkbox"/>	<input checked="" type="checkbox"/>
	B Delay	Justification not met	<input type="checkbox"/>	<input checked="" type="checkbox"/>